### REZ2023-00003, HICKORY HILL II, L.L.C.

Residential Rezoning Report Beaverdam Magisterial District Board Meeting Date: July 26, 2023



### Overview

Requested Zoning	RS(c), Single-Family Residential District with conditions		
Requested Density	2.87 dwelling units per acre		
Current Zoning	A-1, Agricultural District		
Acreage	52.25 acres		
Location	North line of East Patrick Henry Road (State Route 54) at its		
	intersection with Providence Church Road (State Route 662)		
GPINs	7880-92-1335(part) and 7880-81-6802		
General Land Use Plan	Planned Business and Commercial		
Major Thoroughfare Plan	East Patrick Henry Road is shown as a Minor Arterial with a 100'		
-	ultimate right-of-way		
Case Planner	Claudia Cheely		

### **Executive Summary**

This is a request to rezone from A-1, Agricultural District, to RS(c), Single-Family Residential District with conditions, to allow 150 age-restricted, single-family detached homes for a gross density of 2.87 dwelling units per acre.

The proposed development is located on East Patrick Henry Road (State Route 54) at its intersection with Providence Church Road, just east of the Town of Ashland on the edge of the Suburban Service Area (SSA). The layout of this subdivision includes the relocation of the intersection of Providence Church Road with East Patrick Henry Road approximately 580 feet to the east from its current location to provide safer spacing from Woodside Lane. Providence Church Road currently intersects East Patrick Henry Road immediately east of Woodside Lane and is at an angle that creates unsafe turning movements. With this request, Providence Church Road will be relocated to the east and its current right-of-way is to be abandoned, with the exception of that portion that is to remain for the cul-de-sac shown for use by an adjacent property (GPIN 7880-72-6758). The abandoned right-of-way will be divided between this project and the Providence subdivision.

### Recommendations

Staff

Should the Board determine that residential development at the density proposed is appropriate, then staff recommends **APPROVAL** of this request, subject to the submitted conceptual plan and proffers.

### Planning Commission

The Planning Commission recommends APPROVAL, subject to conceptual plan and proffers submitted.

### **Planning Analysis**

This is a request to rezone 52.25 acres to RS(c), Single-Family Residential District with conditions, for the development of 150 age-restricted, single-family homes.

### Conceptual Plan

The following chart summarizes the required features of the proposed RS development as shown on the conceptual plan:

Overview Conceptual Pla	n Features		
Housing Types	150 single-family detached homes that are age-restricted		
Gross Residential Density	2.87 units per acre		
Net Residential Density	2.88 units per acre		
Lot Sizes	• Lot size range: 6,250 square feet (0.14 acres) to 8,389 square feet (0.19 acres)		
Access and Circulation	<ul> <li>Two full-movement entrances on East Patrick Henry Road that include the following road improvements:</li> <li>A left turn lane and a right turn lane with 200' of storage at the northern entrance to Relocated Providence Church Road</li> <li>A left turn lane and a right turn taper to Meyers Park Drive (main entrance to project)</li> <li>In addition, there is one full-access entrance on Providence Church Road.</li> </ul>		
Open Space	• 10.41 acres are required, and 12.65 acres of open space are provided (24.2% of site).		
Streetlights	<ul> <li>Streetlights are shown in accordance with the RS District requirements on both sides of each entrance from East Patrick Henry Road and from Providence Church Road.</li> <li>One additional streetlight is shown to the front-right of the center pavilion, which will balance the view to that feature from East Patrick Henry Road.</li> </ul>		
Landscaping	<ul> <li>The landscaping for the 25' (along Providence Church Road) and 50' (along East Patrick Henry Road) road buffers consists of deciduous and evergreen trees and shrubs and has been proffered to meet the standard in §26-264.3, but enhanced by increasing the number of trees by 25% for trees and 30% for shrubs.</li> <li>Street trees will also be provided on both sides of each road.</li> <li>In the buffers shown on the plan between the eastern and southern property lines and interior lots, existing vegetation will be maintained.</li> <li>Open spaces will also be landscaped in accordance with the landscaping plan, which includes additional evergreen shrubs where the side or rear of lots might be visible from Route 54.</li> </ul>		
Pedestrian/Bicycle Infrastructure	<ul> <li>Sidewalks are provided on both sides of all subdivision streets.</li> <li>Asphalt pedestrian paths are provided within the thoroughfare buffers and open spaces, which will connect the proposed sidewalks to the Providence subdivision via two sidewalk connections.</li> <li>Mid-block pedestrian cut-throughs are provided at two locations.</li> </ul>		

The following are unique features of the proposed subdivision, which are provided to address existing features on the property, the applicant's desired amenities for this community, and the provision of a transportation improvement that the applicant has agreed to incorporate into this development for better roadway design and safety purposes:

### • Relocation of Providence Church Road

- o Providence Church Road will be relocated 580' to the east with a design that provides a 90-degree radius at both corners where it intersects East Patrick Henry Road.
- o From the area of roadway to be abandoned, a short cul-de-sac road will be located at a possible future entrance to the adjacent parcel at the corner of Woodside Lane and East Patrick Henry Road for the purpose of maintaining that property's entrance from this location.
- o After Providence Church Road is vacated, the abandoned roadway will be divided between the proposed subdivision and the Providence Subdivision, with the exception of that portion that is to remain for the cul-de-sac shown for use by GPIN 7880-72-6758.

### • Enhanced Thoroughfare Buffers

The RS zoning district regulations do not require specific plantings within the thoroughfare buffer. However, the applicant has proffered to plant an enhanced buffer in accordance with the commercial thoroughfare buffer standard in Section 26-264.3 and increases the number of trees required by 25% and by 30% for shrubs. That equates to twenty-eight (28) trees and forty-nine (49) shrubs that will be clustered in several locations across the property's frontage on East Patrick Henry Road. Clustering plantings within the buffer (instead of planting them in a row) will create a more natural and organic design, reflecting the rural character found along Route 54 in the area.

The applicant has also revised the conceptual plan to provide rows of evergreen screening where the sides of homes would be oriented towards Route 54, further improving views from that roadway.

### • Active Open Space along East Patrick Henry Road

Three additional active open space areas are provided that expand the open space along the thoroughfare buffers on East Patrick Henry Road and include a pavilion or gazebo and trails; these spaces are 2.55 acres, 2.29 acres and 1.82 acres in size.

### • Preserved Cemetery with Pedestrian Access

A cemetery will be preserved and surrounded by a black aluminum fence and have sidewalk access. Landscaping is being provided between the end lot and the cemetery, and a sidewalk to it is provided.

### • Resident RV/Boat Storage

A parking area for residents' RVs and boats is provided that will be screened with a 6' board-on-board privacy fence and existing vegetation to the north.

The applicant has proffered to provide a vegetative screen along the rear line of the cemetery that will be clustered to create a more natural and organic design should that be determined necessary during construction plan review. They have also shown on the conceptual plan groupings of evergreen trees to the front of the RV storage area.

### • Amenitized Central Open Space

A central active open space area includes parking for the following active recreational amenities:

- o Pavilion with a fire pit
- o Pickleball courts
- o Community garden

### o Dog Park

The following are the RS standards for review of the conceptual plan found in Section 26-67.3 of the Zoning Ordinance:

- a. The open and common space areas shall be directly accessible to the largest practicable number of lots within the district. Non-adjoining lots shall be provided with safe and convenient pedestrian access to neighborhood common and open space.
  - The interior open space is central to the overall layout and provides active recreational amenities, including a pavilion with a fire pit, pickleball courts, a community gardening area, a dog park, and residential parking. Access to this space is provided by sidewalks and a pedestrian trail system that connects the entire community to this area as well as the other open spaces provided along the development's frontage on East Patrick Henry Road. The layout includes open space breaks between longer blocks of housing to allow for easier pedestrian access throughout the site.
- b. The common and open space areas shall be suitable for recreational uses, either active or passive, without interfering with adjacent dwelling units, parking, driveways, and roads.
  - The open spaces are provided in areas that are suitable for recreational uses, both active and passive. These larger open areas are near homes, but will not interfere with the dwellings, parking, driveways, and roads.
- c. The open and common space areas shall be undivided by public or private roads, except where necessary for proper traffic circulation.
  - The three main open and common space areas are not divided by roads and are large enough for activities to occur in those areas. Where the two main roads are accessed from East Patrick Henry Road, pedestrian crosswalks are provided to interconnect the separate open space areas.
- d. Open space areas shall be left in their natural state to the maximum practicable extent, unless located to provide a screening buffer from adjoining property and roadways. Pedestrian paths may be located within the open space, including the buffer areas.
  - A tree line is shown along the southern property line adjacent to existing residential development. It continues around the site along the eastern and northern lines of the property to provide a buffer between this development and adjacent residential and along Providence Church Road on the northern side. The notes indicate that existing vegetation will be maintained in these areas to provide a buffer between this development and adjacent properties.
- e. Historic features other than structures, when identified on the property, shall be incorporated into the open space areas and a preservation plan shall be provided for protection and maintenance of the features. If approved by the board, historic structures may be included in common space for use by the homeowners or may be located on a residential lot for conveyance and use as a residence.
  - There are no historic structures on the property, but there is a historic cemetery. The development will incorporate this cemetery into one of its passive open space areas, and it will be protected by a black aluminum fence. A sidewalk is shown to the cemetery for visitor access.

f. Streets shall generally be designed to minimize the area within the project devoted to vehicular travel; calm traffic speeds; promote pedestrian movement; and terminate in views of open or common space or other appropriate vista.

Providence Church Road is being relocated 580' east of Woodside Lane to provide a safer intersection design. It will continue to mainly serve the existing residents along that road but will also provide secondary access for the proposed development. The interior roads simply provide access to all interior lots. Several views to open space areas are provided from these roads, including vistas to these areas where three roads terminate at cul-de-sacs that are surrounded by open space instead of lots.

### Transitions between Suburban and Rural Areas

The proposed development is located on the edge of the Suburban Service Area (SSA), which is the County's designated growth area. The applicant is attempting to develop a residential community that is more consistent with the residential development to the north and then transition to both the rural landscape to the east and commercial development to the south. The applicant is providing a thoroughfare buffer with an enhanced planting standard that is proffered as well as a three-rail fence along the entire frontage to promote a more agricultural aesthetic across the frontage on Route 54. Additionally, wide open space areas along Route 54 further improve views from the corridor and provide a visual transition from rural to suburban areas.

### Elevations

Residential elevations have been provided that represent the architectural style proposed for this community. Each of the elevations shows one or more design elements that may be included with the future homes in this development. With each building permit, the particular elevation provided with the permit must include at least three (3) design elements. With individual homeowners choosing the design elements and colors, some variation in design should be realized.

Elevations have also been provided for a 1,320 square foot pavilion that includes a stone fireplace, a pergola, a gazebo, and the mail kiosk. These structures appear to use a consistent style and materials as provided with the elevations for the homes.

### **Transportation**

A traffic impact analysis was prepared for the proposed community of 150 age-restricted, single-family homes. The study looked at existing traffic conditions, No-Build 2027 conditions, Build 2027 conditions, and Build 2027 conditions with trip counts from Phase I of the Iron Horse Business Park added. The study included the AM and PM peak hour counts for the following intersections:

- Route 54 at Hill Carter Parkway
- Route 54 at 1-95 Southbound Ramps
- Route 54 at 1-95 Northbound Ramps
- Route 54 at Woodside Lane
- Route 54 at Providence Church Road
- Route 54 at Meyers Park Drive
- Route 54 at Goddins Hill Road

According to the study, all of the study intersections are expected to operate acceptably at build-out of the neighborhood with LOS of D or greater (Hanover County Board of Supervisors Transportation Policy dated March 13, 2013, revised July 27, 2016).

The projected build-out traffic volumes do not warrant any turn lanes on Route 54 at realigned Providence Church Road or the proposed site driveway. However, the applicant is proposing significant improvements to Route 54 and Providence Church:

- Realign Providence Church Road approximately 580 feet to the east to improve the spacing from Woodside Lane.
  - o The relocation of Providence Church Road to the east is an improvement that the County and VDOT have long supported in order to improve safety.
- Widen Route 54 to a three-lane section from Woodside Lane to Meyers Park Drive to provide eastbound left-turn lanes on Route 54 at realigned Providence Church Road and Meyers Park Drive.

The applicant has also proffered specific improvements at each intersection along Route 54, which will be designed in accordance with VDOT standards and specifications:

- Construct one eastbound left-turn lane on Route 54 with 200 feet of storage and a right-turn lane with 200 feet of storage at Relocated Providence Church Road.
- Provide separate southbound left and right-turn lanes on Relocated Providence Church Road.
- Construct one eastbound left-turn lane on Route 54 with 200 feet of storage and a 100 foot right-turn taper into the southwest entrance into the proposed subdivision (Meyers Park Drive); and
- Provide separate southbound left and right-turn lanes on the southwest entrance (Meyers Park Drive).

Some of these proposed improvements are intended to address community concerns regarding traffic flow along Route 54, including comments received during the community meeting and Planning Commission public hearing. A concern about a possible backup of right-turning traffic into the westbound through lane of Route 54 (East Patrick Henry Road) was raised. To address those concerns, the applicant is now providing a dedicated right turn lane with 200' of storage to Providence Church Road from Route 54 in accordance with VDOT standards and specifications. The number of trips at Meyer Park Drive is adequately served by the provision of a 100' right turn taper.

### Comprehensive Plan

The Comprehensive Plan designation shown for a majority of this site is *Planned Business*, with some *Commercial* at the western end of the site where it currently extends to the corner of Providence Church Road and Woodside Lane. The applicant has submitted two previous zoning applications on this site for developments that were consistent with the current land use designations. The previous applications were denied by the Board of Supervisors. In both cases, the Board agreed with nearby residents who voiced opposition to any commercial or retail development, which was consistent with the current General Land Use Plan Map.

In response to Board and citizen concerns regarding commercial development of the property, the applicant has submitted an application for residential zoning in an effort to more closely align

development of the property to the vision expressed by residents. The application for a residential development consisting of 150 single-family homes with a gross density of 2.87 dwelling units per acre is within the density range recommended within the *Suburban General* (1.5 - 3.0 dwelling units per acre) land use designation. Further, the proposal provides for age-restriction of all units in this community to mitigate traffic and school capacity concerns.

Although the application does not align with the *Planned Business* designation as shown on the Comprehensive Plan, it does appear to align more closely with the community's vision and the recommended *Suburban Neighborhood Residential* (1.5 - 3.0 dwelling units per acre) land use designation in the 2023 Draft Plan currently under review.

The General Land Use map in the Comprehensive Plan was considered by the Planning Commission at its meeting on June 15, 2023, but they had no comments on land use as it related to the subject proposal.

Should the Board of Supervisors determine that development consistent with the Suburban General land use designation is more appropriate for this location, staff has provided a review of the goals and objectives for Suburban General land use and notes the following:

• Achieve quality neighborhoods by encouraging the use of high-quality construction techniques and materials

The elevations provided show use of brick or stone foundations and include a list of materials that the homes must use. This list includes quality building materials.

• Community design should incorporate concepts outlined in Section 4, Active Living and Healthy Neighborhoods; Promote the interconnection of communities to enhance vehicular, bicycle, and pedestrian circulation and reduce demand along existing and proposed thoroughfares.

Sidewalks are provided on both sides of all interior roads, and pedestrian trails are shown throughout the common open spaces, including all road buffer areas. The trails in the buffers on the north side of the proposed development show possible pedestrian connections to the Providence Subdivision to allow both communities to use the trails and sidewalks in both communities. Providing both communities with additional trails and sidewalks promotes physical exercise and strengthens social cohesion between these communities.

• Ensure provision of adequate and safe vehicular access to thoroughfares

With development of this property, Providence Church Road will be relocated approximately 580 feet to the east to provide better spacing with Woodside Lane and to improve the design of its intersection with East Patrick Henry Road. After widening this section of East Patrick Henry Road to a three-lane section, left turn lanes will be provided for both Providence Church Road and the subdivision road, Meyers Park Drive, as well as a dedicated right turn lane to Providence Church Road. The applicant indicates that with these improvements, adequate and safe vehicular access to thoroughfares will be provided.

• Ensure provision of contiguous open space designed for passive and active recreation that is conveniently and centrally located for residents, and encourage the preservation of cultural resources and environmental features such as wetlands and Resource Protection Areas, scenic viewsheds, and existing trees/vegetation

The community provides contiguous open spaces that are designed for both passive and active recreation, including an active area that is centrally located with pedestrian and sidewalk access from all directions. A small parking lot is provided should any resident need that option. In addition, the layout provides for preservation of a cemetery, which will be protected using a black aluminum fence. No wetlands are on this property, but scenic viewsheds are provided. Existing trees and vegetation will be maintained in buffer areas.

• Ensure land uses are harmonious with surrounding uses and residential density and nonresidential uses should transition appropriately to adjoining uses

This property is located on the north line of East Patrick Henry Road and is immediately south of the Providence Subdivision. To the east is the preservation lot for the Hickory Hill Subdivision, and to the west is the Town of Ashland. With this request, the applicant has submitted a proposal that provides for similar types of homes and density of development as the Providence subdivision. The applicant has indicated that this proposal provides a harmonious transition between the Providence subdivision and the existing zoning of the development south of East Patrick Henry Road, which is currently zoned MX, Mixed Use District, and the subject of a proposal to rezone to OS, Office Service District, for the development of flex warehouse space. In addition, this property is adjacent to a major thoroughfare road where the density may be managed with the proposed roadway improvements.

• Promote variation in building elevations through diverse but complementary architectural forms, materials, colors, and texture

The elevations submitted represent the architectural styles that are proposed, and variation will be achieved through application of the rules provided on the elevations sheet that require the builder or purchaser to make choices regarding architectural features and colors with each building permit. No more than three (3) adjacent houses may be constructed with the same elevations and the same materials.

### Compatibility with Surrounding Area

This property is located on the north line of East Patrick Henry Road and is immediately south of the Providence subdivision. To the east is the preservation lot for the Hickory Hill Subdivision, and to the west is the Town of Ashland. To the south is the East Ashland/Ironhorse project, which currently has a rezoning request under review for O/S, Office/Service District, for the development of large flex space buildings, and RM, Multi-Family Residential District, for townhomes.

With this request, the applicant has submitted a proposal that provides for similar types of homes and density as the Providence subdivision. The original Providence subdivision had a density of 2.54 dwelling units per acre, but the section of Providence that was added after the original rezoning and is immediately north of the subject site was approved at a density of 2.92 dwelling units per acre. Together, the overall density in Providence is approximately 2.63 dwelling units per acre. This proposal contemplates a density of 2.87 dwelling units per acre, which is a slight reduction from the original application. The applicant has indicated that this proposal provides a harmonious transition between the Providence subdivision and the East Ashland site to the south and to the agricultural areas to the east.

However, based on community input and to further address the County's vision for transitions between rural areas and the Suburban Service Area, staff recommends that the density proposed be further reduced to a density that is less than the density of the overall Providence subdivision.

### Schools

Because the proposed development will be age-restricted, there should be no impact on the capacity of Hanover County Public Schools.

### Historical Analysis

Under the Guidelines for Historical Commission Review, zoning requests are brought to the Commission for review that are Category 1 and 2 sites, which include sites that are designated as:

- National Historic Landmarks and/or are determined to be of exceptional cultural and/or historic resources;
- Properties and districts that are on the National Register of Historic Places, or determined eligible or potentially eligible for the National Register;
- Century Farms;
- Battlefield sites; and,
- Documented historic cemeteries.

The Commission reviewed this request because the property is near two surveyed cemeteries and the Civil War feature, Jackson's March from Beaverdam to Mechanicsville, passes by this property along Providence Church Road. Both cemeteries are far enough away that they will not be impacted by the development of the subject property, and there are no physical features in this area related to the Civil War feature. The Commission determined this request will not impact the historic resources.

### Community Meeting

A community meeting was held by the applicant on April 19, 2023. Issues raised at the meeting included the following:

- <u>Traffic Concerns</u>: Questions were raised about the traffic impact analysis because it was based on 2019 data and VDOT had indicated that traffic counts on Route 54 had increased 10-19% since 2019. It was also asked why right turn lanes into the site were not being provided.
  - The traffic study was updated to include updated data and traffic counts, and its outcome did not change.
- <u>Features of the Development</u>: There were many questions about the details of the development's layout and the type of homes planned, high density, use of emergency access to Providence, and when the project would be built out. Also, there was a request for more breaks between long rows of homes for additional pedestrian connections.

• <u>Comprehensive Plan</u>: The Comprehensive Plan for this site shows *Planned Business*, and questions were asked why a comprehensive plan amendment prior to the rezoning was not being considered.

### **Agency Analysis**

Most agency comments on this request have been addressed with the applicant's resubmittals or they apply to the construction and subdivision processes. The following comments that do relate to the rezoning do not appear to have been addressed:

- VDOT VDOT had concerns about the design of some interior roads. The applicant has made
  corrections to the design of those roads with the latest conceptual plan, which was provided to
  VDOT. At the time of this report, VDOT had not yet responded as to whether the roadway
  redesign was acceptable.
- Public Utilities The Department of Public Utilities had noted that portions of the property could
  not be served by public sanitary sewer. When questioned about this comment, they indicated that
  this is only an issue for the existing conditions of the property. Coordinating the grading with the
  lot layout can lead to an acceptable design for the RS zoning. Public Utilities believes the site
  can be designed to create an approvable construction plan layout that can be served entirely by
  gravity public sanitary sewer.

### **Proffers**

The applicant has submitted the following proffers, submitted June 30, 2023:

- Age Restriction. Accept. All units are proffered to be age restricted. Age restricted communities
  produce less traffic and have no impact on the school system since no school age children will
  reside in these homes.
- Right-of-Way Dedication. Accept. The applicant has proffered to dedicate right-of-way in accordance with the Major Thoroughfare Plan for both Providence Church Road and East Patrick Henry Road.
- 3. <u>Transportation Improvements</u>. Accept. The applicant has proffered to complete roadway improvements along East Patrick Henry Road at its intersections with Relocated Providence Church Road and Meyers Park Drive in accordance with VDOT standards and specifications.
- 4. Providence Church Road. Accept. Should the portion of Providence Church Road be approved for abandonment as proposed on the conceptual plan, the applicant has proffered to construct and dedicate right-of-way for a cul-de-sac road to the access drive for the off-site parcel, GPIN 7880-72-6758, prior to such vacation or abandonment to allow access from Providence Church Road to the off-site parcel. This proffer refers to an attached exhibit that has not been provided with this application. The conceptual plan shows the proposed cul-de-sac, so the applicant should either submit the referenced exhibit or refer to the conceptual plan.
- 5. Enhanced Thoroughfare Buffer. Accept. The applicant has proffered to provide the plantings for a thoroughfare buffer along East Patrick Henry Road that is enhanced with 25% more trees and

30% more shrubs than are required for that type buffer that will be planted in appropriate clusters to create a more natural and organic design.

- 6. <u>Recreational Amenities</u>. Accept. The applicant proffers that the listed amenities must be designed, bonded, and constructed as subdivision improvements with the first section that is developed.
- 7. <u>Trailer/RV Storage Area Landscaping</u>. Accept. If existing vegetation is found inadequate to screen the storage area during construction plan review by the Zoning Administrator, the owner will provide commercial screening of the back of the storage area. If additional plantings are needed, a combination of deciduous and evergreen trees and plant materials clustered to create a natural and organic design.

### Summary/Final Analysis

While the current Comprehensive Plan land use designations for this property are for *Planned Business* and *Commercial*, two previous zoning requests that were designed to align with these designations were not supported by the Board or the community. The applicant is now proposing residential zoning in an effort to align development of the property more closely to the vision expressed by residents. The application is for an age-restricted residential development consisting of 150 single-family homes with a gross density of 2.87 dwelling units per acre, which is within the density range recommended within the *Suburban General* (1.5 - 3.0 dwelling units per acre) land use designation.

At the Planning Commission meeting, it considered the land use in its review but had no specific comments. Further, it recommended approval of the request if the staff's recommended changes were made. Therefore, the Board of Supervisors should consider whether the proposed use and density are appropriate in a location that transitions to agricultural/residential uses to the east along Route 54. The proposed density is 2.87 dwelling units per acre. While the Providence subdivision's overall density is 2.63 dwelling units per acre, Section 2 of Providence that is immediately north of the subject property had a density of 2.92 dwelling units per acre. The applicant has indicated that this demonstrates that this site is providing a density transition from Providence to the agricultural/residential areas to the east. The applicant is also providing substantial vegetative plantings in the thoroughfare buffer with rural features (e.g., a 3-rail fence) along its frontage on Route 54. Additionally, significant open space areas are provided along the Route 54 frontage. To the south of Route 54 is an area zoned MX, Mixed Use, for a mix of commercial and residential development, but that is currently under consideration by both the Town and County for office and warehouse uses with some retail along the frontage on Route 54. Finally, the land use proposed for this area under the Comprehensive Plan 2023 update is for residential development at this location. The draft maps currently show this area for Suburban Neighborhood Residential (1.5 - 3.0) dwelling units per acre).

Staff recommends that the Board make a determination as to whether residential use is appropriate at this location and whether the density of the proposed residential development is appropriate, given its location on the edge of the Suburban Service Area (SSA). Should the Board determine that residential development at the density proposed is appropriate, then staff recommends approval subject to the submitted conceptual plan and proffers.

### **Planning Commission Recommendation**

At their meeting of June 15, 2023, the Hanover County Planning Commission, on a motion by Mrs. Iverson, seconded by Mr. Bailey, voted to recommend **DENIAL** as submitted, but **APPROVAL** to rezone to RS(c), Single-Family Residential District with conditions, on GPIN(s) 7880-92-1335(part) and 7880-81-6802, consisting of approximately 52.25 acres, subject to the applicant addressing the following recommendations:

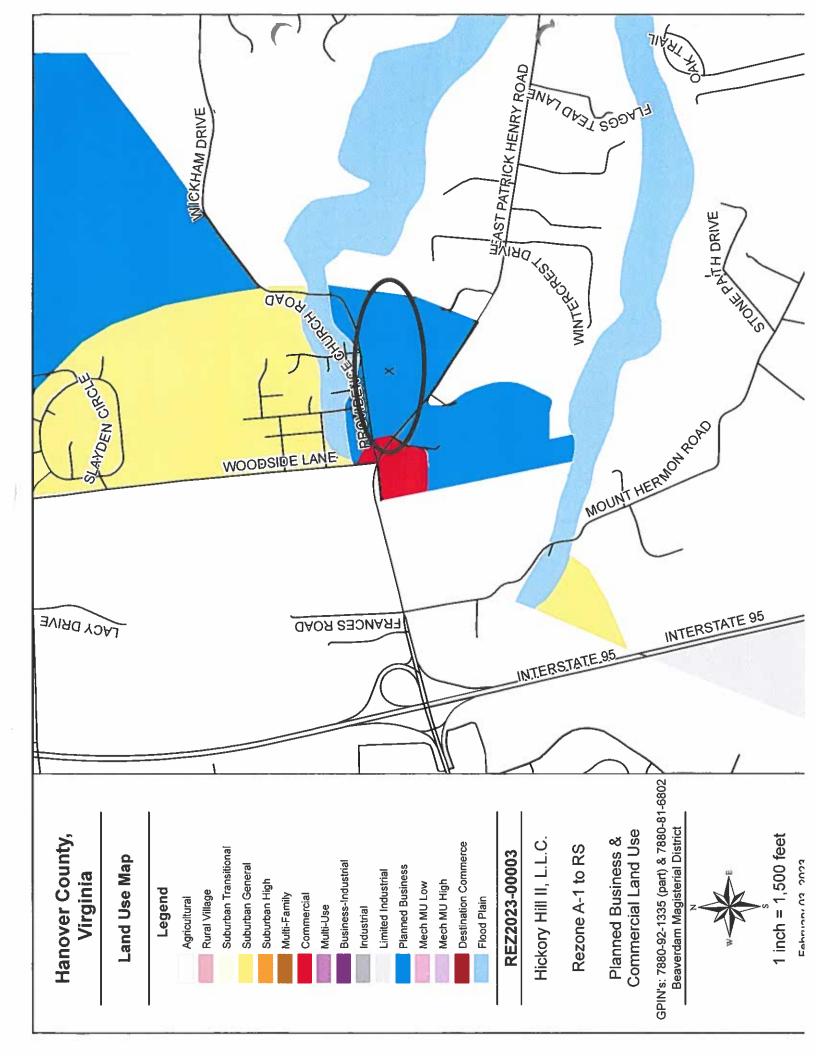
- 1. To transition from the Suburban Service Area to the rural area, density should be reduced to reflect that transition, which staff recommends be less than the density of the entire Providence Subdivision.
- 2. Staff has recommended that the landscaping for the central open space be provided in a manner that better screens views from East Patrick Henry Road to the cul-de-sac and the side yards of the last homes on that cul-de-sac. Since the proffer contemplates specific numbers of planting materials, staff suggests that more landscaping may be needed than has been proffered in order to accomplish this and also provide adequate landscaping along the remainder of the frontage along East Patrick Henry Road.
- 3. Provide additional plantings to enhance the existing vegetation around the RV storage lot to further screen this fenced area from view from Providence Church Road.
- 4. To address citizen concerns regarding traffic, the right turn tapers proposed from East Patrick Henry Road should be redesigned as right turn lanes in accordance with VDOT standards and specifications.
- 5. The inconsistencies related to the elevations and the depiction of those structures on the conceptual plan should be corrected.
- 6. The applicant should provide information as to how the applicant has resolved the Public Utilities comment that sewer may not be available to the entire site.
- 7. To address citizen comments related to the rural character of the area, staff recommends that a fence be added to the road buffer to enhance the site along the property frontage with an agricultural feature that cues an observer that they are moving from the Town of Ashland into the rural area east of the Town along East Patrick Henry Road. The Planning Commission should determine whether this recommendation is appropriate.
- 8. The conceptual plan must be amended to show landscaping in areas between paths and lots.
- 9. Proffer #4 regarding the proposed Providence Church Road cul-de-sac should be revised to correct the exhibit reference and add language that indicates when in the process the cul-de-sac design will be reviewed, approved, and constructed.
- 10. The proffers should be amended to revise Proffer #5 regarding the road buffer in accordance with staff's recommendations.

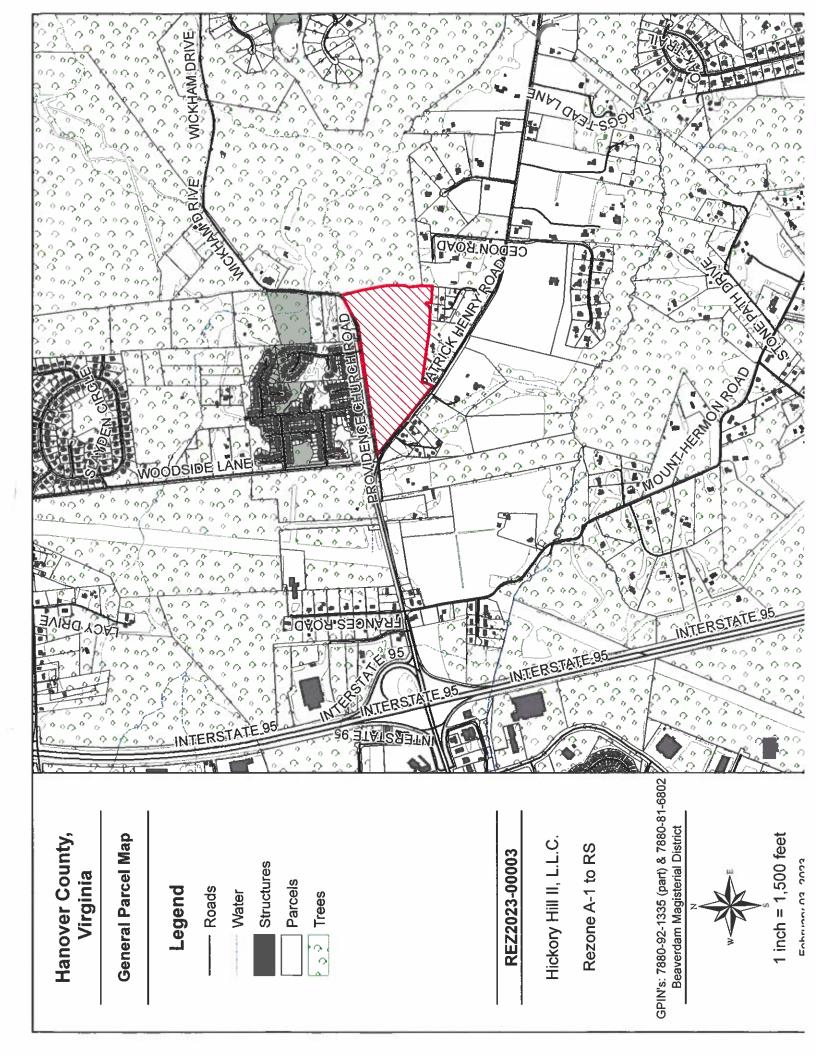
The applicant has revised the conceptual plan and proffers to address the recommended changes, with the exception of reducing the density. Therefore, should the Board determine that residential development at the density proposed is appropriate, then staff recommends approval subject to the submitted conceptual plan and proffers.

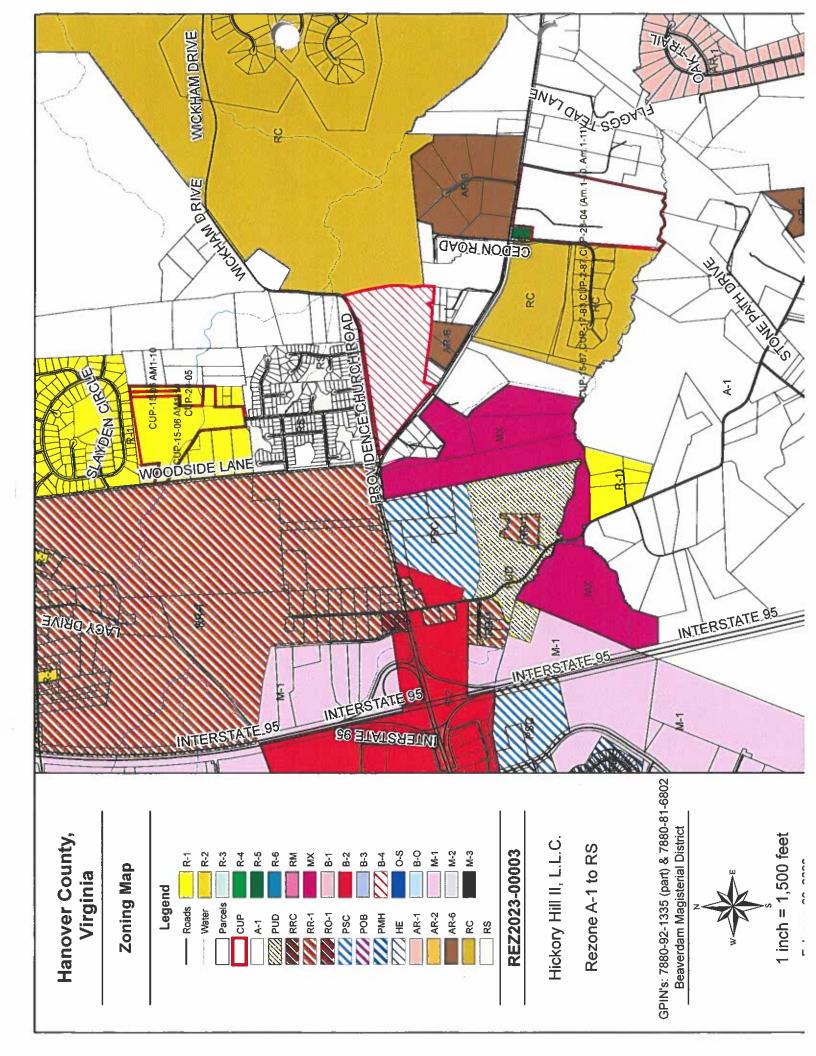
# Attachments ✓ Maps (land use, vicinity, zoning, aerial) ¬ Photographs ✓ Application ✓ Postal Verification of Adjacent Owner Notifications ✓ Traffic Study ¬ Approval Letter Proffers/Elevations/Conceptual Plan ✓ Historical Commission Recommendation ✓ Agency Review Comments ✓ Community Meeting Notes ✓ Citizen Correspondence ✓ Proffers ✓ Conceptual Plan Set with Elevations ✓ Ordinance

Initials: CDC

Maps









Application

# **Hanover County Planning Department Application**

8202 **8.2** YAM

Request	for	REZONING	Y
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RECEIVED

Case #: <u>RE22023-000</u> 03	Please type or print in <b>black</b> ink.		
APPLICANT INFORMATION			
Owner: Hickory Hill II, LLC  Contact Name: Todd D. Rogers  Address: 9245 Shady Grove Road, Suite 200  Mechanicsville, VA 23116	Telephone No. 804-569-1534 Fax No. Email Address ToddR@HTRSI.com		
Applicant/Contract Purchaser: Rogers-Chenault, Inc. Contact Name: Todd D. Rogers Address: 9245 Shady Grove Road, Suite 200 Mechanicsville, VA 23116	Telephone No. 804-569-1534 Fax No Email Address ToddR@HTRSI.com		
PARCEL INFORMATION	For multiple parcels, please complete Page 4 🗸		
7880-81-6802  Deed Book see attached Page see attached  Magisterial District Beaverdam  Location Description (Street Address, if applicable)  10035 Providence Church Rd, south of Providence  Church Rd at its intersection with Route 54	cotal Area (acres/square feet) 52.25  Current Zoning A-1  Requested Zoning RS  Requested Use 150 Age-Restricted Single-Family detached nomes		
As owner or authorized agent of this property, I hereby cerbest of my knowledge, and I authorize County representations request.  Signature  Print Name Todd D. Rogers  Signature	rtify that this application is complete and accurate to the		
Print Name			
QUESTIONS/ LETTERS/ REPORTS SHOULD BE FO	RWARDED TO THE FOLLOWING**:		
Name Todd Rogers  Address: 9245 Shady Grove Road, Suite 200  Mechanicsville, VA 23116  **It is the responsibility of the contact person to provide copies of all conterested parties to the application.	Telephone No. 804-569-1534 Fax No Email Address ToddR@HTRSI.com		

GPIN	Property Owner(s)	Deed Book	Area	Current	Requested
		and Page Number	(acres/square feet)	Zoning	Requested Zoning
7880-92-1335 (in part)	Print Hickory Hill II, LLC	2968, p158	51.30 acres	A-1	RS
7880-81-6802	Print Hickory Hill II, LLC	3217, p3428	0.95 acres	A-1	RS
	Print Sign				
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	Sign	1			
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ATTA	CHMENTS - For ALL REQUESTS you must submit the following:
×	a. Acknowledgement of Application Fee Payment Procedure (Page 6)
×	b. Adjacent property owners, Board of Supervisors, and Planning Commissioner notification form (Page 7) – please list all adjacent property owners including those across roadways, watercourses, and/or railroads as well as the members of the Board of Supervisors and Planning Commission for the magisterial district in which the property is located. Adjacent property owners, Board members, and Planning Commissioners must be notified prior to submittal of this application. The form must include owners' names, address, and GPINs for all adjacent property owners. (This information is available from the County website or may be obtained from the Planning Department.) A sample letter has been provided (Page 9), and may be used to notify the adjacent property owners.
х	c. A plat of the subject property, which accurately reflects the current property boundaries, includes metes and bounds, is drawn to scale, and shows existing structures. If the full-size plat is larger than 8 ½" x 11", the plat must be folded no larger than 9" x 12", and a reduction of the plat must be submitted which is 8 ½" x 11" in size. (Typically available from the County Clerk's Office in the Circuit Court building.)
x	d. Responses to questions on Pages 10 and 11
×	Historic Impact Information (Page 12) (This information is available on the County website or may be obtained from the Planning Department.)
x	<ul> <li>f. Traffic Impact Analysis Certification Form (Page 13) In compliance with VDOT's new Traffic Impact Analysis Regulations (24 VAC 30-155 et seq., commonly known as "Chapter 527"), rezonings that meet certain thresholds require Traffic Impact Analyses (TIA). The process for submitting a TIA is as follows: <ol> <li>Submit the number of copies of the TIA required by VDOT to the Hanover County Planning Department with your comprehensive plan amendment/rezoning/conditional use permit submittal.</li> <li>The Hanover County Planning Department will stamp "received" on all copies of the TIA, and will keep a copy for its files.</li> <li>The applicant will deliver the remaining copies of the TIA to VDOT and pay the necessary TIA review fee directly to VDOT.</li> </ol> </li></ul>
x	g. Community Meeting Guide (Please check if you have read and understand Pages 14 & 15.)
x	h. USPS Cluster Box Units (CBUs), please show the general location of USPS' Central Box Units (CBUs) along with elevations, access, parking and lighting, if provided. Please contact the local postmaster to obtain specific guidelines.
x	<ul> <li>i. For applications requiring plans, please submit thirteen (13) full-size plans, with sheets no larger than 24" x 36", folded to 9" x 12" in size, and one - 8 ½" x 11" reduction. MX Project Design Manuals no larger than 8.5" x 11"         Specific district requirements:         RS* - Conceptual plans that meet the requirements of Section 26-67 of the Zoning Ordinance.         RC - Existing Feature and Site Analysis plans that meet the requirements of Section 26-54(a).         RM* - Conceptual plans that meet the requirements of Section 26-84.         MX - Master Plan that meets the requirements of Section 26-93.         BP - Master Plan that meets the requirements of Section 26-157.     </li> <li>*RS and RM conceptual plans may also serve as the subdivision preliminary plat. In addition to the Zoning</li> </ul>
	Ordinance requirements noted above, the preliminary plat requirements in Section 25-25 of the Subdivision Ordinance must also be addressed.  Check here if the conceptual plan will serve as the preliminary plat.
	NOTE: When conceptual plans and/or elevations are requested by the Director of Planning which are larger in size than 8½" x 11" or are in color, please submit thirteen (13) full-size or colored plans, with sheets no larger than 24" x 36", folded to 9" x 12" in size, and one - 8½" x 11" reduction.

# ACKNOWLEDGEMENT OF APPLICATION FEE PAYMENT PROCEDURE Application fees are not accepted at the time of submittal. I hereby acknowledge that this application is not complete until the payment for all applicable application fees has been received by the Hanover County Planning Department. The Hanover County Planning Department shall notify me by mail, email and/or fax, (if selected below) of the applicable fee(s) at such time that they determine that the application is complete and acceptable. I acknowledge that I am responsible for ensuring that such fees are received by the Hanover County Planning Department by the Tuesday the week following the application deadline. I further acknowledge that any application fee submitted after this date shall result in the application being considered filed for the next application deadline. Should the applicable fees not be submitted within forty-five (45) days of the date of the notification letter, it shall be my responsibility to arrange for the retrieval of all application materials. The application and any supplementary materials for incomplete applications that are not retrieved within forty-five (45) days of the date of the notification letter shall be destroyed by the Hanover County Planning Department. Should my application be accepted, my fee payment will be due by \_\_\_\_\_\_\_. (To be filled in by a Planning Staff member.) Signature of applicant/authorized agent Print Name Todd D. Rogers Signature of applicant/authorized agent Print Name \_\_\_ Address to which notification letter is to be sent: 9245 Shady Grove Road, Suite 200 Mechanicsville, VA 23116 If you would like your letter emailed and/or faxed, please make selections, and provide the information below: Email NathenaH@HTRSI.com FEES Following application acceptance, make checks payable to Treasurer, Hanover County: A-1, OHP \$500 AR-6 (>2 lots), RC, RS, RM, MX \$1500 + \$75/acre\* for 1st 200 acres; \$30/acre\* for acreage>200 acres B, OS, M, BP \$1100 Amendment of Proffer or Planned Unit Development \$1500 \*Fractions of acreage are rounded up to the nearest whole number. Please note: Applicants which request tax-exempt status may have their application fee waived upon presentation of official documentation of such status.

FOR STAFF USE ONLY:	
Base Fee	
Acreage Fee	Accepted by:
TOTAL FEE	HTE #:

NOTIFICATION OF ADJOI PLANNING COMMISSION		BOARD OF SUPERVISORS, AND
Applicant's Statement:		
request, as well as the membe which the property is located. and/or municipal boundaries.	rs of the Board of Supervisors and Adjacent property includes all pro	to the property, which is the subject of this Planning Commission for the magisterial district in perty across roadways, watercourses, railroads, d addresses below are those of the adjacent r of Revenue of Hanover County.
Applicant's Signature:	200	
COMMONWEALTH OF VIRGIN	NIA >	MATHEMA ELIZABETH HADDRULL Motary Public
COUNTY OF HANOVER	) to-wit: )	Commonwealth of Virginia Registration No. 7866620 My Commission Expires Aug 31, 2024
The foregoing instrum	ent was acknowledged before m	te this 33 day of May, 2022, by (Name of Applicant).
My commission expires:	31-2004 J- Jeton 2/2	tary Public
Board of Supervisors Repres	sentative: R. Allen Davidson	V
Planning Commission Repre	sentative: Edmonia Iverson	
List of Adjacent Property Ow	ners: Check here if list is at	tached.
GPIN	Name	Address
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of Adjacent Property Ow	ners:	
GPIN	Name	Address
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GPIN	Name	Mailing Address
7880-63-9486	Riley & Helen Lowe Rev Trust	PO Box 175, Mechanicsville, VA 23111
7880-70-4924 & 7880-72-4067	Harris & Douglas Properties, LLC	7113 Three Chopt Road, Suite 209, Richmond, VA 23226
7880-71-9467 & 7880-80-6263	Harris & Douglas Properties, LLC	7113 Three Chopt Road, Suite 209, Richmond, VA 23226
7880-71-9482	Harris & Douglas Properties, LLC	7113 Three Chopt Road, Suite 209, Richmond, VA 23226
7880-72-5367	Hanover County	PO Box 470, Hanover, VA 23069
7880-72-7200	Joyce M. Corker Rev Trust	11101 Wychwood Dr, Mechanicsville, VA 23116
7880-72-7041	Kimberly Baber & Donald Matthews	13097 Hill Top Dr, Ashland, VA 23005
7880-71-6956	Kimberly Baber & Donald Matthews	13097 Hill Top Dr, Ashland, VA 23005
7880-71-6798	Christopher Culley	13083 Hill Top Drive, Ashland, VA 23005
7880-71-8619	Harold & Gary Dyson	10820 Hinton Court, Glen Allen, VA 23060
		10109 East Patrick Henry Road, Ashland, VA 23005
7880-81-0951	Lewis Hopkins	<u> </u>
7880-71-9959	James & Virginia Dyson	10115 East Patrick Henry Road, Ashland, VA 23005
7880-72-8161	Jolene Gonner	10125 East Patrick Henry Road, Ashland, VA 23005
7880-81-7146	Russell & Joanie Pond	10067 East Patrick Henry Road, Ashland, VA 23005
7890-00-0204	Harneet & Sarbjit Kaur Jaswal	9988 East Patrick Henry Road, Ashland, VA 23005
7890-01-1439	Ann & Herman Smith	28248 Signboard Rd, Ruther Glen, VA 22546
7890-01-0411	Grace Anderson	28248 Signboard Rd, Ruther Glen, VA 22546
7890-01-0503	Mary Graves & Dorothy Tyler	PO Box 1431, Ashland, VA 23005
7880-91-8500	Mary Cash	322 New Street, Ashland, VA 23005
7880-91-8516	James Tyler & Nora Bowles	3205 Aldersgate Drive, Richmond, VA 23223
7880-91-8617	Kelvin & Linda Holman	PO Box 119, Ashland, VA 23005
7890-01-0675	Melinda Spurlock	12473 Tarrers Ln, Ashland, VA 23005
7880-91-8737	Betty Burleigh	PO Box 944, Ashland, VA 23005
7890-01-0787	AAA Plumbing Co Inc.	PO Box 438, Lightfoot, VA 23090
7880-91-4455	Georgie L. Stephens Rev Trust	10034 East Patrick Henry Road, Ashland, VA 23005
7880-91-1671 & 7880-91-0618	Rose Moore	10052 East Patrick Henry Road, Ashland, VA 23005
7880-81-9731	Pam Moore & Angela Burney	10068 East Patrick Henry Road, Ashland, VA 23005
7880-81-8734	Berry Norris	11339 Flemings Ln, Rockville, VA 23146
7880-81-7757	Daniel Davis	10076 East Patrick Henry Road, Ashland, VA 23005
7890-23-4555	Ashley Farm LLC	9245 Shady Grove Rd, Suite 200, Mechanicsville, VA 23116
7880-93-6580	George & Carrie Winston	9544 Providence Church Road, Ashland, VA 23005
7880-93-6288	Augusta Tarrer-Reel	9534 Providence Church Road, Ashland, VA 23005
7880-93-6078 & 7880-92-4935	Jean Folley	10028 Providence Church Road, Ashland, VA 23005
7880-92-0864 & 7880-83-0625		7231 Forest Ave, Ste 102, Richmond, VA 23226
7880-72-6758	Billie Smith	19036 Hewlett Rd, Ruther Glen, VA 22546
		10845 Providence Woods Ln, Ashland, VA 23005
7880-72-9777	William & Janina Pearce	
7880-82-0788	Paul & Heather Agnello	10841 Providence Woods Ln, Ashland, VA 23005
7880-82-1860	Darryl & Ashleigh Puller	10837 Providence Woods Ln, Ashland, VA 23005
7880-82-2821	Josh & Morgan Merkel	10833 Providence Woods Ln, Ashland, VA 23005
7880-82-2882	Donna Rea	10829 Providence Woods Ln, Ashland, VA 23005
7880-82-3832	Jonathan Garner	10825 Providence Woods Ln, Ashland, VA 23005
7880-82-3893	Jason & Patricia Wood	10821 Providence Woods Ln, Ashland, VA 23005
7880-82-4864	Stephen & Kelly Ernest	10817 Providence Woods Ln, Ashland, VA 23005
7880-82-5835	Maria Bordera-Amerigo	10813 Providence Woods Ln, Ashland, VA 23005
7880-82-6806	Judith & Mary Obrien	10809 Providence Woods Ln, Ashland, VA 23005
7880-82-6877	Ronnie & Emilie Smithwick	10805 Providence Woods Ln, Ashland, VA 23005
7880-82-7838	Donald Rea	10801 Providence Woods Ln, Ashland, VA 23005
7880-82-7899	Sayak & Moumita Bhattacharya	10797 Providence Woods Ln, Ashland, VA 23005
7880-82-8950	Erkan & Nilay Yardimci	10793 Providence Woods Ln, Ashland, VA 23005
7880-82-9911	Keith & Ebony King	10789 Providence Woods Ln, Ashland, VA 23005
7880-82-9972	Ricardo Rodriguez & Ana Afanador	10785 Providence Woods Ln, Ashland, VA 23005
7880-92-0932	Samantha Sperry	10781 Providence Woods Ln, Ashland, VA 23005
7880-92-0993	John & Alison Denler	10777 Providence Woods Ln, Ashland, VA 23005
7880-92-1953	Lewis & Iris Vaughan	10773 Providence Woods Ln, Ashland, VA 23005
7880-92-2945	Keith & Angela Manion	10765 Providence Woods Ln, Ashland, VA 23005
7880-92-3906	Antoine & Crystal Fitzgerald	10761 Providence Woods Ln, Ashland, VA 23005
7890-00-5918	Laura Divine	36073 Malinda Ct, Round Hill, VA 20141
	Robert & Lesley Meisoll Rev Trust	10281 Brickerton Dr, Mechanicsville, VA 23116
7880-91-8174		
7890-04-3243	Providence Baptist Church	9491 Providence Church Rd, Ashland, VA 23005

7880-73-5067	David McCall & Fiona Murphy-McCall	13400 Dyson Trail Circle, Ashland, VA 23005
7880-73-6069		13404 Dyson Trail Circle, Ashland, VA 23005
7880-73-7127		13408 Dyson Trail Circle, Ashland, VA 23005
7880-73-6254	Frank Meyers & Sara Fields	13412 Dyson Trail Circle, Ashland, VA 23005
7880-73-6340	Chad Ratcliff & Jacqueline Donaghy	13416 Dyson Trail Circle, Ashland, VA 23005
7880-73-7238	Filippo Amato & Farwa Mateen	10733 Providence Park Dr. Ashland, VA 23005
	James & Alison Gurley	10739 Providence Park Dr, Ashland, VA 23005
7880-73-7299	· · · · · · _ ·	10725 Providence Park Or, Ashland, VA 23005
7880-73-8259	Ryan & Hope Riddeil Brian & June Sealy	10721 Providence Park Dr., Ashland, VA 23005
7880-73-9219		
7880-73-9279	Ryan Poznanczyk & Kourtney Davis	10717 Providence Park Dr. Ashland, VA 23005
7880-83-0239	Brandon & Sara Brugh	10713 Providence Park Dr, Ashland, VA 23005
7880-83-0392	Peter & Dana Hamm	10709 Providence Park Dr, Ashland, VA 23005
7880-83-1353	Ryan & Sarah Zylstra	10705 Providence Park Dr, Ashland, VA 23005
7880-83-2313	Taj Khan	10701 Providence Park Dr, Ashland, VA 23005
7880-83-1253	Brett & Sinath Wallace	13517 Providence Run Rd, Ashland, VA 23005
7880-83-1155	Joel & Heather Martin	13513 Providence Run Rd, Ashland, VA 23005
7880-83-1057	Corbin & Rochele Nelson	13509 Providence Run Rd, Ashland, VA 23005
7880-82-2969	Jordan & Joshua Price	10828 Providence Woods Ln, Ashland, VA 23005
7880-82-1998	Seth & Amanda Bontrager	10832 Providence Woods Ln, Ashland, VA 23005
7880-82-1937	Michael Peterson & Barbara Peterson-W	10836 Providence Woods Ln, Ashland, VA 23005
7880-82-0967	Bekele Defersha & Redet Shiferaw	10840 Providence Woods Ln, Ashland, VA 23005
7880-72-9987	Krista Park	10844 Providence Woods Ln, Ashland, VA 23005
7880-72-9837	Titanium Jubilee LLC	100 S 15th Ave, Hopewell, VA 23860
7880-73-5428	Steven & Alicia Talbot	10744 Providence Park Dr, Ashland, VA 23005
7880-73-5487	Kevin & Carmen Klaibor	10740 Providence Park Dr. Ashland, VA 23005
7880-73-6447	Wesley & Judith Austin	10736 Providence Park Dr. Ashland, VA 23005
7880-73-7408	Ross Almanza Living Trust	10732 Providence Park Dr., Ashland, VA 23005
7880-73-7459	<u> </u>	10728 Providence Park Dr, Ashland, VA 23005
7880-73-8428	Rashida & Gerrick Waters	10724 Providence Park Dr, Ashland, VA 23005
7880-73-8499	Christopher & Patricia Condon	10720 Providence Park Dr., Ashland, VA 23005
7880-83-0418	Steven Newcomb	10712 Providence Park Dr., Ashland, VA 23005
7880-83-0489	Debora Volk	10708 Providence Park Dr., Ashland, VA 23005
7880-83-1439		10704 Providence Park Dr., Ashland, VA 23005
	Barbara Lownes	10700 Providence Park Dr., Ashland, VA 23005
7880-83-2500		
7880-83-3264	Nicole Newville	13516 Providence Run Rd, Ashland, VA 23005
7880-83-3177	Thomas & Lore Lynch	13512 Providence Run Rd, Ashland, VA 23005
7880-83-3181	Timothy & Nicole Waer	13508 Providence Run Rd, Ashland, VA 23005
7880-83-3095	Frances Owens-Bell & Donald Bell	13504 Providence Run Rd, Ashland, VA 23005
7880-82-4918	Kranthi Gandham & Vuha Cheedalla	13500 Providence Run Rd, Ashland, VA 23005
7880-83-5020	William & Brandee Wilkins	13601 Providence Trail Cir, Ashland, VA 23005
7880-83-5016	Lillian Chenault	13605 Providence Trail Cir, Ashland, VA 23005
7880-83-5103	Ian & Nancy Wilson	13609 Providence Trail Cir, Ashland, VA 23005
7880-83-4199	Angela Lawson	13613 Providence Trail Cir, Ashland, VA 23005
7880-83-4286	Brian & Stephanie Flynn	13617 Providence Trail Cir, Ashland, VA 23005
7880-83-6364	Matthew & Monica Morris	13620 Providence Trail Cir, Ashland, VA 23005
7880-83-6298	Michael Denisar & Emily Withers	13616 Providence Trail Cir, Ashland, VA 23005
7880-83-6282	Rui Wang	13612 Providence Trail Cir, Ashland, VA 23005
7880-83-6195	Andrew & Judith D'Amato	13608 Providence Trail Cir, Ashland, VA 23005
7880-83-6099	Kakhramon & Aziza Talibdjanov	13604 Providence Trail Cir, Ashland, VA 23005
7880-83-7002	Felicia Smithers	13600 Providence Trail Cir, Ashland, VA 23005
7880-83-8007	Lisa Slade	10792 Providence Woods Ln, Ashland, VA 23005
7880-83-8068	Jerry & Irene Robertson	10788 Providence Woods Ln, Ashland, VA 23005
7880-83-9028	Bryan & Lindsey Voltz	10784 Providence Woods Ln, Ashland, VA 23005
7880-83-9089	Adam Caston & Alisa Emelianova	10780 Providence Woods Ln, Ashland, VA 23005
7880-93-0140	Curtis & Abby Vanbrackle	10776 Providence Woods Ln, Ashland, VA 23005
7880-93-1101	Eric Voltz	10772 Providence Woods Lt., Ashland, VA 23005
7880-93-0292	Lamar & Amy Adkins	10801 Parmele Pl, Ashland, VA 23005
7880-93-0222	Ebony Butler & Raymond Benton	10805 Parmele PI, Ashland, VA 23005
7000 92 0271	Tinn Induan 9. Innether Mandage	10000 Parmala DI Arbland VA 2200E
7880-83-9271 7880-83-8280	Tina Jackson & Jonathan Woodson Joscelyn Scarpello	10809 Parmele Pl, Ashland, VA 23005 10813 Parmele Pl, Ashland, VA 23005

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Christopher & Lucy Trapp	10817 Parmele Pl, Ashland, VA 23005
Kory & Tracey Mohr	10821 Parmele Pl, Ashland, VA 23005
Rudolph Benz & Denise Leffler-Benz	13634 Hewlett Trail Dr, Ashland, VA 23005
Katherina Koh	13630 Hewlett Trail Dr, Ashland, VA 23005
Usoro Udo & Kurston Anderson	13626 Hewlett Trail Dr, Ashland, VA 23005
David & Jordan Motsek	13622 Hewlett Trail Dr, Ashland, VA 23005
Rita D'Amico & Kathleen Kindle	13645 Hewlett Trail Dr, Ashland, VA 23005
Kristin Coilins & Melissa Ramsey	13649 Hewlett Trail Dr., Ashland, VA 23005
Elliot & Caitlin Shiben	13653 Hewlett Trail Dr, Ashland, VA 23005
George Maddox, Sr.	13654 Hewlett Trail Dr, Ashland, VA 23005
Christopher Thomas	13650 Hewlett Trail Dr, Ashland, VA 23005
Jae & Mi Cho	13646 Hewlett Trail Dr, Ashland, VA 23005
Samuel Kube & Angelica Masiero	13642 Hewlett Trail Dr, Ashland, VA 23005
Robert Monolo - Beaverdam supervisor	7516 County Complex Rd, Hanover, VA 23069
Edmonia Iverson - Beaverdam PC	7516 County Complex Rd, Hanover, VA 23069
	Kory & Tracey Mohr Rudolph Benz & Denise Leffler-Benz Katherina Koh Usoro Udo & Kurston Anderson David & Jordan Motsek Rita D'Amico & Kathleen Kindle Kristin Coilins & Melissa Ramsey Elliot & Caitlin Shiben George Maddox, Sr. Christopher Thomas Jae & Mi Cho Samuel Kube & Angelica Masiero Robert Monolo - Beaverdam supervisor

### NOTIFICATION OF ZONING APPLICATION SUBMITTAL

TO:	Adjacent Property Owner
FROM:	Hickory Hill II, LLC
DATE:	
The follow Departmen	ing application will be submitted for review to the Hanover County Planning
	ing ional Use Permit al Exception
Applicant:	Rogers-Chenault, Inc.
	ocation: 52.25 acres, generally south of Providence Church Road at its intersection with Route 54.
	ce Church Rd, Ashland, VA 23005
GPIN(s): 7	880-92-1335 (in part) & 7880-81-6802
Requested	Zoning District: RS
	Use/Exception: 150 Age-Restricted Single Family homes.

The application will be available for viewing at the Hanover County Planning Department. The Planning Department shall notify all adjacent property owners of the time, day, and place of the public hearings to be held on this application. Should you have any questions or comments, please contact the Planning Department at (804) 365-6171.

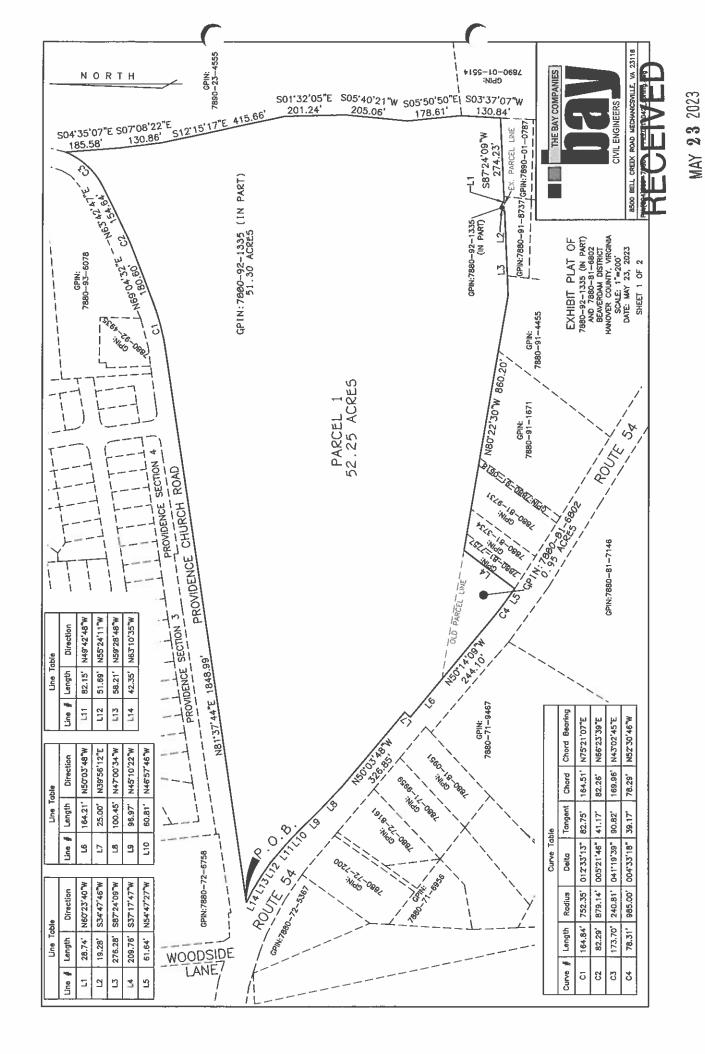
1. What is the General Land Use Plan Map designation for the subject property?  Commercial and Planned Business  2. What, if any, is the Major Thoroughfare Plan designation for the public road on which the subject property had Minor Arterial  3. Describe in detail the proposed use of the property.  Age-Restricted subdivision with 150 Single Family lots.	as frontage?					
Minor Arterial  3. Describe in detail the proposed use of the property.	as frontage?					
	2. What, if any, is the Major Thoroughfare Plan designation for the public road on which the subject property has frontage?  Minor Arterial					
Age-Restricted subdivision with 150 Single Family lots.						
List any sensitive environmental or unique features on the property. Are there any 150kV or greater transmit transmission lines for natural gas, other public utilities, or other entity? N/A	ission lines,					
5. Is the subject property located in a Dam Break Inundation Zone? Yes No (Please contact the Depa Planning or Public Works for assistance in addressing this question.) If yes, please contact the Department Works for further information.	artment of ent of Public					
RESPOND FOR RS AND RM REZONING APPLICATIONS: (Attach additional pages, if needed						
Have you provided a conceptual plan of the proposed development, including general lot configurations as	· I I I I I I I I I I I I I I I I I I I					
locations? Yes.  2. How does your proposal preserve or protect the existing trees on the property? If the property is treeless, proposal posts in provide trees on the property? The RS District require significant amount of on						
proposal contain provisions to provide trees on the property? The RS District require significant amount of op- space and amenity space. The Proposal provides for preservation of significant treed areas already existing on the						
and planting of buffers and landscaping throughout the project.	ie i topoit,					
3. Are recreational amenities being proposed for the project? If so, specify in detail the amenities planned. Gaz	zebo					
Pavilions, a fire pit, walking trails, sidewalks, RV/Trailer parking area, community gardens, a dog park, and a pond	d with a pier.					
4. What provisions will be made to ensure safe and adequate access to the subject property? (Note: A second access should be provided for any project in which there will be fifty-one (51) or more lots.)   Two means of a second access should be provided for any project in which there will be fifty-one (51) or more lots.)						

RESPOND FOR AR-6 and RC REZONING APPLICATIONS: (Attach additional pages, if needed)		
1.	For AR-6 rezoning requests: Have you provided a conceptual plan of the proposed development, including general lot configurations and road locations? Are the proposed lot sizes compatible with existing parcel sizes in the area?	
2.	How is the proposed subdivision compatible with the rural setting and sensitive to natural and cultural features?	
3.	What provisions will be made to ensure safe and adequate access to the subject property? (Note: A second means of access should be provided for any project in which there will be fifty-one (51) or more lots.)	
A C. S.	SPOND FOR B-1, B-2, B-3, BO, OS, M-1, M-2, M-3 REZONING APPLICATIONS: tach additional pages, if needed)	
1.	Has a conceptual plan of the proposed development been provided, showing proposed building locations, parking lots, entrances, and other features?	
2.	What provisions will be made to ensure safe and adequate access to the subject property?	
•		
3.	How will the traffic impact of this development be addressed?:	
4.	Describe why the proposed use is desirable and appropriate for the area. What measures will be taken to assure that the proposed use will not have a negative impact on the surrounding vicinity?	
5.	What type of signage is proposed for the site?	
6.	Have architectural/building elevations been submitted with this application?	

HISTO	ORIC SITE IMPACT ANALYSIS	
includ includ Shoul	le both structural and non-structural resources, such a le the GPIN (Tax Parcel Number) associated with the d you need assistance completing this form, please co	<u> </u>
1. H	listoric Resource/File No. N/A	GPIN
		GPIN
	have identified known or suspected historic resources lowing information on each site:	on the subject property or adjacent property, please provide
a)	Is the historic site listed as a National or State Regi	stered Landmark?
b)		
c)	on the identified historic resources with regard to noise, traffic,	
d)	Describe voluntary measures that will be undertake on the identified historic resources.	n to help mitigate the impact that the proposed use may have
	ural and non-structural resources, trenches, cemeterie	n the subject property or immediately adjacent, including s, and archeological sites, please sign and date.  Date: 5-23-23

# COMPLIANCE WITH VDOT & COUNTY TRAFFIC IMPACT ANALYSIS REQUIREMENTS

The following must be completed for all applications: The a projected daily trip generation of 836 vehicles per generation of 53 AM / 61 PM vehicles per hour, based on the st The 11th edition (latest edition) of the ITE Trip Generate determining the trip generation (Code Number 251 and	day and a site peak hour trip ipulations of 24 VAC 30-155.
Choose one of the two options below:	
I certify that this proposal <u>DOES NOT EXCEED</u> 380 veh require submittal of a Traffic Impact Analysis.	icle trips per day that would
I certify that this proposal <u>DOES EXCEED</u> 380 vehicle trip a Traffic Impact Analysis be submitted.	s per day that would require
Choose one of the two options below:	
I certify that this proposal DOES NOT MEET any of the V the Traffic Impact Analysis Regulations Administrative Guid would require a Traffic Impact Analysis to be submitted application.	delines (24 VAC 30-155) that
I certify that this proposal MEETS at least one of the VDO Traffic Impact Analysis Regulations Administrative Guide would require a Traffic Impact Analysis to be submitted application. A Traffic Impact Analysis, prepared in accord Analysis Regulations Administrative Guidelines (24 VAC) and will be submitted to VDOT the same day.	elines (24 VAC 30-155) that ed in conjunction with this ance with the Traffic Impact
Cal M/D	05-18-23
(Signature of Applicant/Applicant's Representative)	(Date)
Carl Hultgren, P.E., PTOE	
(Applicant/Applicant's Representative – Print Name)	
For questions regarding VDOT requirements:  Virginia Department of Transportation (VDOT) – Ashland Research Butler, Assistant Residency Administrator 523 North Washington Highway Ashland, VA 23005 Phone (804) 752-5511 Fax (804) 752-6431 Email: robert.butler@vdot.virginia.gov http://www.virginiadot.org/projects/chapter527/	<u>sidency</u>



### PARCEL 1: 52.25 ACRES

ALL THAT CERTAIN PIECE OR PARCEL OF LAND, LYING AND BEING IN THE BEAVERDAM MAGISTERIAL DISTRICT OF HANOVER COUNTY, VIRGINIA, SITUATED ALONG THE NORTHERN RIGHT-OF-WAY LINE OF ROUTE 54 AND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEGINNING AT A POINT (P.O.B.) ON THE NORTHERN LINE OF ROUTE 54 AT THE INTERSECTION OF ROUTE 54 AND PROVIDENCE CHURCH ROAD LABELED P.O.B. ON THIS PLAT:

```
THENCE N81°37'44"E 1848.99':
THENCE ALONG A CURVE TO THE LEFT WITH A RADIUS OF 752.35' A LENGTH OF 164.84';
THENCE N69'04'32"E 180.60';
THENCE ALONG A CURVE TO THE LEFT WITH A RADIUS OF 879.14' A LENGTH OF 82.29';
THENCE N63'42'47"E 154.64':
THENCE ALONG A CURVE TO THE LEFT WITH A RADIUS OF 240.81' A LENGTH OF 173.70';
THENCE S04'35'07"E 185.58';
THENCE S07'08'22"E 130.86';
THENCE S12'15'17"E 415.66';
THENCE S01'32'05"E 201.24';
THENCE S05'40'21"W 205.06';
THENCE S05'50'50"E 178.61';
THENCE S03'37'07"W 130.84';
THENCE S87'24'09"W 274.23';
THENCE N60'23'40"W 28.74':
THENCE S34'47'46"W 19.28':
THENCE S87'24'09"W 276.28';
THENCE N80'22'30"W 860.20';
THENCE S37'17'47"W 209.76';
THENCE N54'47'27"W 61.64';
THENCE ALONG A CURVE TO THE RIGHT WITH A RADIUS OF 985.00' A LENGTH OF 78.31';
THENCE N50'14'09"W 244.10':
THENCE N50'03'48"W 164.21':
THENCE N39'56'12"E 25.00';
THENCE N50'03'48"W 326.85';
THENCE N47'00'34"W 100.45';
THENCE N45'10'22"W 96.97';
THENCE N46'57'46"W 60.81'
THENCE N49'42'48"W 82.15';
THENCE N55'24'11"W 51.69';
THENCE N59'28'48"W 58.21';
THENCE N63'10'35"W 42.35' TO THE POINT AND PLACE OF BEGINNING CONTAINING
52.25 ACRES OF LAND.
```

EXHIBIT PLAT OF 7880-92-1335 (IN PART) AND 7880-81-6802 BEAVERDAM DISTRICT HANOVER COUNTY, VIRGINIA DATE: MAY 23, 2023

SHEET 2 OF 2



8500 BELL CREEK ROAD MECHANICSVILLE, VA 23116

PH:(804)569-7060 FILED:18042\_zoning.dwg

Postal Ve	erification of A	djacent Own	er Notifica	ations

## Confirmation Services Certification United States Postal Service® Company Information Address (Number, street, suite no., city, state and ZIP Code™) Company Name PO Box 470 COUNTY OF HANOVER Hanover VΑ Mailer Identification (MID) 23069 902800903 US The electronic file submitted by the company shown above has been certified by the National Customer Support Center (NCSC) to be complete and accurate in both content and transmission and to meet the requirements as defined in Publication 199, Intelligent Mail Package Barcode (IMpb) Implementation Guide for: Confirmation Services and Electronic Verification System (eVS) Mailers. **Date Signed Authorized Signature** Barcoses Labels The barcoded labels printed and submitted by the company shown above have been certified by the NCSC to meet the standards and specifications as prescribed in Publication 199 and the appropriate ANSI or AIM published standards. Date Signed Keep the original of this form in a safe place and provide a copy to your local USPS® facility if requested. Shipments included in Service Performance Measurement should submit a PS Form 3152 with each mailing. In the space below, place a CS1-128 barcode representing the Electronic File Number from the Header Record. If you cannot print the barcome, fill in the sequence numbers and check digit (the digits that follow the MID) from your Electronic File Number. This information is in the Header Record of the electronic file. IMPB barcodes are required for all tracking numbers effective 1/27/2013. Valid Application Identifiers (AI): IMpb barcode AI "92" uses a 9-digit Maller ID for commercial permit payment mallers. IMpb barcode "93" uses a 6-digit Maller ID for commercial permit payment mallers. IMpb barcode "94" is used for online and meter mallers. Legacy barcode AI "91" will continue to be accepted for a limited time with an approved exception or Place the barcode here or write the serial number and check digit of the electronic file in the spaces provided.

REZ2023-00000 Please return signed form to hallup hirly Planning Mailed 7/11/23

Confirmation Services Electronic File Number 9475014902800903007651

#### structions for Asceptance Employee

If mailings are presented under an authorized manifest mailing system, verify payment of postage and fees, where applicable, using standard sampling procedures for pieces with special services. In addition, check the barcode formatting for the following:

- 1. Horizontal bars above and below the barcode.
- 1. Horizontal bars above and below the barcode.
  2. Human-readable numbers below the barcode.
  3. Depending on the product used, the words "USPS TRACKING #". "USPS SIGNATURE TRACKING #", "USPS CERTIFIED MAIL".
  3. Depending on the product used, the words "USPS TRACKING #". "USPS SIGNATURE TRACKING #", "USPS CERTIFIED MAIL".
  4. For Electronic Verification System, (eVS) or "e-VS" should be included either before or after the service banner text.
  4. For Electronic Verification System, (eVS) or "e-VS" should be included either before or after the service banner text.
  4. For example: "eVS USPS TRACKING", "USPS TRACKING e-VS."

  For example: "eVS USPS TRACKING", "USPS TRACKING e-VS."

  Tracking #", "USPS TRACKING e-VS."

Date and Time of Mailing (if different from date of verification) Computerized PS Form 3152, July 2013 Facsimile

RECEIVED

JUL 1 3 2023

HANOVER COUNTY PLANNING OFFICE

JSPS Manifest Mailing	System				Page 1									
lailer's Name & Address		Permit Number		ļ	MAC Ver. Number									
OUNTY OF HANOVER					Transh	ream								
O Box 470	i	Sequence Number		Ì	Class of Mail									
Hanover, VA 23069		1-2779			Mixed									
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9414814902800903069265 8798	"HICKORY HILL II LLC		ERR	0.600	2.10			6 65						
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	HENRICO, VA 23229													
9414814902800903069272 8759	"ROGERS-CHENAULT	INC	ERR	0.800	2.10 4.16			6.85						
REZ2023-00003 BOS 7.26.2023	9245 SHADY GROVE R		c		4,10									
	MECHANICSVILLE, VA	23116												
9414814902800903069289	AAA PLUMBING CO INC	:	ERR	0.600	2.10			6 85						
8800 REZ2023-00003 BOS 7.26 2023	PD BOX 438.		C		4,15									
	LIGHTFOOT, VA 23090													
9-114814902800903059296 8801	agnello, Paul Thok	kas & Heather Rae	ERR	0.600	2.10			6.85						
REZ2023-00003 BOS 7.26.2023	10841 PROVIDENCE V	YOODS LANE.	Ç		4.15									
	ASHLAND, VA 23005													
9414814902800903069307	ASHLEY FARLULO			0.600	2.40			6.85						
8502 REZ2023-00003 BOS 7.26 2023	9745 SHADY GROVE	ROAD SUITE 200.	err C		2.10 4.15									
	MECHANICSVILLE, V	A 23116												
9414814902800903069319	BABER, KIMBERLY &	MATTHEWS, DONALD R/S	ERA	0.600	2.10			6.85						
8803 REZ2023-00003 BOS 7,26,2023	13097 HILL TOP DRIV	Æ,	С		4,15	i								
	ASHLAND, VA 23005													
Page Totals	6			3.60	37.	.50		41.1						
Cumulative Totals	•					.50		41.1						

JSPS Manifest Mailing	j System				Page 2									
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8804	MOUMITA R/S		ERR		2.10									
REZ2023-00003 BOS 7.26 2023	10797 PROVIDENCE W	OODS LANE	c		4.15									
	ASHLAND, VA 23005													
9414814902800903069333	BORDERA-AMERIGO	BZOL AIRAM		0.600				6.85						
6805	10513 PROVIDENCE V	UDOOS! ANE	ERR		2.10 4.15									
REZ2023-00003 BOS 7 26 2023		7007 LUE.	•											
	ASHLAND, VA 20005													
9414814902800903069340	BURLEIGH, BETTY RO	SE		0.600				6.85						
8505 REZ2023-00003 BOS 7, 26 2023	PO BOX 944.		ERR		2.10 4.15									
REZ2023-00003-BUS / 28-2023			3											
	ASHLAND, VA 23005													
9414814902800903069357	CORKER, JOYCE M.F	EVOCABLE TRUST		0.600				6.65						
8807			ERR		2.10 4.15									
REZ2023-00003 BOS 7.26.2023	11191 WYCHWOOD D	MIVE.												
	MECHANICSVILLE, V	A 23116												
9414814902800903049364	DAVIS DANIEL PAUL			0.600	2.10			6.85						
8805 REZ2023-00003 BOS 7.26.2023	10076 EAST PATRICE	CHENRY ROAD	ERR C		4.15									
	ASHLAND, VA 23005													
9414814907800903069071	DENLER, JOHN R & I	DENLER, ALISON C		0.600				6.85						
8809			ERR		2.10 4.15									
REZ2023-00003 BOS 7.26.2023	10777 PROVIDENCE	WOODS LANE,	C		4,13									
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Cumulative Totals	12			7,20	75.	00		82.2						

USPS Manifest Mailing		Page 3								
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9414814902800903069388	DIVINE LAURA RUSSO	)	ERŔ	0.600	2.10			6,65		
8810 REZ2023-00003 BOS 7.26 2023	13047 CEDON ROAD		C		4.15					
	ASHLAND, VA 23005									
\$41481490Z8009Q\$Q69395	DYSON JAKES O AR	I VIRGINIA H		0.600	2.10			6.85		
8811 REZ2023-00003 BOS 7,26.2923	10115 EAST PATRICK	HENRY ROAD	err C		4.15					
	ASHLAND, VA 23005									
9114814902800903069401	ERNEST, STEPHEN &	KELLY		0,600	2 10			6.86		
8812 REZ2023-00003 6OS 7.26 2023	10317 PROVIDENCE V	YOODS LANE,	ERR G		4.15					
	ASHLAND VA 23005									
9414814902800903069418 8813	FITZGERALD, ANTOII CRYSTAL	E PIERRE & FITZGERALD,	ERR	0.600	2.10			6.65		
REZ2023-00003 BOS 7.26-2023	10761 PROVIDENCE	WOODS LAHE,	Ç		4.15					
	ASHLAND, VA 23005									
9414814902800903069425	FOLLEY, JEAN WILSO	ON .		0.600	2-10			6.85		
8814 REZ2023-00003 BOS 7.26 2023	19028 PROVIDENCE	CHURCH RD.	ERR C		4,15					
	ASHLAND: VA 23005									
								6		
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USPS Manifest Mailing		Page 4							
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9414814902800903069432	FOLLY JEAN A	1	ERR	0.600	2.10			6.85	
REZ2023-00003 BOS 7 26 2023	10028 PROVIDENCE C	HURCH ROAD	C		4.15				
	ASHLAND, VA 23005								
9414814902800903069449	GARNER, JONATHAN			0.609				6 85	
6816 REZ2023-00003 BOS 7,26-2023	10825 PROVIDENCE V	OODS LANE.	ERR		2.10 4.15				
RE22023-00003 BOS 7.24-1923	ASHLAND, VA 23005		•						
9414814902800903069456	GONNER JOLENE F			0 600				6.85	
6817			EAR		2.10				
REZ7023-00003 BOS 7.26 2023	10125 EAST PATRICK	HENRY ROAD	¢		4,15				
	ASHLAND, VA 23005								
9414814902800903069463	HANOVER COUNTY			0.600				6.85	
818	*** **********		ERR		4.15				
REZ2023-00003 BOS 7:26 7023	FO BOX 470.  HANOVER, VA 23069								
	HARRIS & DOUGLAS	DRODERYIES II C		0.600				6.85	
9414814902800903069470 8819	HWW3 & OCOURS	PROPERTING, SEC	ERR		2.18				
REZ2023-00003 BOS 7:26:2023	7113 THREE CHOPT	ROAD STE 209	C		4,15				
	RICHMOND, VA 2322	•							
9414814902800903069487 8820	HICKORY HILL II LLC		ERR	0.600	2.10			6.85	
RE22023-00003 BOS 7.26-2023	6729 RUGGLES ROA	D,	C		4.15				
	HENRICO, VA 23229								
Page Totals	5			3.00	31.	25		34.25	
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JSPS Manifest Mailing	System	<u></u>	Page 5									
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PO Box 470		Sequence Number			Class of Mail							
Hanover, VA 23069		1-2779		-	Mixed	,						
Article #/ Piece ID Shipper Ref#	Addressee Name Delivery Address		ES Type	Postage	ES Fee	Insurance Amount	Due/ Sender	Total Charge				
9414814992800903069494 8821	HOPKINS, LEWIS O IR		ERR C	0.600	2.10 4.15			6.85				
REZ2023-00003 BOS 7.26.2023	10109 EAST PATRICK ASHLAND, VA 23005	HENRY ROAD.	C		4.10							
9414814902800903069500 8822	TE & KING, EBONY JETER	ERR	0.600	2.10 4.15			6.85					
REZ2023-00003 BOS 7,26,2023	YOODS LANE.	¢		4.10								
94148  4902800903069517 8823	LOWE, RILEY B REVO	CABLE TRUST & LOWE, HELEN	ERR	0,600	2.10			6 85				
REZ2023-00003 BOS 7.26 2023	PO BOX 175.		c		4.15							
	MECHANICSVILLE, V	. 23111										
9414814902800903069524 8824	MANION, KEITH L & L	MHON, ANGELA MICHELLE	ERR C	0.500	2.10 4.15			6,85				
REZ2023-00003 BOS 7.26.2023	ASHLAND, VA 23005	NOODS LANE.			,,,-							
9414814902800903069531	MERKEL JOSHUA I	WORGAN LEE	500	0.600	2.10			6.85				
8525 REZ2023-00003 BO\$ 7.26.2023	10833 PROVIDENCE	WOODS LANE.	ERR C		4.15							
	ASHLAND, VA 23005											
9414814902800903069548 8826	STEPHANIE L	TURNEY, ANGELA M & MOORE,	ERR	0.600	2.10			6.85				
REZ2023-00003-80\$-7,26-2023	10068 EAST PATRICE	HENRY ROAD.	C		4,15							
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Cumulative Totals 29				17,40	181	.25		198.0				

JSPS Manifest Mailing		Page 6											
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Hanover, VA 23069		1-2779											
Article #/ Piece ID Shipper Ret#	Addressee Name Delivery Address		E8 Type	Postage	ES Fee	Insurance Amount	Due/ Sender	Total Charge					
	ASHILAND, VA 23005												
9414814902800903069556	MOORE, ROSE			0.600	2.10			6.65					
8827 REZ2023-00003 BOS 7.26.2023	10052 EAST PATRICK	HENRY ROAD	ERR C		4.15								
	ASHLAND, VA 23005												
9414814902800903069562	HORRIS, BERRY			0.600	2.10			6 85					
8828 REZ2023-00003 BOS 7,28,2023	11339 FLEMINGS LAN	E	err C		2.10 4.15								
	ROCKVILLE, VA 23146	•											
9414814902800903069579	OBRIEN, JUDITH & OF	IRIEN MARY R/S		0.600				6.65					
8879 REZ2023-00023 BOS 7.26.2023	19809 PROVIDENCE	WOODS LANE	err C		2 10 4 15								
	ASHLAND, VA 23005												
9414814902800903069586	PEARCE WILLIAM F	I PEARCE JANINA V RIS		0.600				6 85					
8430	2587		ERR C		2.10 4.15								
REZ2023-00003 BOS 7:26:2923	ASHLAND, VA 23005	WOODS LANE.	·		115								
	, , , , , , , , , , , , , , , , , , , ,												
9414814902800903069593 8831	POND, D RUSSELL &	JOANIEA	ERR	0,600	2.10			6.85					
REZ2023-00003 BOS 7 26 2023	10067 EAST PATRICE	(HENRY ROAD	¢		4,15								
	ashland, va 23005												
Page Totals	6			3.60	37.5	50		41,10					
Cumulative Totals	35			21.00	218	.75		239.7					

USPS Manifest Mailing	Page 7												
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PO Box 470		Sequence Number		•	Class	of Mail							
Henover, VA 23069		1-2779			Mixed								
Article #/	Aridressee Name	<del>!</del>	ES		ES	Insurance	Due/	Total					
Piece ID Shipper Ref#	Delivery Address		Туре	Postage	Fee	Amount	Sender	Charge					
N414814902800903069609	PROVIDENCE HOLLEON	VINERS ASSOCIATION INC	ERR	0.600	2.10			6.85					
REZ2023-00003 BOS 7,26,2023	7231 FOREST AVENUE	i.e	C		4.15								
	FLICHMOND, VA 23226												
9414814902800903069616	PULLER: DARRYL C &	ASHLEIGH D		9.600				6.85					
6933 REZ2023-00003 BOS 7.26.2923	10917 PROVIDENCE W	OODS LANE,	ERR Ç		2.10 4.15								
	ASHLAND, VA 23005												
9414814902800903069623	REA. DONALD		ERR	0.600	2.10			6.85					
8834 REZ2023-00003 BOS 7-28-2923	10801 PROVIDENCE W	OODS LANE,	C		4.15								
	ASHLAND, VA 23005												
9414814902800903069630	REA, DONNA			0.600				6.65					
8835 REZ2023-00003 BOS 7.26.2023	18829 PROVIDENCE V	VOCOS LANE.	ERR C		4.15								
	ASHLAND, VA 23005												
9414814902800993069647		O MURLLO & AFANADOR, ANA		0,600				6.05					
8836 REZ2023-00003 BOS 7,26 2023	L BARAJAS R/S 10785 PROVIDENCE V	VOODS LANE.	ERA C		4.15								
	ASHLAND, VA 23005												
9414814902800903069654	SMITH, BILLUE T			0.500				5.85					
8837 REZ2023-00003 BOS 7.26 2023	19036 HEWLETT ROA	D.	ERR Ç		2,10 4.15								
	RUTHER GLEN, VA 22				507								
Page Totals	5			3.00	31.2	25		34.25					
Cumulative Totals		24.00	250	.00		274.0							

USPS Manifest Mailing	Page 8												
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O Box 470		Sequence Number			Class of Mail Mixed								
Hanover, VA 23069		1-2779											
Article #/ Piece ID Shipper Ref#	Addressee Name Delivery Address		ES Type	Postage	ES Fee	Insurance Amount	Due/ Sender	Total Charge					
9414814902800903069661 8838	SMITHWICK, RONNIE	& ENGLIFE	ERR	0.600	2.10			6,85					
REZ2023-00003 BOS 7.26 2023	OODS LANE	¢		4.15									
	ASHLAND, VA 23005			0.600				6.85					
9414814992803993069678 8839 REZ2073-40003 BOS 7.26 2923	10761 PROVIDENCE V	ANE MOON	ERR	0.600	2.10 4.15			6.65					
	ASHLAND: VA 23005												
9414814902800903089685	STEPHENS, GEORGIE	LEE REVOC TR 3/1/04		0.600				6.85					
8840 REZ2023-00003 BQS 7,26,2023	10034 EAST PATRICK	HENRY ROAD	ERR C		4.15								
	ASHLAND, VA 23005												
9414814902820903069692 6841	VAUGHAN, LEVAS C	VALIGHAN, IRIS N	ERR	0.500	2.10			6.65					
REZ2023-00003 BOS 7.26 2023	10773 PROVIDENCE S ASHLAND, VA 23005	WOODS LANE	¢		4.15								
		DIGIA		0.600				6.85					
9414814902800903069708 8842 R622023-00003 BOS 7.26.2023	10821 PROVIDENCE		EAR G	0.600	2.10 4,15			****					
	ASHLAND, VA 23005												
9414814902800903069715	YARQIMCI, ERKAN &	YARDINGI, NILAY		0.600	2 10			6.85					
88-13 REZ2023-00003 BOS 7.26 2023	6602 SATINWOOD DE	RIVE.	era Ç		4.15								
Page Totals	6			3.60	37,5			41,10					
Cumulative Totals	46			27.60	287	.50		315.1					

USPS Manifest	Mailing System				Page 9								
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Hanover, VA 23069		1-2779			Mixed								
Article #/ Piece ID Shipper Ref#	Addressee Name Delivery Address		ES Type	Postage	E8 Fee	Insurance Amount	Due/ Sender	Total Charge					

URBANA, MD 21704

					HANOVER
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OWN_NAME1  **HICKORY HILL II LLC  **ROGERS-CHENAULT INC  AAA PLUMBING CO INC  AGNELLO, PAUL THOMAS & HEATHER RAE  ASHLEY FARM LLC	BABER, KINBERLY & MATTHEWS, DONALD R/S BHATTACHARYA, SAYAK & BHATTACHARYA, MOUMITA R/S BORDERA-AMERIGO, MARIA JOSE	BURLEIGH, BETTY ROSE CORKER, JOYCE M REVOCABLE TRUST DAVIS, DANIEL PALII	DENLER, JOHN RE, REDISON C DIVINE, LAURA RUSSO  DIVINE, LAURA COLO 8, VIDEGINIA U	DTSON, JAMES OJR & VINGINIA H ERNEST, STEPHEN & KELLY FITZGERALD, ANTOINE PIERRE & FITZGERALD, CRYSTAL	FOLLY, JEAN WILSON FOLLY, JEAN A	GARNER, JONATHAN GONNER, JOLENE F	HANOVER COUNTY HARRIS & DOI IST AS PROPERTIES 11.C	HICKORY HILL II LLC	HOPKINS, LEWIS O JR KING KEITH CIEMENTE & KING ERONY IETER	LOWE, RILEY B REVOCABLE TRUST & LOWE, HELEN F REVOCABLE TRUS	MANION, KEITH L'& MANION, ANGELA MICHELLE MERKEL, JOSHUA & MORGAN LEE	MOORE, PAMELA & BURNEY, ANGELA M & MOORE, STEPHANIE L	MODRE, ROSE	NORRIS, BERRY	OBRIEN, JUDITH & OBRIEN, MARY R/S PEARCE WILLIAM E & PEARCE JAMINA V R/S	POND, D RUSSELL & JOANIE A	PROVIDENCE HOMEOWNERS ASSOCIATION INC	PULLER, DARRYL C & ASHLE GH D	REA, DONALD	REA, DONNA	RODRIGUEZ, RICARDO MURILLO & AFANADOR, ANA L'BARAJAS N'S	SMITHWICK, RONNIE & EMILIE	SPERRY, SAMANTHA	STEPHENS, GEORGIE LEE REVOC TR 3/1/04	VAUGHAN, LEWIS C & VAUGHAN, IRIS N	WOOD, JASON & PATRICIA YARDIMCI, ERKAN & YARDIMCI, NILAY
GPIN 7880-92-1335 7890-01-0787 7880-82-0788 7890-23-4555	7880-71-6956 7880-82-7899 7880-82-5835	7880-91-8737 7880-72-7200	7880-92-0993 7890-00-5918	7880-82-4864 7880-82-4864 7880-92-3906	7880-93-6078 7880-92-4935	7880-82-3832 7880-72-8161	7880-72-5367	7880-81-6802	7880-81-0951	7880-63-9486	7880-92-2945 7880-82-2821	7880-81-9731	7880 91 1671	7880-81-8734	7880-82-6806	7880-81-7146	7880-83-0625	7880-82-1860	7880-82-7838	7880-82-2882	7880-82-9972	7880-82-6877	7880-92-0932	7880-91-4455	7880-92-1953	7880-82-3893 7880-82-8950

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GPIN	OWN NAME1	OWN_NAME2	MAIL_ADDRESS	MAIL_CITY	MAIL_STATE	MAIL ZIP
7880-63-9486	LOWE, RILEY B REVOCABLE TRUST & LOWE, HELEN F REVOCABLE TRUS	<null></null>	PO BOX 175	MECHANICSVILLE	VA	23111
7880-71-6956	BABER, KIMBERLY & MATTHEWS, DONALD R/S	<null></null>	13097 HILL TOP DRIVE	ASHLAND	٧A	23005
7880-71-0467	HARRIC & DOUGLAS PROPERTIES 11C	<nul></nul>	7113 THREE CHOPT ROAD STE 209	RICHMOND	VA	23226
7880-71-9959	DOCON TAMES OF IR & VIRGINIA H	<nul></nul>	10115 EAST PATRICK HENRY ROAD	ASHLAND	٧A	23005
7909 Ct 0002	VANOVED COLINE	<n:n></n:n>	PO 80X 470	HANOVER	٧A	23069
7000-72-5307	SANTEL BELLEH	SIN	19036 HEWLETT ROAD	RUTHER GLEN	٧A	22546
70007 17 0005	COOKED LOVER M DOVOCADI E TRI IST	\[\]\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	11101 WYCHWOOD DRIVE	MECHANICSVILLE	٧A	23116
7880-72-7200	CONNER, JOTCE IN REVOCABLE INC.		10125 FAST PATRICK HENRY ROAD	ASHLAND	٧A	23005
/880-/2-8161	GONNEK, JOLENE F	Nully Nully	10946 DROWDENCE WOODS I ANE	ASHIAND	VA.	23005
7880-72-9777	PEARCE, WILLIAM F & PEARCE, JANINA V K/S	SNUELS SNIETS	10043 FACVIDENCE WOODS EAVE	ASHI AND	٧٨	23005
7880-81-0951	HOPKINS, LEWIS O JR	<nuii></nuii>	TOTOS EAST PAINICN REINAT NORD	COLONGE	٧٨	23229
7880-81-6802	HICKORY HILL II LLC	<nui!></nui!>	8/29 KUGGLES KUAD	ACT AND	47	23005
7880-81-7146	POND, D RUSSELL & JOANIE A	<nul></nul>	10067 EAST PATRICK HENRY ROAD	ASHLAND	¥ ;	2000
7880-81-7757	DAVIS, DANIEL PAUL	<null></null>	10076 EAST PATRICK HENRY ROAD	ASHLAND	VA.	23003
7880-81-8734	NORRIS, BERRY	<null></null>	11339 FLEMINGS LANE	ROCKVILLE	٧A	23146
7880-81-9731	MOORE, PAMELA & BURNEY, ANGELA M & MOORE, STEPHANIE L	<null></null>	10068 EAST PATRICK HENRY ROAD	ASHLAND	٧A	23005
7880-82-0788	AGNELLO, PAUL THOMAS & HEATHER RAE	<null></null>	10841 PROVIDENCE WOODS LANE	ASHLAND	٧A	23005
7880-82-1860	PULLER, DARRYL C & ASHLEIGH D	<null></null>	10837 PROVIDENCE WOODS LANE	ASHLAND	٧A	23005
7880-82-2821	MERKEL, JOSHUA & MORGAN LEE	<null></null>	10833 PROVIDENCE WOODS LANE	ASHLAND	VA	23005
7880-82-7887	RFA DONNA	<null></null>	10829 PROVIDENCE WOODS LANE	ASHLAND	VA	23005
7880-82-3832	GARNER LONATHAN	<no!></no!>	10825 PROVIDENCE WOODS LANE	ASHLAND	VA	23005
7000 60 2002	WOOD IASON & DATRICIA	<nrd></nrd>	10821 PROVIDENCE WOODS LANE	ASHLAND	٧A	23005
7000 00 4064	CONC. CLOOK & VEILS	ANG	10817 PROVIDENCE WOODS LANE	ASHLAND	VA	23005
1000-05-4004	CONTROL, DIETTEN & NEEL		10913 DROWING WOODS I ANE	ASHLAND	۸۸	23005
7880-82-5835	BORDERA-AMERICO, MARIA JOSE	Vivelly V	10000 DECAMPENCE WOODS LANE	ASHLAND	V.A	23005
7880-82-6806	OBRIEN, JUDITH & OBRIEN, MARY K/S	<iiin></iiin>	LOSON PROVIDENCE WOODS DANK	ONCH AND	V.V	23005
7880-82-6877	SMITHWICK, RONNIE & EMILIE	<null></null>	10805 PROVIDENCE WOODS LANE	ASHLAND	¥ ^ / ·	23003
7880-82-7838		<null></null>	10801 PROVIDENCE WOODS LANE	ASHLAND	٧٨ :	23005
7880-82-7899	BHATTACHARYA, SAYAK & BHATTACHARYA, MOUMITA R/S	<null></null>	10797 PROVIDENCE WOODS LANE	ASHLAND	۸۸	23005
7880-82-8950	YARDIMCI, ERKAN & YARDIMCI, NILAY	<null></null>	8602 SATINWOOD DRIVE	URBANA	MD	21704
7880-82-9911	KING, KEITH CLEMENTE & KING, EBONY JETER	<nul></nul>	10789 PROVIDENCE WOODS LANE	ASHLAND	VA	23005
7880-82-9972	RODRIGUEZ, RICARDO MURILLO & AFANADOR, ANA L BARAJAS R/S	<null></null>	10785 PROVIDENCE WOODS LANE	ASHLAND	٧A	23005
7880-83-0625	PROVIDENCE HOMEOWNERS ASSOCIATION INC	<nul></nul>	7231 FOREST AVENUE	RICHMOND	۸۸	23226
7880-91-0618	MOORE, ROSE	<nul></nul>	10052 EAST PATRICK HENRY ROAD	ASHLAND	VA	23005
7880-91-1671	MOORE, ROSE	<null></null>	10052 EAST PATRICK HENRY ROAD	ASHLAND	۸A	23005
7880-91-4455	STEPHENS, GEORGIE LEE REVOC TR 3/1/04	<null></null>	10034 EAST PATRICK HENRY ROAD	ASHLAND	۸A	23005
7880-91-8737	BURLEIGH, BETTY ROSE	<null></null>	PO BOX 944	ASHLAND	٧A	23005
7880-92-0864	PROVIDENCE HOMEOWNERS ASSOCIATION INC	<null></null>	7231 FOREST AVENUE	RICHMOND	٧A	23226
7880-92-0932	SPERRY, SAMANTHA	<null></null>	10781 PROVIDENCE WOODS LANE	ASHLAND	٧A	23005
7880-92-0993	DENLER, JOHN R & DENLER, ALISON C	<nul></nul>	10777 PROVIDENCE WOODS LANE	ASHLAND	٧A	23005
7880-92-1335	HICKORY HILL II LLC	<null></null>	496 ST ALBANS WAY	HENRICO	VA	23229
7880-92-1953	VALIGHAN LEWIS C.& VALIGHAN, IRIS N	<null></null>	10773 PROVIDENCE WOODS LANE	ASHLAND	VA	23005
7880-92-2945	MANION KEITH & MANION ANGELA MICHELE	<nri< td=""><td>10765 PROVIDENCE WOODS LANE</td><td>ASHLAND</td><td>VA</td><td>23005</td></nri<>	10765 PROVIDENCE WOODS LANE	ASHLAND	VA	23005
7880-92-3906	FITZGERALD, ANTOINE PIERRE & FITZGERALD, CRYSTAL	<nul></nul>	10761 PROVIDENCE WOODS LANE	ASHLAND	VA	23005
7880-92-4935	FOLLY. IFAN A	<nul></nul>	10028 PROVIDENCE CHURCH ROAD	ASHLAND	VA	23005
7880-93-6078	FOLLEY, JEAN WILSON	<null></null>	10028 PROVIDENCE CHURCH RD	ASHLAND	VA	23005
7890-00-5918	DIVINE LAURA RUSSO	<nul></nul>	13047 CEDON ROAD	ASHLAND	VA	23005
7890-01-0787	AAA PLUMBING CO INC	<null></null>	PO BOX 438	LIGHTFOOT	VA	23090
7890-23-4555	ASHLEY FARM LLC	<null></null>	9245 SHADY GROVE ROAD SUITE 200	MECHANICSVILLE	٧A	23116

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## **HANOVER COUNTY**

ESTABLISHED IN 1720
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DEPUTY DIRECTOR OF PLANNING

GRETCHEN W. BIERNOT CURRENT PLANNING MANAGER

DONNA BOWEN
PRINCIPAL PLANNER

ANDREW J. POMPEI PRINCIPAL PLANNER

C. JASON HAZELWOOD CODE COMPLIANCE SUPERVISOR

> ERIN BABER BUSINESS MANAGER

P. O. BOX 470 HANOVER, VIRGINIA 23069 PHONE 804-365-6171

## HANOVER COUNTY PLANNING OFFICE NOTICE TO APPLICANT

DATE: 07/06/2023

This is to inform you that YOUR ZONING REQUEST has been advertised for public hearing.

## REZ2023-00003, HICKORY HILL II, L.L.C.

Requests to rezone from A-1, Agricultural District, to RS(c), Single-Family Residential District with conditions, on GPINs 7880-92-1335(part) and 7880-81-6802, consisting of approximately 52.25 acres, and located on the north line of East Patrick Henry Road (State Route 54) at its intersection with Providence Church Road (State Route 662). The subject property is designated on the General Land Use Plan Map as Planned Business and Commercial. The proposed zoning amendment would allow the development of 150 single-family homes for a gross density of 2.87 dwelling units per acre. (PUBLIC HEARING) Magisterial District: Beaverdam

The Hanover County Board of Supervisors will hold a hearing on the following date and time:

## WEDNESDAY, JULY 26, 2023, AT 6:00 P.M.

The hearing will be held in the Board Room of the Hanover County Government Building (Administration Building), Hanover Courthouse, Hanover, Virginia. You or your representative should plan to be present at this hearing.

If you have questions on the request please contact the Planning Office or you may also go to <a href="https://www.hanovercounty.gov/697/Board-of-Supervisors-Upcoming-and-Archiv">https://www.hanovercounty.gov/697/Board-of-Supervisors-Upcoming-and-Archiv</a> for more information.

NOTE: YOU ARE REQUESTED TO RETURN THE SIGN(S) POSTED ON YOUR PROPERTY TO THE PLANNING OFFICE AT THE TIME OF YOUR <u>BOARD OF SUPERVISORS</u> PUBLIC HEARING.

#### BOARD OF SUPERVISORS

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SUSAN P. DIBBLE, VICE-CHAIR SOUTH ANNA DISTRICT

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BEAVERDAM DISTRICT

FAYE O. PRICHARD ASHLAND DISTRICT

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CURRENT PLANNING MANAGER

DONNA BOWEN
PRINCIPAL PLANNER

Andrew J. Pompei Principal Planner

C. JASON HAZELWOOD
CODE COMPLIANCE SUPERVISOR

ERIN BABER
BUSINESS MANAGER

P. O. BOX 470 HANOVER, VIRGINIA 23069 PHONE 804-365-6171

# HANOVER COUNTY PLANNING OFFICE NOTICE TO ADJACENT PROPERTY OWNER(S)

DATE: 07/06/2023

This is to inform you that a ZONING REQUEST has been filed on property adjacent to yours. A public hearing has been scheduled for the following request:

## REZ2023-00003, HICKORY HILL II, L.L.C.

Requests to rezone from A-1, Agricultural District, to RS(c), Single-Family Residential District with conditions, on GPINs 7880-92-1335(part) and 7880-81-6802, consisting of approximately 52.25 acres, and located on the north line of East Patrick Henry Road (State Route 54) at its intersection with Providence Church Road (State Route 662). The subject property is designated on the General Land Use Plan Map as Planned Business and Commercial. The proposed zoning amendment would allow the development of 150 single-family homes for a gross density of 2.87 dwelling units per acre. (PUBLIC HEARING) Magisterial District: Beaverdam

The Hanover County Board of Supervisors will hold a hearing on the following date and time:

## WEDNESDAY, JULY 26, 2023, AT 6:00 P.M.

The hearing will be held at 6:00 PM or as soon thereafter as the Board's consideration of the administrative agenda permits. The public hearing will be held in in the Board Room of the Hanover County Government Building (Administration Building), Hanover Courthouse, Hanover, Virginia. You are cordially invited to comment on this request either in person at the hearing or by writing to:

Hanover County Planning Office c/o Jo Ann Hunter, AICP Post Office Box 470 Hanover, Virginia 23069

Please be sure to include the case number in your correspondence. If you have questions on the request, please contact the Planning Office or you may also go to <a href="https://www.hanovercounty.gov/697/Board-of-Supervisors-Upcoming-and-Archiv">https://www.hanovercounty.gov/697/Board-of-Supervisors-Upcoming-and-Archiv</a> for more information.

Traffic

## **Traffic Impact Analysis**

# **Hickory Grove**

Hanover County, Virginia

May 2023

## Prepared for:

RCI Builders
9245 Shady Grove Road
Suite 200
Mechanicsville, VA 23116





## Prepared by:



Richmond, VA

Fairfax, VA

Alexandria, VA

Washington, DC

www.goroveslade.com

This document, together with the concepts and designs presented herein, as an instrument of services, is intended for the specific purpose and client for which it was prepared. Reuse of and improper reliance on this document without written authorization by Gorove/Slade Associates, Inc., shall be without liability to Gorove/Slade Associates, Inc.

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## **APPENDICES**

Appendix A - VDOT TIA Scope Form

Appendix B - Traffic Count Data

Appendix C - Synchro Output - Existing (2023) Conditions

Appendix D - Synchro Output - No-Build (2027) Conditions

Appendix E - Synchro Output - Build (2027) Conditions

Appendix F - Turn Lane Warrant Analysis

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## **Executive Summary**

RCI Builders is proposing to construct an age-restricted neighborhood in the northeast quadrant of the Route 54 at Providence Church Road intersection in Hanover County, Virginia. This Traffic Impact Analysis (TIA) was performed in accordance with Hanover County and Virginia Department of Transportation (VDOT) requirements.

The development plan includes up to 150 single-family detached age-restricted lots. The access plan includes realigning Providence Church Road to the east to improve the geometrics of the intersection, and one full-movement driveway on Route 54. If approved, the neighborhood is expected to be built and occupied in 2027.

The original TIA was dated June 2019, and was a Chapter 527 TIA because the proposed development plan generated over 9,500 daily trips. The revised TIA dated September 2021 removed the convenience store and fast-food restaurant, and replaced those uses with fast casual and quality restaurants, which resulted in 4,820 daily trips, which is below the VDOT threshold for a Chapter 527 TIA. The current plan generates just 836 daily trips – a reduction of 83% compared to the September 2021 TIA.

#### Site Location and Study Area

The proposed neighborhood is located in the northeast quadrant of the Route 54 at Providence Church Road intersection. Based on TIA scope coordination with the County and VDOT, the following scenarios were analyzed:

- Existing 2023 traffic conditions
- No-build 2027 traffic conditions
- Build 2027 traffic conditions
- Build 2027 traffic conditions with Phase 1 of the Iron Horse Business Park Trips

The weekday AM and PM peak hour were studied for the following intersections:

- · Route 54 at Hill Carter Parkway
- Route 54 at I-95 Southbound Ramps
- Route 54 at I-95 Northbound Ramps
- Route 54 at Woodside Lane
- Route 54 at Providence Church Road
- Route 54 at Meyers Park Drive
- Route 54 at Goddins Hill Road

#### Recommendations

All of the study intersections are expected to operate acceptably at build-out of the neighborhood.

The projected build-out traffic volumes do not warrant any turn lanes on Route 54 at realigned Providence Church Road or the proposed site driveway. However, the applicant is proposing the following improvements:

- Realign Providence Church Road approximately 580 feet to the east to improve the spacing from Woodside Lane
- Widen Route 54 to a three-lane section from Woodside Lane to Meyers Park Drive to provide eastbound left-turn lanes on Route 54 at realigned Providence Church Road and Meyers Park Drive.

Introduction

This report summarizes the findings of the TIA that was performed for the proposed age-restricted neighborhood in the northeast quadrant of the Route 54 at Providence Church Road intersection in Hanover County, Virginia. This TIA was performed in accordance with Hanover County and VDOT requirements.

## Scope of the Traffic Analysis

Based on the TIA scoping coordination with the County and VDOT, the study area includes the following intersections:

- Route 54 at Hill Carter Parkway
- Route 54 at I-95 Southbound Ramps
- Route 54 at I-95 Northbound Ramps
- Route 54 at Woodside Lane
- Route 54 at Providence Church Road
- Route 54 at Meyers Park Drive
- · Route 54 at Goddins Hill Road

Figure 1 shows the site location and study intersections.

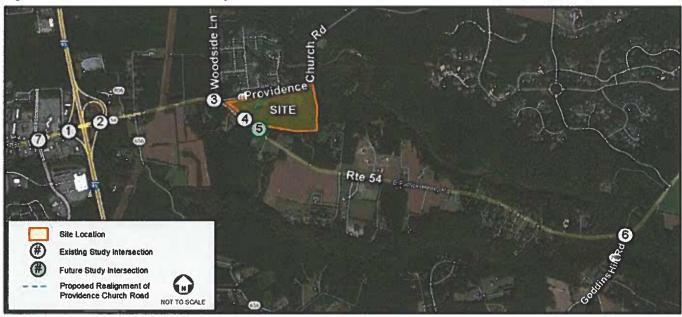


Figure 1: Site Location and Study Intersections

## **Existing Conditions**

### Existing Roadway Network

Existing lane configurations and turn lane storage lengths were collected in the field by Gorove Slade and signal timing information was provided by VDOT. A description of the major roadways within the study area is presented below in Table 1. The existing 2023 lane configurations at the study intersections are illustrated on Figure 2.

**Table 1: Existing Roadway Network** 

Roadway	RTE#	VDOT Classification	Legal/Design Speed Limit (mph)	AADT (vpd)
England Street/East Patrick Henry Road	54	Minor Arterial	35/55	6,000/4,300*
Woodside Lane	1055	Local	25	3,000
Goddins Hill Road	798	Major Collector	45	820*
Providence Church Road	662	Local	35	60*
*VDOT 2019 ADT Traffic Data				

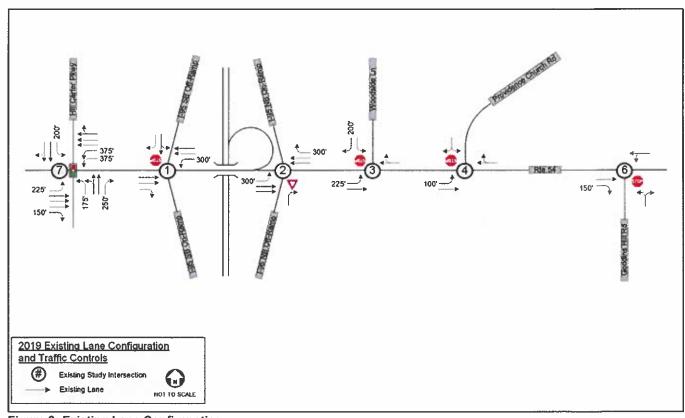


Figure 2: Existing Lane Configuration

#### Existing Traffic Volumes

The AM peak hour (7:00 to 9:00 AM) and PM peak hour (4:00 to 6:00 PM) turning movement counts were conducted at the following intersections by Peggy Malone & Associates, Inc. in May 2019:

- Route 54 at Hill Carter Parkway (from 2009 East Ashland TIA)
- Route 54 at I-95 Southbound Ramps
- Route 54 at I-95 Northbound Ramps
- Route 54 at Woodside Lane
- Route 54 at Providence Church Road
- Route 54 at Goddins Hill Road

These traffic counts were performed before the Covid-19 shutdowns started in March 2020. To calibrate the baseline traffic volumes, Burns Services, Inc. performed updated AM and PM peak hour turning movement counts at the following intersection on April 25, 2023:

Route 54 at Woodside Lane

Figure 3 shows that the total approach volume at this intersection has dropped by 3.1% in the AM peak hour since 2019 – a decrease of 0.8% per year. The total approach volume has increased by 14.3% in the PM peak hour since 2019 – an increase of 3.4% per year.

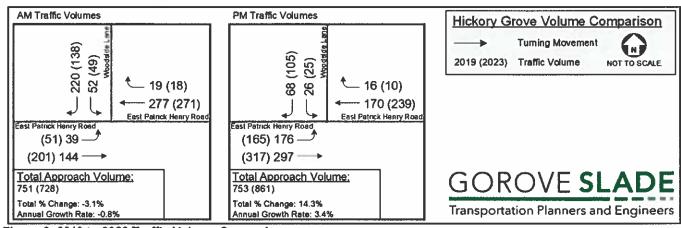


Figure 3: 2019 to 2023 Traffic Volume Comparison

Based on discussion with the County, the PM peak hour volumes have been adjusted up to match the April 2023 traffic volumes. Even though the AM peak hour volumes have dropped slightly since 2019, the AM peak hour traffic volumes were not adjusted down to be conservative.

The existing 2019 and 2023 peak hour traffic volumes for the study intersections are shown in Figure 4 and Figure 5, respectively. The turning movement count data are included in Appendix B.

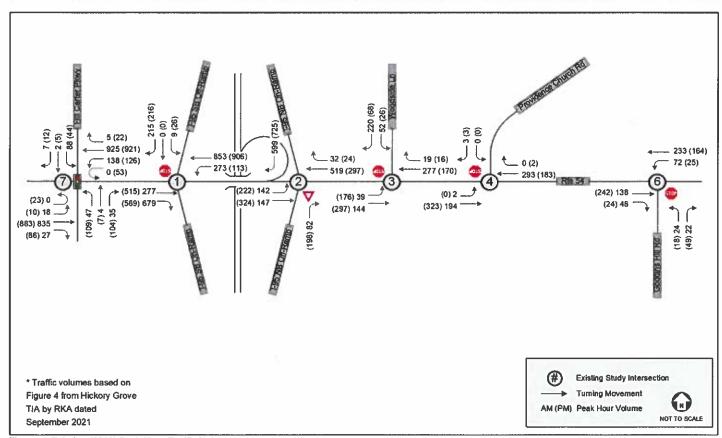


Figure 4: Existing (2019) Peak Hour Traffic Volumes

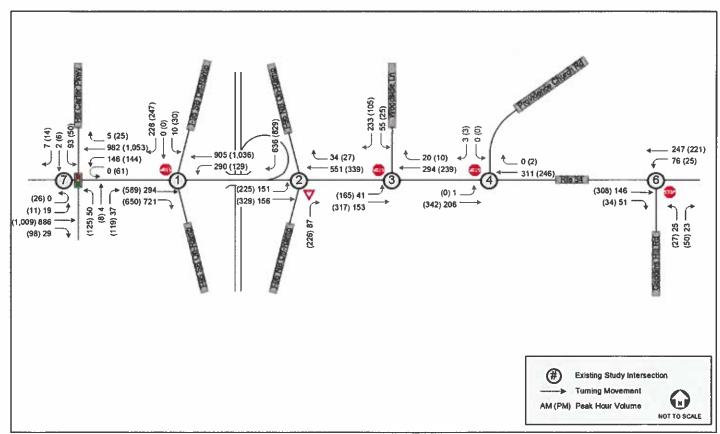


Figure 5: Existing (2023) Peak Hour Traffic Volumes

## No-Build Conditions

In order to determine the traffic impact of the proposed development, a comparison of the future conditions of the study intersections must be made. This is achieved by analyzing the horizon year (2027) with and without the traffic generated by the proposed neighborhood. The future year condition without the development is called the no-build condition, and it is determined by projected the existing traffic to the build-out year using an annualized growth rate and adding it the traffic from approved (but not yet built) developments in the study area.

#### Approved Development Traffic

Based on discussions with the County and VDOT, there are no approved developments in the vicinity of the site that need to be included in addition to the annual traffic growth rate.

An additional scenario was analyzed that includes the site trips and Phase 1 of the Iron Horse Business Park trips. The Iron Horse Business Park TIA was performed by Timmons Group in August 2022. Figure 6 shows the Iron Horse Business Park site trips.

#### Background Traffic Growth

Based on the TIA scope meeting with the County and VDOT, an annual traffic growth rate of 1.5% was applied to the existing traffic count volumes. The existing 2023 counts were increased by the annual growth rate for four years to estimate the 2027 no-build traffic volumes, which are shown in Figure 7.

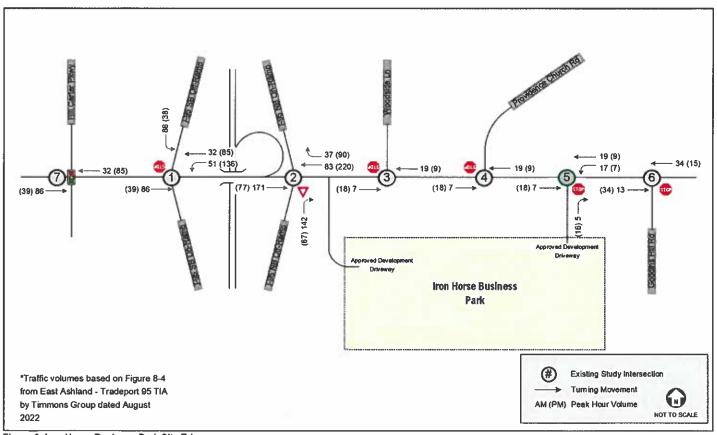


Figure 6: Iron Horse Business Park Site Trips

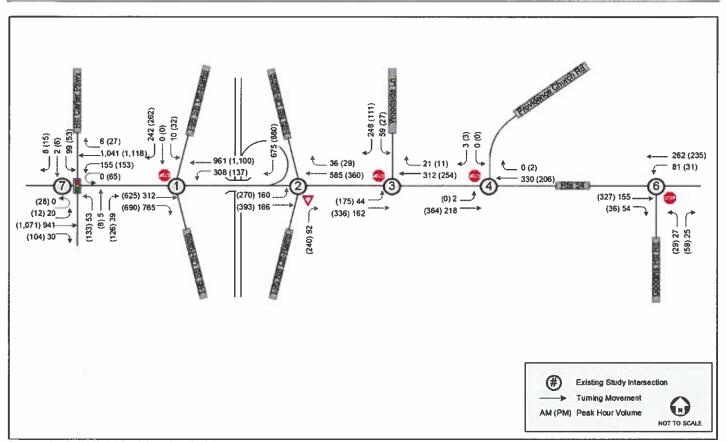


Figure 7: No-Build (2027) Peak Hour Traffic Volumes

## **Build Conditions**

Conditions were also analyzed with the recommended lane configurations and traffic control used to mitigate site trip impacts.

### Site Trip Generation

The Institute of Transportation Engineer's (ITE) *Trip Generation Manual, 11th Edition* was used to estimate the trip potential of the proposed neighborhood during the weekday AM and PM peak hours. Table 2 shows the trip generation potential of the project.

Table 2: Hickory Grove - ITE Trip Generation - Typical Weekday - 11th edition

Land Use (ITE Land Use Code)	Size	Averag Tra (vr	ffic	AM Pea (vp		PM Pea (vp		
		Enter	Exit	Enter	Exit	Enter	Exit	
Senior Adult Housing – Single Family (251)	150 lots	418	418	17	36	37	24	
February 2020 TIA	4,793	4,793	435	409	420	425		
% Reduction from February 202	0 TIA	-91%		-94%		-93%		
September 2021 TIA		2,410	2,410	47	72	195	147	
% Reduction from September 20	21 TIA	-83	3%	-55	5%	-82	%	

The trip potential of the neighborhood was estimated based on the ITE trip rates and equations published in Volume 3 of the 11<sup>th</sup> edition of the ITE *Trip Generation Manual* on Pages 405-407.

For land use 251, ITE publishes an average trip rate per unit, and a fitted curve equation to predict the trip potential. The trip potential of the project was estimated based on the average rate and the fitted curve equation, and the higher results were used to be conservative.

As shown in Table 2, the trip potential of the current plan is a small fraction of the previous TIA's because all of the commercial uses have been removed.

## Site Trip Distribution

The site trip distribution was determined based on discussions with the County and VDOT, a review of existing traffic patterns, surrounding land uses, and engineering judgement:

- 40% to / from the south on I-95
- 25% to / from the north on I-95
- 15% to / from the west on Route 54
- 15% to / from the east on Route 54
- 5% to / from the north on Woodside Lane

The site trip distribution and assignment are shown in Figure 8 and Figure 9, respectively.

#### Build 2027 Traffic Volumes

The build (2027) traffic volumes were determined by adding the no-build 2027 volumes (Figure 7) and the site trips (Figure 8) to estimate the projected build 2027 peak hour traffic volumes which are shown in Figure 10.

#### Build 2027 with Iron Horse Business Park Traffic Volumes

The build (2027) with Iron Horse Business Park trips were determined by adding the build traffic volumes (Figure 10) with the Iron Horse Business Park Trips (Figure 6), which are shown in Figure 11.

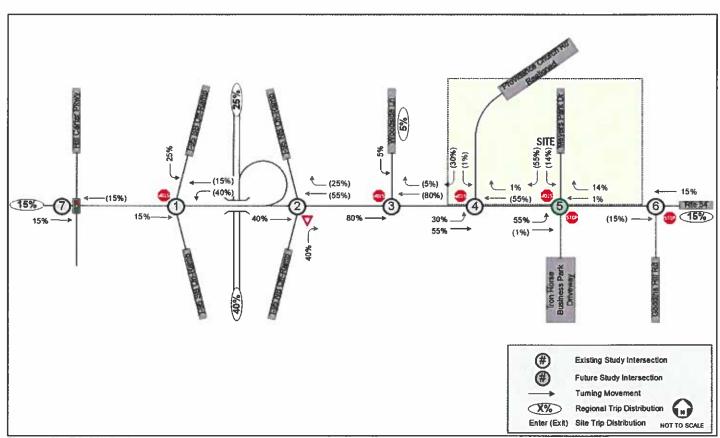


Figure 8: Site Trip Distribution

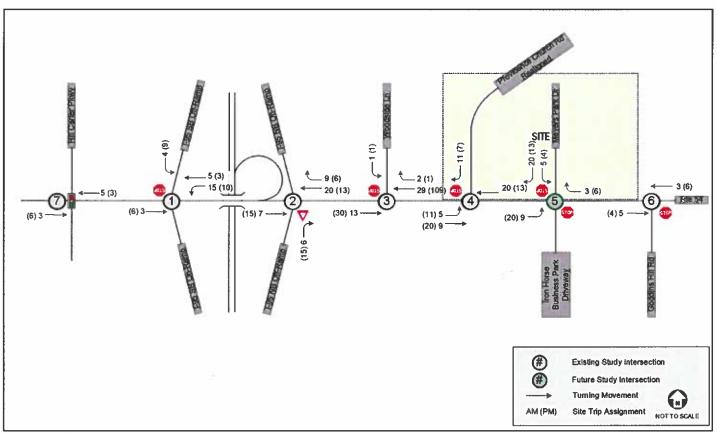


Figure 9: Site Trip Assignment

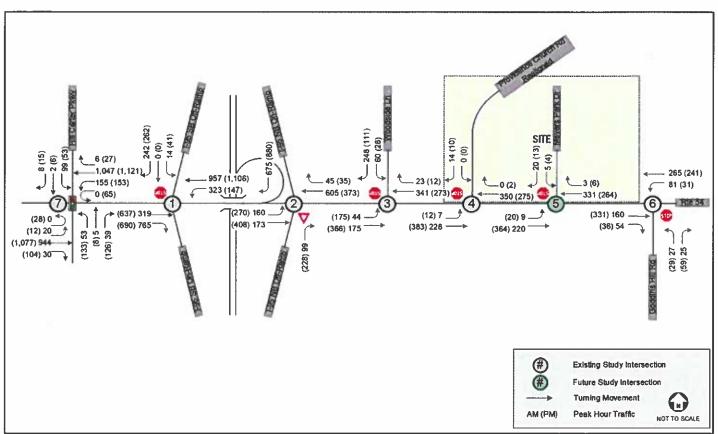


Figure 10: Build (2027) Peak Hour Traffic Volumes

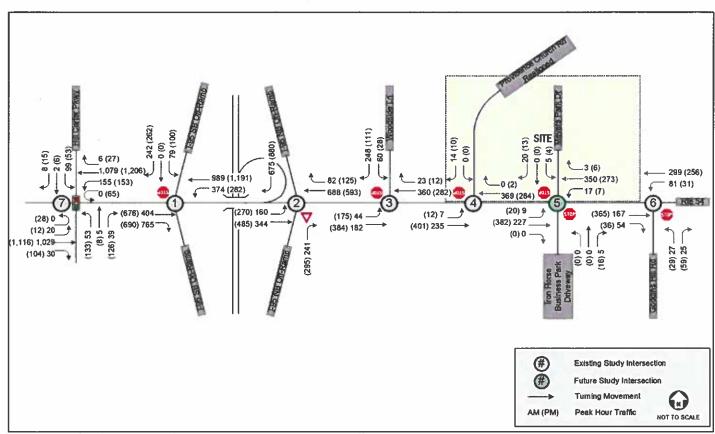


Figure 11: Build (2027) with Phase 1 of Iron Horse Business Park Peak Hour Traffic Volumes

## **Traffic Capacity Analysis**

### Capacity Analysis Procedure

Capacity analyses were performed at the study intersections during the weekday AM and PM peak hours. Synchro Version 11 was used to analyze the study intersections based on the Highway Capacity Manual (HCM) methodology and include level of service, delay, and queue length comparisons for the turning movements analyzed. For the purpose of this analysis, a peak hour factor of 0.92 was used for each of the study intersections only if existing PHF was less than 0.92. Otherwise, existing PHF was used. The Synchro and SimTraffic outputs are included in the Appendix.

#### Capacity Analysis Results

For unsignalized intersections, the average delays for the minor street left-turn movements are described as short delays (less than 25 seconds), moderate delays (between 25 and 50 seconds), and long delays (greater than 50 seconds). It is common for side street movements and left turns to experience long delays during the peak hours at intersections with major thoroughfares. Capacity analysis results are summarized in Table 3 through Table 9.

Table 3: Capacity Analysis Results - Route 54 at Hill Carter Parkway

Condition		Lane			AM Peak Hour				PM Peak Hour				
	Lane Group	Storage (ft.)	LOS	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max. Queue (ft.)	Overall LOS	LOS	Delay (sec)	Queue (It.)	SimTratfic Avg. Max. Queue (ft.)	Overall LOS	
	EBL	225	E	62.9	42	60		E	56.3	65	90		
	EBT	-/-/-	В	14.9	200	327		В	17.1	262	256		
	EBR	150	В	12	0	124		В	13.7	19	137		
Eviatina (2022)	WBL	375/375	D	53.4	94	104	В	D	53.5	123	115	С	
Existing (2023)	WBT/R	-/-/-	В	11.1	206	162		В	13.7	257	186		
Conditions	NBL	250	E	56	56	15	(19.6 Sec)	D	54.3	105	126	(23.0 Sec)	
	NBL/T	250	D	54.2	29	102		D	51.4	55	157		
	NBR SBL	250 200	D E	53.5 54.6	0 135	56 156		E	50.3 56.7	52 81	80 105		
	SBT/R	-/-	- D	49.1	133	32		D	55.5	16	37		
	EBL	225	D	51.5	38	65		E	56.3	69	92		
	EBT	-l-l-	В	16.9	208	331		В	18.0	287	322		
	EBR	150	В	13.3	0	170		В	14.2	24	249		
	WBL	375/375	Ď	43.3	83	109		Ď	53.4	129	141		
No-Build (2027)	WBT/R	-l-l-	В	12.7	215	188	В	В	14.3	283	216	С	
Conditions	NBL	250	Ď	45.5	49	23	(19.5 Sec)	Ď	54.2	109	138	(23.6 Sec)	
Conditions	NBL/T	-	Ď	44.2	27	98	(10.0 000)	Ď	51.2	58	174	(20.0 000)	
	NBR	250	Ď	43.6	0	58		Ď	50.1	59	115		
	SBL	200	Ď	48.8	123	153		E	56.9	86	111		
	SBT/R	-/-	Ď	40.4	0	31		Ē	52.3	16	42		
	EBL	225	D	51.5	38	64		E	56.3	69	86		
	EBT	-/-/-	В	16.9	208	357		В	18.1	289	333		
	EBR	150	В	13.3	0	212		В	14.2	24	215		
	WBL	375/375	D	43.3	83	114		D	53.4	129	162		
Build (2027)	WBT/R	-1-1-	В	12.7	217	185	В	В	14.4	283	230	С	
Conditions	NBL	250	D	45.5	49	5	(19.5 Sec)	D	54.2	109	146	(23.6 Sec)	
	NBL/T	-	D	44.2	27	93		D	51.2	58	178		
	NBR	250	D	43.6	0	58		D	50.1	59	119		
	SBL	200	D	48.8	123	152		Ε	56.9	86	109		
	SBT/R	-/-	D	40.4	0	36		D	52.3	16	46		
	EBL	225	D	51.5	38	56		Ε	56.3	69	94		
	EBT	-/-/-	В	17.4	230	335		В	18.3	302	344		
D. 312 (0007)	EBR	150	В	13.3	0	162		В	14.2	24	250		
Build (2027)	WBL	375/375	D	46.9	88	106	_	D	48.9	131	157		
Conditions	WBT/R	-/-/-	Α	9.9	197	196	В	В	19.1	388	236	С	
with Iron Horse	NBL	250	D	45.5	49	8	(18.6 Sec)	D	54.2	109	144	(24.9 Sec)	
Business Park Trips	NBL/T	-	D	44.2	27	98		D	51.2	58	190		
	NBR	250	D	43.6	0	61		D	50.1	59	110		
	SBL	200	D	48.8	123	141		Ε	56.9	86	116		
	SBT/R	-/	D	40.4	0	32		Ε	52.3	16	34		

Based on the capacity analysis, the intersection currently operates at LOS B during the AM peak hour and LOS C during the PM peak hour. Under no-build conditions, the intersection is expected to continue to operate at LOS B during the AM peak hour and LOS C during the PM peak hour.

Under build conditions, the intersection is expected to continue to operate at LOS B during the AM peak hour and LOS C during the PM peak hour, with all movements operating at LOS E or better.

No improvements are warranted or recommended at this intersection at build-out of the proposed neighborhood.

Under build conditions with Phase 1 of the Iron Horse Business Park trips, the intersection is expected to continue to operate at LOS B during the AM peak hour and LOS C during the PM peak hour, with all movements operating at LOS E or better.

Table 4: Capacity Analysis Results - Route 54 at I-95 Southbound Ramps

	MILL STATE OF THE	Lane			AM Pea	k Hour				PM P	eak Hour	
Condition	Lane Group	Storage (ft.)	LOS	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max Queue (ft.)	Overall LOS	LOS	Delay (sec)	Queve {ft.}	SimTraffic Avg. Max. Queue (ft.)	Overall LOS
	EBT	-/-	-	-		-		-	-	-	-	
Existing (2023)	WBL	300	Α	8.9	25	97		Α	9.5	13	79	
Conditions	WBT	-/-	-	-	-	-	N/A	-	-	-	-	N/A
Conditions	SBL/T	-	F	84.0	18	42		F	96.4	48	104	
	SBR	-	-	-	-	-				_	-	
	EBT	-/-	-	(9	75	-		-	-	-	-	
lo-Build (2027)	WBL	300	Α	9.0	28	121		Α	9.7	15	85	
	WBT	-/-	-	-	7.0	-	N/A	-	-	-	-	N/A
Conditions	SBL/T	-	F	105.6	20	49		F	136.9	63	109	
	SBR	_	-	-	-	-		-	-	-	-	
	EBT	-/-	-	-	-	-		-	-	-	-	
D. 314 (2027)	WBL	300	Α	9.2	30	209		Α	9.8	15	126	
Build (2027)	WBT	-/-	7:	.5	-	35	N/A	-	-	-	-	N/A
Conditions	SBL/T	-	F	128.2	33	53		F	197.5	90	152	
	SBR	_	-	-	- =	-		-	-	-	-	
Build (2027)	EBT	-/-	В	14.0	60	181		С	34.9	376	196	
	WBL	300	D	43.9	319	354	•	E	56.5	304	232	
with Iron Horse	WBT	-/-	Α	6.0	214	584	C (20.7.0)	Α	8.4	318	398	C (27.0.0
Business Park	SBL/T	-	D	39.0	104	150	/20.7 Sect	D	42.7	99	137	(27.9 Sec
Trips	SBR	_	D	48.7	160	101		Ε	63.4	243	80	

Based on the capacity analysis, the minor street left-turn movement currently operates with long delays during the AM and PM peak hours. Under no-build conditions, the minor street left-turn movement is expected to continue to operate with long delays during the AM and PM peak hours.

Under build conditions, the minor street left-turn movement is expected to continue to operate with long delays during the AM and PM peak hours. Long delays are common for left-turn movements at intersections with major thoroughfares. The trip potential of the Hickory Grove project will not meet any Manual on Uniform Traffic Control Devices (MUTCD) traffic signal warrants. No improvements are warranted or recommended at this intersection at build-out of the proposed neighborhood.

Under build conditions with Phase 1 of the Iron Horse Business Park trips, the intersection is expected to be signalized or converted to an alternative configuration approved by VDOT as part of the Iron Horse Business Park improvements. If a conventional traffic signal is installed, the intersection is expected to operate at LOS C during the AM and PM peak hours.

Table 5: Capacity Analysis Results - Route 54 at I-95 Northbound Ramps

		Lane		AM I	Peak Hou	r		PM	Peak Ho	ur
Condition	Lane Group	Storage (ft.)	LOS	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max. Queue (ft.)	LOS	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max. Queue (ft.)
	EBL.	300	С	20.1	48	116	С	17.3	65	140
Existing (2023)	EBT	-/-	6. <del>-</del> 5	-	-	¥	-	-	-	-
Conditions	WBT	-/-	-	-	-	-	-	-	-	-
Conditions	WBR	300	-	-	-	-	-	-	-	-
	NBR	-	-	-	-	-	-	-	-	-
	EBL	300	С	22.5	58	109	С	18.9	78	145
No-Build (2027)	EBT	-/-	-	-	-	-	-	-	-	-
Conditions	WBT	-/-	-	-	-	-	-	-	-	-
CONDITIONS	WBR	300	-	-	-	-	-	-	7	-
	NBR	-	-	-	-	-	-	-	-	-
	EBL	300	С	23.5	63	146	С	19.6	83	158
Build (2027)	EBT	-/-	-	-	-	-	-	-	-	-
Conditions	WBT	-/-	-	-	-	-	-	-	-	-
Conditions	WBR	300	-	-	-	-	-	-	-	-
	NBR	-	-	-	-	-	-	-	-	- 0
Build (2027) Conditions	EBL	300	D	29.2	78	158	Ε	42.3	165	175
	EBT	-/-	-	-	-	-	-	-	-	-
	WBT	-/-	-	-	-	-	-	-	-	-
	WBR	300	-	-	-	4	-	-	2	_
	NBR	_	_	-	-	_	_	-	2	-

Based on the capacity analysis, the major street left-turn movement is expected to operate with short delays during the AM and PM peak hours. Under no-build conditions, the major street left-turn movement is expected to continue to operate with short delays during the AM and PM peak hours.

Under build conditions, the major street left-turn movement is expected to continue to operate with short delays during the AM and PM peak hours. No improvements are warranted or recommended at this intersection at build-out of the proposed neighborhood.

Under build conditions, with Phase 1 of the Iron Horse Business Park trips, the major street left-turn movement is expected to operate with moderate delays during the AM and PM peak hours.

Table 6: Capacity Analysis Results - Route 54 at Woodside Lane

		Lane		AM F	eak Hou	r		PM	Peak Ho	ur
Condition	Lane Group	Storage (ft.)	LOS	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max. Queue (ft.)	LOS	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max. Queue (ft.)
	EBL	225	Α	8.1	3	7	Α	8.3	13	21
Existing (2023)	EBT	-	_	-	-	-	-	-		-
Conditions	WBT/R	-	-	-	-		-	-	-	-
Conditions	SBL	2	В	14.9	13	55	С	24.0	10	33
	SBR	200	В	13.4	45	76	В	10.6	15	58
	EBL	225	Α	8.2	3	6	Α	8.4	15	18
No-Build (2027)	EBT	-	-	-	-	-	_	-	-	-
	WBT/R	_	-	-	_	-	_	_	-	-
Conditions	SBL	-	С	15.7	15	60	D	26.7	13	51
	SBR	200	В	14.2	53	71	В	10.9	15	63
	EBL	225	Α	8.3	3	37	Α	8.5	15	72
D (14 (0007)	EBT	-	-	-	-	-	-	-	-	-
Build (2027)	WBT/R	-	_	_	_	_	_	_	_	_
Conditions	SBL	-	С	16.7	18	69	D	29.1	15	51
	SBR	200	В	14.9	58	142	В	11.1	15	68
D.::LI (0007)	EBL	225	Α	8.4	3	48	Α	8.5	15	89
Conditions	EBT	-	_	_	_	-	_	-	-	-
	WBT/R	-	_	_	-	-	-	-	_	-
with Iron Horse	SBL	_	С	17.3	18	72	D	30.3	18	46
	SBR	200	Ċ	15.4	60	126	В	11.2	18	68

Based on the capacity analysis, the minor street left-turn movement is expected to operate with short delays during the AM and PM peak hours. Under no-build conditions, the minor street left-turn movement is expected to operate with short delays during the AM peak hour and moderate delays during the PM peak hour.

Under build conditions, the minor street left-turn movement is expected to continue to operate with short delays in the AM peak hour and moderate delays during the PM peak hour. No improvements are warranted or recommended at this intersection at build-out of the proposed neighborhood.

Under build conditions, with Phase 1 of the Iron Horse Business Park trips, the minor street left-turn movement is expected to continue to operate with short delays during the AM peak hour and moderate delays during the PM peak hour.

Table 7: Capacity Analysis Results - Route 54 at Providence Church Road

		Lane		AM I	Peak Hou	ır		PM	Peak Ho	ur
Condition	Lane Group	Storage (ft.)	LOS	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max. Queue (ft.)	LOS	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max, Queue (ft.)
	EBL	100	Α	8.0	0	0	Α	7.8	0	0
Eviatina (2002) Canditiana	EBT	-	-	_	_	-	-	-	-	_
Existing (2023) Conditions	WBT/R	-	-	-	-	-	_	_	_	-
	SBL/R	-	В	10.9	0	4	В	10.8	0	4
	EBL	100	Α	8.1	0	0	Α	7.9	0	0
N. B. S. L. (OCOTA C. P.)	EBT	•	-	_	_	-		•		
No-Build (2027) Conditions	WBT/R	_	-	_	_	_	-	_	-	_
	SBL/R	-	В	11.1	0	3	В	11.0	0	4
	EBL	100	Α	8.1	0	24	A	7.9	0	18
Build (2027) Conditions	EBT WBT/R	-	-	-	-	-	=	-	-	
Build (2027) Conditions with Iron Horse Business Park Trips	SBL/R	_	В	10.9	3	29	В	10.5	3	33
	EBL	100	A	8.2	Ő	26	Ā	8.0	Ö	24
	EBT	-	-	-	-	-	-	-	-	-
	WBT/R	-	-	-	-	-	•		<b></b>	**
	SBL/R	-	В	11.1	3	31	В	10.6	3	28

Capacity analysis indicates that the minor street left-turn movement currently operates with short delays during the AM and PM peak hours. Under no-build conditions, the minor street left-turn movement is expected to continue to operate with short delays during the AM and PM peak hours.

The applicant is proposing to realign Providence Church Road approximately 580 feet to the east to improve the spacing from Woodside Lane, and build an eastbound left-turn lane on Route 54. Under build conditions, the minor street left-turn movement is expected to continue to operate with short delays during the AM and PM peak hours, with queue lengths of two vehicles or less.

Under build conditions, with Phase 1 of the Iron Horse Business Park trips, the minor street left-turn movement is expected to continue to operate with short delays during the AM and PM peak hours.

Table 8: Capacity Analysis Results - Route 54 at Meyers Park Drive / Iron Horse Driveway

		Lane		AM I	Peak Hou	r		РМ	Peak Ho	ur
Condition	Lane Group	Storage (ft.)	LOS	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max. Queue (ft.)	LOS	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max. Queue (ft.)
Build (2027) Conditions	EBL	200	Α	8.0	0	23	Α	7.9	3	29
	EBT	_	-	-	_ 03	-	-	-	-	_
	WBT/R	-	-		-	-	-	-	-	-
	SBL/R	-	В	11.1	3	33	В	11.1	3	38
	EBL	200	Α	8.1	0	22	Α	7.9	3	29
uild (2027) Conditions	EBT/R	_	_	-	-	-	-	-	-	-
Build (2027) Conditions, with Iron Horse Business Park Trips	WBL	200	Α	7.8	0	26	Α	8.2	0	26
	WBT/R	_	_		_	-	_	_	_	-
	NBL/T/R	-	Α	0.0	0	0	В	10.8	3	29
	SBL/T/R		В	11.7	5	40	В	11.8	- 3	33

Under build conditions, the minor street left-turn movement is expected to operate with short delays during the AM and PM peak hours, with queue lengths of two vehicles or less.

The Iron Horse Business Park is proposing to construct a full-movement driveway on Route 54 across from the Hickory Grove proposed site driveway. Under build conditions with Meyers Park Drive in place and Phase 1 of the Iron Horse Business Park trips, the minor street left-turn movement is expected to continue to operate with short delays during the AM and PM peak hours.

Table 9: Capacity Analysis Results - Route 54 at Goddins Hill Road

		Lane		AM F	Peak Hou	r		РМ	Peak Ho	ur
Condition	Lane Group	Storage (ft.)	Los	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max. Queue (ft.)	Los	Delay (sec)	Queue (ft.)	SimTraffic Avg. Max. Queue (ft.)
	EBT	-	-		1	-			-	-
Suintine (2022) Conditions	EBR	150	-	25	_	_	_	-	-	-
Existing (2023) Conditions	WBL/T	-	Α	7.9	5	0	Α	8.2	3	0
	NBL/R	-	В	12.2	8	63	В	12.6	15	71
	EBT	_	-	_	-	-	-	-	_	_
L D 11 L (0007) 0 111	EBR	150	-	-	-	-	-	-	-	-
lo-Build (2027) Conditions	WBL/T	-	Α	7.9	5	0	Α	8.2	3	0
	NBL/R	-	В	12.6	10	64	В	13.1	15	70
	EBT		-	_	_		_	_	-	
	EBR	150	_	_	_111		_	_		
Build (2027) Conditions	WBL/T	_	Α	7.9	5	84	Α	8.2	3	63
	NBL/R		В	12.7	10	74	В	13.2	18	93
	EBT	_	-		-	-	-	. 0.2		-
Build (2027) Conditions with Iron Horse Business	EBR	150	_	_	-	_		_	-	_
	WBL/T	-	Α	7.9	5	64	A	8.4	3	56
	NBL/R	_	В	13.1	10	64	В	13.8	18	96

Capacity analysis indicates that the minor street left-turn movement currently operates with short delays during the AM and PM peak hours. Under no-build conditions, the minor street left-turn movement is expected to continue to operate with short delays during the AM and PM peak hours.

Under build conditions, the minor street left-turn movement is expected to continue to operate with short delays during the AM and PM peak hours, with gueue lengths of three vehicles or less.

No improvements are warranted or recommended at this intersection at build-out of the proposed neighborhood.

Under build conditions, with Phase 1 of the Iron Horse Business Park trips, the minor street left-turn movement is expected to continue to operate with short delays during the AM and PM peak hours.

#### Turn Lane Warrant Analysis

Turn lanes were evaluated at the intersection of Route 54 at realigned Providence Church Road and at Meyers Park Drive based on Build 2027 traffic conditions. The projected build-out traffic volumes do not warrant any turn lanes on Route 54 at either intersection. The VDOT turn lane warrant worksheets are included in the appendix.

#### Recommendations

All of the study intersections are expected to operate acceptably at build-out of the proposed Hickory Grove neighborhood.

The projected build-out traffic volumes do not warrant any turn lanes on Route 54 at realigned Providence Church Road or Meyers Park Drive. However, the applicant is proposing the following improvements:

- Realign Providence Church Road approximately 580 feet to the east to improve the spacing from Woodside Lane
- Widen Route 54 to a three-lane section from Woodside Lane to Meyers Park Drive to provide eastbound left-turn lanes on Route 54 at realigned Providence Church Road and at Meyers Park Drive.

The recommended lane configuration is shown in Figure 12.

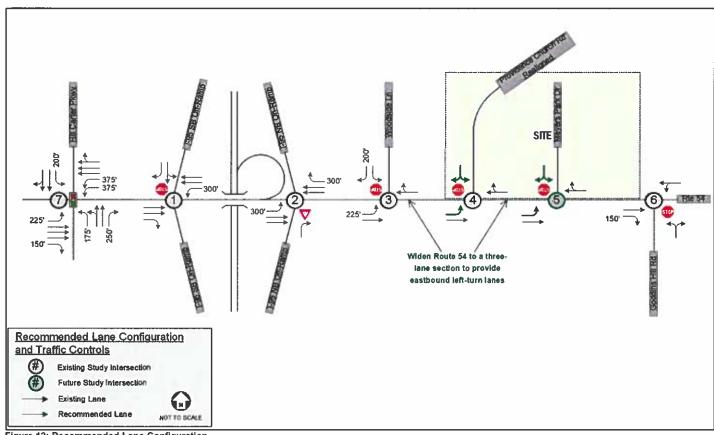


Figure 12: Recommended Lane Configuration

## **APPENDIX**

## **APPENDIX A: TIA SCOPING DOCUMENT**



### PRE-SCOPE OF WORK MEETING FORM

## Information on the Project Traffic Impact Analysis Base Assumptions

The applicant is responsible for entering the relevant information and submitting the form to VDOT and the locality no less than three (3) business days prior to the meeting. If a form is not received by this deadline, the scope of work meeting may be postponed.

<b>Contact Information</b>	1			
Consultant Name:	•	Hultgren, P.E., PTOE		
Tele:	(804) 310-6040			
E-mail:	ch@goroveslade.con			
Developer/Owner Name:	ROgers-Chenault - C	Corey Sedlar		
Tele:	(804) 249-3501			
E-mail:	coreys@htrsi.com			
<b>Project Information</b>				
Project Name:	Hickory Grove		Locality/County:	Hanover County
Project Location: (Attach regional and site specific location map)	See Figure 1			
Submission Type	Comp Plan	Rezoning 🛚	Site Plan	Subd Plat
Project Description: (Including details on the land use, acreage, phasing, access location, etc. Attach additional sheet if necessary)	project includes 157	he north side of Route age-restricted homes a e Church Road and an	nd apartments. The a	ccess plan includes
Proposed Use(s): (Check all that apply; attach additional pages as necessary)	Residential 🗵	Commercial	Mixed Use	Other
	Residential Uses(s)			
	Number of Units:	See trip table		<del></del>
	ITE LU Code(s):			
			Other Use(s)	
			ITE LU Code(s):	
	Commercial Use(s)			-
	ITE LU Code(s):			
			Independent Variable	e(s):
	Square Ft or Other V	ariable:		
Total Peak Hour Trip Projection:	Less than 100	100 – 499	500 – 999	1,000 or more

Traffic Impact Analy	sis Assumption	S			A LEVEL OF THE SECOND S				
Study Period	Existing Year: 201	.9 Buil	d-out	Yea	r: 2027		Design \	ear/	: 2027
Study Area Boundaries	North: See Figure	1		Sout	:h:				
(Attach map)	East:			Wes	t:				
External Factors That Could Affect Project (Planned road improvements, other nearby developments)	Iron Horse Busines	ss Park TIA	by Ti	imme	ons Group				
Consistency With Comprehensive Plan (Land use, transportation plan)	Existing zoning is	A-1							
Available Traffic Data (Historical, forecasts)	Route 54 - 3,500 v Providence Church			_		in 2017	,		
Trip Distribution	Road Name: See I	Figure 2		Roa	ad Name:				
(Attach sketch)	Road Name:			Roa	ad Name:				
Annual Vehicle Trip	1.5%	Peak Perio			ıdy	⊠ A	м 🛛	PM	SAT
Growth Rate:		Peak Hou	r of th	ne G	enerator				
	1.Route 54 at Hill	Carter Park	way	6.R	Coute 54 at	Goddin	s Hill Ro	ad	
Study Intersections	2.Route 54 at I-95 Ramps	Southbound	1	7.					
and/or Road Segments (Attach additional sheets as	3.Route 54 at I-95 Ramps	Northbound	1	8.					
necessary)	4.Route 54 at Prop Driveway	osed Site		9.					
	5.Route 54 at Prov Road	idence Chui	rch	10.					
Trip Adjustment Factors	Internal allowance Reduction:0		⊠ No	)	Pass-by a Reduction		e: 🗌 Y _% trips	es	No
Software Methodology	Synchro 🗌 H	CS (v.2000/	'+) [	aa	SIDRA	CORS	м 🛛 с	the	r SimTraffic
Traffic Signal Proposed or Affected (Analysis software to be used, progression speed, cycle length)	Synchro / SimTrafi study intersections				•				

It is important for the applicant to provide sufficient information to county and VDOT staff so that questions regarding geographic scope, alternate methodology, or other issues can be answered at the scoping meeting.

Improvement(s) Assumed or to be Considered	The need for turn lanes and other off-site improvements will be determined based of the results of the analysis.
Background Traffic Studies Considered	Iron Horse Business Park TIA by Timmons Group
Plan Submission	☐ Master Development Plan (MDP) ☐ Generalized Development Plan (GDP) ☐ Preliminary/Sketch Plan ☐ Other Plan type (Final Site, Subd. Plan)
Additional Issues to be Addressed	☑ Queuing analysis       ☒ Actuation/Coordination       ☒ Weaving analysis         ☐ Merge analysis       ☒ Bike/Ped Accommodations       ☒ Intersection(s)         ☐ TDM Measures       ☒ Other
NOTES on ASSUMPTIONS	S:
The TIA will include three a	analysis scenarios:
- Existing (2019) Conditions	
- No-Build (2027) Condition	ns and the second secon
- Build (2027) Conditions	
SIGNED:Applicant of	DATE:
PRINT NAME:	
· · · · · · · · · · · · · · · · · · ·	or Consultant

## **APPENDIX B: TURNING MOVEMENT COUNT DATA**

## Peggy Malone & Associates 904-992-8072

File Name: 4-I-95 SB Ramps & Route 55 Al

Site Code:

Start Date : 5/29/2019

Page No : 1

**Groups Printed-Combined** 

								- QI	vulpa	i i initec	- VVIII	PHICH			100				and it		
		SB I-9	5 Off I	Ramp	S			Rt. 54				SB I-9	5 On F	Ramp:	S			Rt. 54			
		Sol	uthbou	und			W	estbou	ınd			No	rthbou	und			Ea	stbou	nd		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. To
07:00 AM	50	0	2	0	52	0	162	67	0	229	0	0	0	0	0	158	44	0	0	202	48
07:15 AM	52	0	.5	0	57	0	189	89	0	278	0	0	0	0	0	182	60	0	0	242	5
07:30 AM	60	0	3	0	63	0	232	66	0	298	0	0	0	0	0	197	62	0	0	259	63
07:45 AM	66	0	-1	0	67	0	226	52	0	278	0	0	0	0	0	152	76	0	0	228	5
Total	228	0	Ш	0	239	0	809	274	0	1083	0	0	0	0	0	689	242	0	0	931	22:
08:00 AM	37	0	0	0	37	0	206	66	0	272	0	0	0	0	0	148	79	0	0	227	5
08:15 AM	46	0	4	0	50	0	167	52	0	219	0	0	0	0	0	146	71	0	0	217	4.
08:30 AM	52	0	1	0	53	0	140	17	0	157	0	0	0	0	0	134	68	0	0	202	4
08:45 AM	47	0	3	0	50	0	152	29	0	181	0	0	0	0	0	141	70	0	0	211	4
Total	182	0	8	0	190	0	665	164	0	829	0	0	0	0	0	569	288	0	0	857	18
Grand Total	410	0	19	0	429	0	1474	438	0	1912	0	0	0	0	0	1258	530	0	0	1788	41:
Apprch %	95.6	0	4.4	0		0	77.1	22.9	0		0	0	0	0		70.4	29.6	0	0		
Total %	9.9	0	0.5	0	10.4	0	35.7	10.6	0	46.3	- 0	0	0	0	0	30.5	12.8	0	0	43.3	

	SE	3 I-95 O Southl		ps		Rt. Westb			SE	3 I-95 O Northb		ps		Rt. Eastb			
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
eak Hour Analysis Froi	n 07:00 AM	to 08:45 Al	M - Peak I	of 1													
eak Hour for Entire	Intersection	Begins at	07:15 AM	1													
07:15 AM	52	0	5	57	0	189	89	278	0	0	0	0	182	60	0	242	577
07:30 AM	60	0	3	63	0	232	66	298	0	0	0	0	197	62	0	259	620
07:45 AM	66	0	1	67	0	226	52	278	0	0	0	0	152	76	0	228	573
08:00 AM	37	0	0	37	0	206	66	272	0	0	0	0	148	79	0	227	536
Total Volume	215	0	9	224	0	853	273	1126	0	0	0	0	679	277	0	956	2306
% App. Total	96	0	4		0	75.8	24.2		0	0	0		71	29	0		
PHF	.814	.000	.450	.836	.000	.919	.767	.945	.000	.000	.000	.000	.862	.877	.000	.923	.930

## Peggy Malone & Associate 904-992-8072

File Name : 4-I-95 SB Ramps & Route 55 Pl Site Code :

Start Date : 5/29/2019

Page No : 1

**Groups Printed- Combined** 

								-		1 1111100		211104									_
		SB I-9	5 Off F	?amp	S			Rt. 54	ļ			SB I-9	5 On I	Ramp	s			Rt. 54			
		Sot	uthbou	und			W	estbou	ınd			No	rthboi	und			Ea	stbou	nd		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. To
04:00 PM	51	0	3	0	54	0	180	28	0	208	0	0	0	0	0	137	92	0	0	229	49
04:15 PM	43	0	4	0	47	0	198	39	0	237	0	0	0	0	0	157	125	0	0	282	50
04:30 PM	37	1	6	0	44	0	212	30	0	242	0	0	0	0	0	147	119	0	0	266	5:
04:45 PM	62	0	6	0	68	0	202	33	0	235	0	0	0	0	0	123	120	. 0	0	243	54
Total	193	I	19	0	213	0	792	130	0	922	0	0	0	0	0	564	456	0	0	1020	21;
05:00 PM	46	0	5	0	51	0	203	27	0	230	0	0	0	0	0	159	135	0	0	294	5
05:15 PM	49	0	9	0	58	0	256	31	0	287	0	0	0	0	0	150	125	0	0	275	6
05:30 PM	59	0	6	0	65	0	245	22	0	267	0	0	0	0	0	137	135	0	0	272	6
05:45 PM	52	0	2	0	54	0	213	17	0	230	0	0	0	0	0	137	122	0	0	259	54
Total	206	0	22	0	228	0	917	97	0	1014	0	0	0	0	0	583	517	0	0	1100	234
Grand Total	399	1	41	0	441	0	1709	227	0	1936	0	0	0	0	0	1147	973	0	0	2120	449
Appreh %	90.5	0.2	9.3	0		0	88.3	11.7	0		0	0	0	0		54.1	45.9	0	0		
Total %	8.9	0	0.9	0	9.8	0	38	5	0	43.1	0	0	0	0	0	25.5	21.6	0	0	47.1	

	Si	3 I-95 O	ff Ram	ps		Rt.	54	Ť.	SE	3 I-95 O	n Ram	ps		Rt.	54		
67		South	oound			Westk	ound			Northb	bnuoc	.		Eastb	ound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
eak Hour Analysis Fron	n 04:00 PM	to 05:45 PM	I - Peak I o	if II:													
eak Hour for Entire I	Intersection	Begins at	04:45 PM	I.													
04;45 PM	62	0	6	68	0	202	33	235	0	0	0	0	123	120	0	243	546
05:00 PM	46	0	5	51	0	203	27	230	0	0	0	0	159	135	0	294	575
05:15 PM	49	0	9	58	0	256	31	287	0	0	0	0	150	125	0	275	620
05:30 PM	59	0	6	65	0	245	22	267	0	0	0	0	137	135	0	272	604
Total Volume	216	0	26	242	0	906	113	1019	0	0	0	0	569	515	0	1084	2345
	89.3	0	10.7		0	88.9	H.I		0	0	0		52.5	47.5	0		
PHF	.871	.000	.722	-890	.000	885	.856	-888	-000	.000	.000	.000	.895	-954	.000	922	946

## Peggy Malone & Associate 904-992-8072

File Name : 3-I-95 NB Ramps & Route 54 Al Site Code :

Start Date : 5/29/2019

Page No : 1
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			_					G	ogha	Fillited	- Colli	DILIER				22					
		NB I-9	5 On F	Ramp:	S			Rt 54				<b>NB I-9</b>	5 Off I	Ramp	S			Rt 54			
		Sot	ithbol	und			We	estbou	ınd			No	rthbou	ınd			Εa	stbou	ınd		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Rìght	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Tes
07:00 AM	134	-0	0	0	134	4	94	0	0	98	13	1	0	0	14	0	- 11	35	0	46	29
07:15 AM	140	0	0	0	140	9	143	0	0	152	17	0	0	0	17	0	25	43	0	68	37
07:30 AM	151	0	0	0	151	7	148	0	0	155	21	0	0	0	21	0	35	30	0	65	39
07:45 AM	164	0	0	0	164	- 11	113	0	0	124	20	0	0	0	20	0	46	31	0	77	38
Total	589	0	0	0	589	31	498	0	0	529	71		0	0	72	0	117	139	0	256	144
08:00 AM	144	0	0	0	144	5	115	0	0	120	24	0	0	0	24	0	41	38	0	79	36
08:15 AM	129	0	0	0	129	l L	89	0	0	90	31	0	0	0	31	0	40	30	0	70	32
08:30 AM	106	0	0	0	106	7	57	0	0	64	23	0	0	0	23	0	27	43	0	70	26
08:45 AM	110	0	0	0	110	3	73	0	0	76	26	0	0	0	26	0	38	36	0	74	28
Total	489	0	0	0	489	16	334	0	0	350	104	0	0	0	104	0	146	147	0	293	123
Grand Total	1078	0	0	0	1078	47	832	0	0	879	175	- 1	0	0	176	0	263	286	0	549	268
Appreh %	100	0	0	0		5.3	94.7	0	0		99.4	0.6	0	0		0	47.9	52.1	0		
Total %	40.2	0	0	0	40.2	1.8	31	0	0	32.8	6.5	0	0	0	6.6	0	9.8	10.7	- 0	20.5	

	NE	3 I-95 O Southb		1 -		Rt Westb			NE	3 I-95 O Northb				Rt Eastb			
Start Time	Right	Thru	Left		Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis Froi	n 07:00 AM	to 08:45 AN	4 - Peak 1	of I				_014-013					2000000		255.475		
Peak Hour for Entire	Intersection	Begins at	07:15 Al	M													
07:15 AM	140	0	0	140	9	143	. 0	152	17	0	0	17	0	25	43	68	377
07:30 AM	151	0	0	151	7	148	0	155	21	0	0	21	0	35	30	65	392
07:45 AM	164	0	0	164	11	113	0	124	20	0	0	20	0	46	31	77	385
08:00 AM	144	0	0	144		115	0	120	24	0	0	24	0	41	38	79	367
Total Volume	599	0	0	599	32	519	0	551	82	0	0	82	0	147	142	289	1521
% App. Total	100	0	- 0		5.8	94.2	0		100	0	0		0	50.9	49.1	- 1	
PHF	.913	.000	.000	.913	.727	.877	.000	.889	.854	.000	.000	.854	.000	.799	.826	.915	.970



File Name: Hanover-Hanover(Woodside Lane and East Patrick Henry Roac

Site Code:

Start Date : 4/25/2023

Page No : 1

	/ Road	trick Henry astbound			atrick Henry Vestbound			odside Lar outhbound		
Int. Total	App. Total	Left	Thru	App. Total	Thru	Right	App. Total	Left	Right	Start Time
126	31	5	26	58	58	0	37	7	30	07:00 AM
153	35	10	25	66	66	0	52	13	39	07:15 AM
168	49	10	39	72	68	4	47	12	35	07:30 AM
203	68	9	59	87	84	3	48	10	38	07:45 AM
650	183	34	149	283	276	7	184	42	142	Total
199	71	20	51	76	67	9	52	12	40	08:00 AM
158	64	12	52	54	52	2	40	15	25	08:15 AM
157	54	.11	43	70	68	2	33	6	27	08:30 AM
130	48	11	37	56	50	6	26	2	24	08:45 AM
644	237	54	183	256	237	19	151	35	116	Total
1294	420	88	332	539	513	26	335	77	258	Grand Total
	45.05.25	21	79		95.2	4.8		23	77	Apprch %
	32.5	6.8	25.7	41.7	39.6	2	25.9	6	19.9	Total %
1242	402	87	315	512	488	24	328	72	256	Cars +
96	95.7	98.9	94.9	95	95.1	92.3	97.9	93.5	99.2	% Cars +
52	18	1	17	27	25	2	7	5	2	Trucks
4	4.3	1.1	5.1	5	4.9	7.7	2.1	6.5	8.0	% Trucks



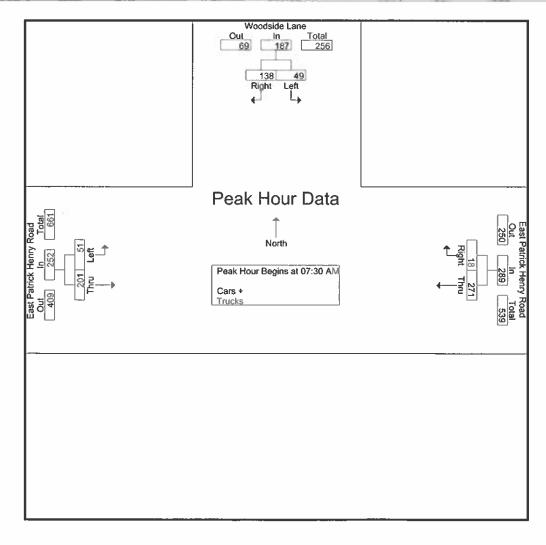
File Name: Hanover-Hanover(Woodside Lane and East Patrick Henry Road

Site Code:

Start Date : 4/25/2023

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		odside La outhbound			atrick Henr Vestbound	•		itrick Henr Eastbound	,	
Start Time	Right	Left	App. Total	Right	Thru	App. Total	Thru	Left	App. Total	Int. Tota
Peak Hour Analysis From	07:00 AM to	08:45 AN	/I - Peak 1 of 1				-271164			
Peak Hour for Entire Inter	section Beg	ins at 07:3	0 AM							
07:30 AM	35	12	47	4	68	72	39	10	49	16
07:45 AM	38	10	48	3	84	87	59	9	68	20:
08:00 AM	40	12	52	9	67	76	51	20	71	19
08:15 AM	25	15	40	2	52	54	52	12	64	15
Total Volume	138	49	187	18	271	289	201	51	252	72
% App. Total	73.8	26.2		6.2	93.8		79.8	20.2		
PHF	.863	.817	.899	.500	.807	.830	.852	.638	.887	.89





File Name: Hanover-Hanover(Woodside Lane and East Patrick Henry Roac

Site Code:

Start Date : 4/25/2023

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	Road	trick Henry astbound			atrick Henry Vestbound			odside Lar outhbound		
Int. Total	App. Total	Left	Thru	App. Total	Thru	Right	App. Total	Left	Right	Start Time
160	91	19	72	46	37	9	23	0	23	04:00 PM
178	95	26	69	53	47	6	30	4	26	04:15 PM
208	112	34	78	65	63	2	31	4	27	04:30 PM
179	97	35	62	51	47	4	31	2	29	04:45 PM
725	395	114	281	215	194	21	115	10	105	Total
216	107	34	73	78	75	3	31	6	<b>2</b> 5	05:00 PM
233	143	51	92	61	60	1	29	10	19	05:15 PM
212	125	43	82	50	49	1	37	3	34	05:30 PM
200	107	37	70	60	55	5	33	6	27	05:45 PM
861	482	165	317	249	239	10	130	25	105	Total
1586	877	279	598	464	433	31	245	35	210	Grand Total
		31.8	68.2		93.3	6.7		14.3	85.7	Apprch %
	55.3	17.6	37.7	29.3	27.3	2	15.4	2.2	13.2	Total %
1562	867	277	590	453	426	27	242	33	209	Cars +
98.5	98.9	99.3	98.7	97.6	98.4	87.1	98.8	94.3	99.5	% Cars +
24	10	2	8	11	7	4	3	2	1	Trucks
1.5	1.1	0.7	1.3	2.4	1.6	12.9	1.2	5.7	0.5	% Trucks



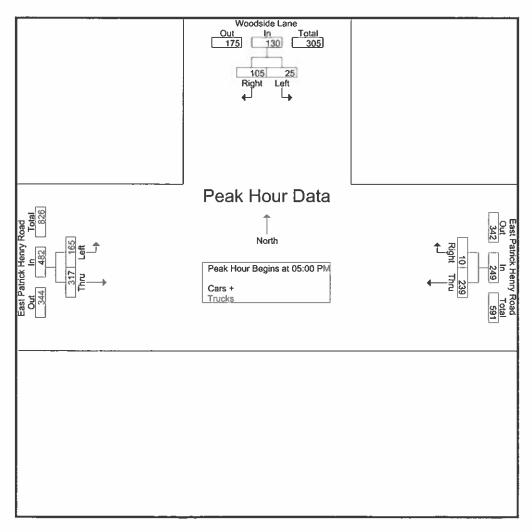
File Name: Hanover-Hanover(Woodside Lane and East Patrick Henry Roac

Site Code:

Start Date : 4/25/2023

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		odside La			itrick Henr Vestbound	,		trick Henr Eastbound		
Start Time	Right	Left	App. Total	Right	Thru	App. Total	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From	04:00 PM to	05:45 PM	/ - Peak 1 of 1	-3 400 6					10-200	
Peak Hour for Entire Inter	section Begi	ins at 05:0	0 PM			5.9				
05:00 PM	25	6	31	3	75	78	73	34	107	216
05:15 PM	19	10	29	1	60	61	92	51	143	233
05:30 PM	34	3	37	1	49	50	82	43	125	212
05:45 PM	27	6	33	- 5	55	60	70	37	107	200
Total Volume	105	25	130	10	239	249	317	165	482	861
% App. Total	80.8	19.2		4	96		65.8	34.2		
PHF	.772	.625	.878	.500	.797	.798	.861	.809	.843	.924



## Peggy Malone & Associate 904-992-8072

File Name : 3-I-95 NB Ramps & Route 54 Pl

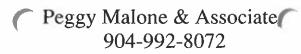
Site Code:

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**Groups Printed- Combined** 

																				-
	NB I-9	5 On F	Ramp:	S			Rt 54				<b>NB I-9</b>	5 Off I	Ramp	s			Rt 54			
	Sou	ıthboı	und			We	estbou	ınd			No	rthbou	und			Ea	astbou	nd		
Right	Thru	Left	Peds	App, Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Tota
136	0	0	0	136	7	67	0	0	74	23	0	0	0	23	0	55	41	0	96	32
147	0	0	0	147	9	89	0	0	98	52	0	0	0	52	0	67	65	0	132	42
150	0	0	0	150	6	92	0	0	98	43	0	0	0	43	0	70	56	0	126	41
171	0	0	0	171	11	65	0	0	76	40	0	0	0	40	0	73	56	0	129	41
604	0	0	0	604	33	313	0	0	346	158	0	0	0	158	0	265	218	0	483	159
153	0	0	0	153	5	81	0	0	86	54	0	0	0	54	0	79	57	0	136	42
194	0	0	0	194	3	88	0	0	91	45	0	0	0	45	0	87	53	0	140	47
207	0	0	0	207	5	63	0	0	68	59	0	0	0	59	0	85	56	0	141	47
165	0	0	0	165	6	65	0	0	71	52	0	0	0	52	0	65	59	0	124	41
719	0	0	0	719	19	297	0	0	316	210	0	0	0	210	0	316	225	0	541	178
1323	0	0	0	1323	52	610	0	0	662	368	0	-()	0	368	0	581	443	0	1024	337
100	0	0	0		7.9	92.1	0	0		100	0	0	0	- 1	0	56.7	43.3	0		
39.2	0	0	0	39.2	1.5	18.1	0	0	19.6	10.9	0	0	0	10.9	0	17-2	13.1	0	30.3	
	Right   136   147   150   171   604   153   194   207   165   719   1323   100	Right   Thru	Southbox   Right   Thru   Left   136   0   0   147   0   0   150   0   0   171   0   0   0   0   153   0   0   194   0   0   0   165   0   0   0   165   0   0   0   1323   0   0   100   0   0   0   0   0   0	Right   Thru   Left   Peds   136   0   0   0   147   0   0   0   155   0   0   0   0   171   0   0   0   0   194   0   0   0   0   165   0   0   0   0   165   0   0   0   0   0   1323   0   0   0   0   100   0   0   0   0	Right         Thru         Left         Peds         App. Total           136         0         0         0         136           147         0         0         0         147           150         0         0         0         150           171         0         0         0         171           604         0         0         0         604           153         0         0         0         153           194         0         0         0         194           207         0         0         0         207           165         0         0         0         165           719         0         0         0         719           1323         0         0         0         1323           100         0         0         0         0	Southbound   Right   Thru   Left   Peds   App. Total   Right   136   0   0   0   136   7   147   0   0   0   147   9   150   0   0   0   150   6   171   0   0   0   0   171   11   604   0   0   0   0   604   33   153   0   0   0   0   194   3   207   0   0   0   0   207   5   165   0   0   0   165   6   6   719   0   0   0   0   1323   52   100   0   0   0   0   7.9	Right         Thru         Left         Peds         App. Total         Right         Thru           136         0         0         0         136         7         67           147         0         0         0         147         9         89           150         0         0         0         150         6         92           171         0         0         0         171         11         65           604         0         0         0         604         33         313           153         0         0         0         153         5         81           194         0         0         0         194         3         88           207         0         0         0         207         5         63           165         0         0         0         165         6         65           719         0         0         0         719         19         297           1323         0         0         0         1323         52         610           100         0         0         0         7.9         92.1 <td>  NB I-95 On Ramps   Southbound   Westbound   Right   Thru   Left   Peds   App. Total   Right   Thru   Left   136   O   O   O   136   7   67   O   O   147   O   89   O   150   O   692   O   171   O   O   O   171   11   65   O   O   O   171   11   65   O   O   O   O   O   O   O   O   O  </td> <td>  NB I-95 On Ramps   Southbound   Westbound   Westbound   Right   Thru   Left   Peds   App. Total   Right   Thru   Left   Peds   136   O   O   O   136   7   67   O   O   O   147   O   89   O   O   O   150   O   692   O   O   O   171   O   O   O   O   171   11   65   O   O   O   O   O   O   O   O   O  </td> <td>  NB I-95 On Ramps   Southbound   Westbound   Westbound   Right   Thru   Left   Peds   App. Total   Right   Right  </td> <td>  NB   -95 On Ramps   Southbound   NB   -95 On Ramps   Southbound   Westbound   Westbound   NB   -100   NB   -100   NB   NB   -100   NB   NB   NB   NB   NB   NB   NB  </td> <td>  NB I-95 On Ramps   Southbound   Westbound   NB I-9    </td> <td>  NB I-95 On Ramps   Southbound   Westbound   Westbound   Northbound   Northbound  </td> <td>  NB I-95 On Ramps   Southbound   Westbound   Westbound   Northbound   Northbound  </td> <td>  NB I-95 On Ramps   Southbound   Westbound   Westbound   Northbound   Northbound  </td> <td>  NB I-95 OF Ramps   Southbound   New Periods   New Periods   Right   Thru   Left   Peds   App. Total   Right   Right   Right   Thru   Left   Peds   App. Total   Right   Righ</td> <td>  NB I-95 On Ramps   Southbound   Westbound   Superior   Northbound   Northbound  </td> <td>  NB I-95 On Ramps   Southbound   Westbound   Westbound   Northbound   Northbound   Right   Thru   Left   Peds   App. Total   Right   Thru   Left   Thru   Left   Peds   App. Total   Right   Thru   Left   Thru   Left   Thru   Left   Peds   App. Total   Right   Thru   Left   Thru   Left   Peds   App. Total   Right   Thru   Left   Thru   Left   Thru   Left   Thru   T</td> <td>  NB I-95 On Ramps   Southbound   Westbound   Westbound   Westbound   Northbound   Right   Thru   Left   Peds   App. Total   Right   Thru   Left   Peds   Right   Thru   Left   Peds   Right   Right</td> <td>  NB I-95 On Ramps   Southbound   Supplementaries   NB I-95 Off Ramps   Southbound   Supplementaries   NB I-95 Off Ramps   NB I-95 Off Ramps   NB I-96 Off Parts   NB I-</td>	NB I-95 On Ramps   Southbound   Westbound   Right   Thru   Left   Peds   App. Total   Right   Thru   Left   136   O   O   O   136   7   67   O   O   147   O   89   O   150   O   692   O   171   O   O   O   171   11   65   O   O   O   171   11   65   O   O   O   O   O   O   O   O   O	NB I-95 On Ramps   Southbound   Westbound   Westbound   Right   Thru   Left   Peds   App. Total   Right   Thru   Left   Peds   136   O   O   O   136   7   67   O   O   O   147   O   89   O   O   O   150   O   692   O   O   O   171   O   O   O   O   171   11   65   O   O   O   O   O   O   O   O   O	NB I-95 On Ramps   Southbound   Westbound   Westbound   Right   Thru   Left   Peds   App. Total   Right   Right	NB   -95 On Ramps   Southbound   NB   -95 On Ramps   Southbound   Westbound   Westbound   NB   -100   NB   -100   NB   NB   -100   NB   NB   NB   NB   NB   NB   NB	NB I-95 On Ramps   Southbound   Westbound   NB I-9	NB I-95 On Ramps   Southbound   Westbound   Westbound   Northbound   Northbound	NB I-95 On Ramps   Southbound   Westbound   Westbound   Northbound   Northbound	NB I-95 On Ramps   Southbound   Westbound   Westbound   Northbound   Northbound	NB I-95 OF Ramps   Southbound   New Periods   New Periods   Right   Thru   Left   Peds   App. Total   Right   Right   Right   Thru   Left   Peds   App. Total   Right   Righ	NB I-95 On Ramps   Southbound   Westbound   Superior   Northbound   Northbound	NB I-95 On Ramps   Southbound   Westbound   Westbound   Northbound   Northbound   Right   Thru   Left   Peds   App. Total   Right   Thru   Left   Thru   Left   Peds   App. Total   Right   Thru   Left   Thru   Left   Thru   Left   Peds   App. Total   Right   Thru   Left   Thru   Left   Peds   App. Total   Right   Thru   Left   Thru   Left   Thru   Left   Thru   T	NB I-95 On Ramps   Southbound   Westbound   Westbound   Westbound   Northbound   Right   Thru   Left   Peds   App. Total   Right   Thru   Left   Peds   Right   Thru   Left   Peds   Right   Right	NB I-95 On Ramps   Southbound   Supplementaries   NB I-95 Off Ramps   Southbound   Supplementaries   NB I-95 Off Ramps   NB I-95 Off Ramps   NB I-96 Off Parts   NB I-



File Name : 2-Woodside Ln\_Providence Church Rd & Route 54 Al

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										1 111100											
		Wo	odside	• Ln				Rt 54					Rt 54			Pi	rovide	nce Ch	iurch	Rd	
		So	uthbou	und			We	estbol	ınd			Ea	stbou	ınd			Sout	hwestk	ound	1	
Start Time	Right	Left	Hard Feh	Peds	App. Total	Hard Right	Right	Thru	Peds	App. Total	Thru	Boir Lefs	Left	Peds	App. Total	Hand Regle	Bear Right	Hard Left	Peds	App. Total	Int. Tota
07:00 AM	49	6	0	0	55	0	- 1	56	0	57	15	- 1	3	0	19	0	0	0	0	0	13
07:15 AM	72	6	0	0	78	0	3	53	0	56	21	0	11	0	32	1	1	0	0	2	16
07:30 AM	62	23	0	0	85	0	2	88	0	90	32	- 1	9	0	42	0	0	0	0	0	21
07:45 AM	42	17	0	0	59	0	3	61	0	64	41	1	10	0	52	0	- 1	-0	0	L	170
Total	225	52	0	0	277	0	9	258	0	267	109	3	33	0	145	1	2	0	0	3	69
08:00 AM	44	6	0	0	50	0	10	73	0	83	48	0	9	0	57	0	0	0	0	0	19
08:15 AM	28	2	0	0	30	0	4	37	0	41	49	0	12	0	61	0	0	0	0	0	13
08:30 AM	21	2	0	0	23	0	2	30	0	32	30	0	9	0	39	0	2	0	0	2	9
08:45 AM	21	3	0.	1	25	0	2	37	0	39	46	0	- 11	0	57	0	0	0	0	0	12
Total	114	13	0	1	128	0	18	177	0	195	173	0	41	0	214	0	2	0	0	2	539
Grand Total	339	65	0	1	405	0	27	435	0	462	282	3	74	0	359	- 1	4	0	0	5	123
Appreh %	83.7	16	0	0.2		0	5.8	94.2	0		78.6	8.0	20.6	0		20	80	0	0		1
Total %	27.5	5.3	0	0.1	32.9	0	2.2	35.3	0	37.5	22.9	0.2	6	0	29.2	0.1	0.3	0	0	0.4	

		Woods	side Ln			Rt	54			Rt	54		Pro	vidence	• Churc	h Rd	
		South	bound			Westb	ound			Eastb	ound			Southwe	estbour	nd	
Start Time	Right	Left	Flard Left	App. Total	Hard Right	Right	Thru	App. Total	Thru	Bear Left	Left	App. Total	Fland Right	Bew Right	Hard Left	App. Total	Int. Total
Peak Hour Analysis Fro	m 07:00 AM	to 08:45 A	M - Peak 1	of I													
Peak Hour for Entire	Intersection	Begins at	07:15 AN	А													
07:15 AM	72	6	0	78	0	3	53	56	21	0	- 11	32	1	1	0	2	168
07:30 AM	62	23	0	85	0	2	88	90	32	1	9	42	0	0	0	0	217
07:45 AM	42	17	0	59	0	3	61	64	41	1	10	52	0	1	0	an La	176
08:00 AM	44	6	0	50	0	10	73	83	48	0	9	57	0	0	0	0	190
Total Volume	220	52	0	272	0	18	275	293	142	2	39	183	1	2	0	3	751
	80.9	19.1	0		0	6.1	93.9		77.6	1.3	21.3		33.3	66.7	0		
PHF	.764	.565	.000	.800	.000	.450	.781	.814	.740	.500	.886	.803	.250	.500	.000	.375	-865

# Peggy Malone & Associates 904-992-8072

File Name : 2-Woodside Ln\_Providence Church Rd & Route 54 Pl

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		We	odside	Ln				Rt 54					Rt 54			P	rovide	nce Cl	nurch	Rd	
i		So	uthbou	ınd			W	estbou	ınd	100		Ea	astbou	nd			Sout	hwest	ound		
Start Time	Right	Left	Haro Lek	Peds	App. Total	Had Right	Right	Thru	Peds	App. Total	Thru	Bear Left	Left	Peds	App. Total	Hard Right	Bear Right	Hard Lask	Peds	App. Total	Int. Total
04:00 PM	14	3	0	0	17	0	4	38	0	42	39	-0	19	0	58	0	0	0	0	0	117
04:15 PM	17	2	0	0	19	0	4	53	0	57	62	- 1	32	0	95	0	1	0	0	- 1	172
04:30 PM	20	5	0	0	25	- 1	2	60	0	63	67	2	30	0	99	0	1	0	0	- 1	188
04:45 PM	25	5	0	0	30	0	5	.30	0	35	58	0	30	0	88	1_	0	0	0	- I	154
Total	76	15	0	0	91	1	15	181	0	197	226	3	111	0	340	- 1	2	0	0	3	631
05:00 PM	14	6	0	0	20	1	3	50	0	54	78	0	34	0	112	0	0	0	0	0	186
05:15 PM	21	6	0	0	27	1	2	47	0	50	84	0	41	0	125	2	- 1	0	0	3	205
05:30 PM	16	- 11	0	0	27	0	5	44	0	49	72	0	58	0	130	0	0	0	0	0	206
05:45 PM	17	.3	0	0	20	0	4	28	0	32	63	0	43	0	106	0	. 0	0	0	0	158
Total	68	26	0	0	94	2	14	169	0	185	297	0	176	-0	473	2	- 1	0	0	.3	755
Grand Total	144	41	0	0	185	3	29	350	0	382	523	3	287	0	813	3	3	0	0	6	1386
Appreh %	77.8	22.2	0	0		0.8	7.6	91.6	0		64.3	0.4	35.3	0		50	50	0	0		
Total %	10.4	3	0	0	13.3	0.2	2.1	25.3	0	27.6	37.7	0.2	20.7	0	58.7	0.2	0.2	0	0	0.4	j

		Woods South				Rt Westk	-			Rt Eastb	-	56,797			e Churc estbour		
Start Time	Right				Hard Right	Right	Thru	App. Total	Thru	Bear Left	Left	App. Total	Hand Right	Bear Right	Hard Left	App. Total	Int. Total
Peak Hour Analysis Fro																	
Peak Hour for Entire	Intersection	n Begins at	05:00 PM	4													
05:00 PM	14	6	0	20	1	3	50	54	78	0	34	112	0	0	0	0	186
05:15 PM	21	6	0	27	1	2	47	50	84	0	41	125	2	1	0	3	205
05:30 PM	16	.11	0	27	0	5	44	49	72	0	58	130	0	0	0	0	206
05:45 PM	17	3	0	20	0	4	28	32	63	0	43	106	0	0	0	0	158
Total Volume	68	26	0	94	2	14	169	185	297	0	176	473	2	1	0	3	755
% App. Total	72.3	27.7	0		1.1	7.6	91.4		62.8	0	37.2		66.7	33.3	0		
PHF	.810	.591	.000	.870	.500	.700	.845	.856	.884	.000	.759	.910	.250	.250	.000	.250	.916

# Peggy Malone & Associate 904-992-8072

File Name: 1-Goddins Hill Rd & Route 54 A

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				200	GIO	ups riiii	rea- cou	IDIFFEC					50	
		Rt.	54			Goddins	Hill Rd				Rt. 54			
		Westb	ound			Northb	ound			Ea	astbound	l		
Start Time	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Tota
07:00 AM	47	9	0	56	0	3	0	3	3	17	0	0	20	79
07:15 AM	53	20	0	73	5	2	0	7	4	23	0	0	27	107
07:30 AM	67	21	0	88	5	5	0	10	18	32	0	0	50	148
07:45 AM	54	15	0	69	6	5	0	11	18	40	0	0	58	138
Total	221	65	0	286	16	15	0	31	43	112	0	0	155	47:
08:00 AM	59	16	0	75	6	12	0	18	8	43	0	0	51	144
08:15 AM	33	3	0	36	3	3	0	6	1	46	0	0	47	89
08:30 AM	26	2	0	28	7	2	0	9	1	27	ı	0	29	60
08:45 AM	27	2	0	29	6	3	0	9	4	35	0	0	39	77
Total	145	23	0	168	22	20	0	42	14	151	T.	0	166	376
Grand Total	366	88	0	454	38	35	0	73	57	263	1	0	321	848
Apprch %	80.6	19.4	0		52.1	47.9	0	See a	17.8	81.9	0.3	0		
Total %	43.2	10.4	0	53.5	4.5	4.1	0	8.6	6.7	31	0.1	0	37.9	

	w	Rt. 54 estbound			dins Hill F orthbound		122.00	Rt. 5 Eastbo	-		
Start Time	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	Left	App. Total	Int. Tota
Peak Hour Analysis From 07:00 AN	of to 08:45 AM - Pea	dk   of t	- 5			- 100 St T0 St		3 17 37 1		20	
Peak Hour for Entire Intersection	on Begins at 07:15	AM									
07:15 AM	53	20	73	5	2	7	4	23	0	27	107
07:30 AM	67	21	88	5	5	10	18	32	0	50	148
07:45 AM	54	15	69	6	5	01.	18	40	0	58	138
08:00 AM	59	16	75	6	12	18	8	43	0	51	144
Total Volume	233	72	305	22	24	46	48	138	0	186	53
% App. Total	76.4	23,6	-	47.8	52,2	533	25.8	74,2	0		
PHF	.869	.857	.866	.917	.500	.639	.667	.802	.000	.802	.90

## Peggy Malone & Associate 904-992-8072

File Name: 1-Goddins Hill Rd & Route 54 PI

Site Code:

Start Date : 5/29/2019

Page No : 1

**Groups Printed- Combined** 

2			-	and the same of th	- 725	aro	upa r iiii	tea- con	ibilied						
			Rt.	54			Goddins	Hill Rd				Rt. 54			
			Westb	ound	- 19		Northb	ound			E	astbound	1		
5	Start Time	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
0	4:00 PM	29	7	0	36	9	8	0	17	3	37	- 1	0	41	94
0	4:15 PM	34	3	0	37	7	- 1	0	8	2	50	0	0	52	97
0	4:30 PM	41	6	0	47	15	4	0	19	7	55	0	0	62	128
0	4:45 PM	30	7	0	37	12	6	0	18	4	52	0	0	56	
	Total	134	23	0	157	43	19	0	62	16	194	- 1	0	211	430
0	5:00 PM	54	5	0	59	11	5	0	16	7	56	0	0	63	138
0	5:15 PM	39	7	0	46	- 11	3	0	14	6	79	0	0	85	145
0	5:30 PM	28	4	0	32	14	7	0	21	12	62	0	0	74	127
0	5:45 PM	26	nears, Le	0	27	7	6	0	13	7	46	0	0	53	.93
	Total	147	17	0	164	43	21	0	64	32	243	0	0	275	503
	nd Total	281	40	0	321	86	40	0	126	48	437	1	0	486	933
A	pprch %	87.5	12.5	0		68.3	31.7	0		9.9	89.9	0.2	0		
	Total %	30.1	4.3	0	34.4	9.2	4.3	0	13.5	5.1	46.8	0.1	0	52-1	

	W	Rt. 54 estbound			ldins Hill F orthbound			Rt. 5 Eastbo			
Start Time	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
eak Hour Analysis From 04:00 PM to	05:45 PM - Peal	k L of I									
eak Hour for Entire Intersection	Begins at 04:30	PM									
04:30 PM	41	6	47	15	4	19	7	55	0	62	128
04:45 PM	30	7	37	12	6	18	4	52	0	56	111
05:00 PM	54	5	59	- 11	- 5	16	7	-56	0	63	138
05:15 PM	39	7	46	- 11	3	14	6	79	0	85	145
Total Volume	164	25	189	49	18	67	24	242	0	266	522
% App. Total	86.8	13.2		73.1	26.9		9	91	0		
PHF	.759	.893	.801	.817	.750	.882	.857	.766	.000	.782	.900

APPENDIX C: SYNCHRO & SIMTRAFFIC OUTPUT - EXISTING (2023) CONDITIONS

ntersection nt Delay, s/veh	2.4													
•														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
ane Configurations		44		ሻ	44						बै	7		
raffic Vol, veh/h	0	294	0	290	905	0	0	0	0	10	1	228		
uture Vol, veh/h	0	294	0	290	905	0	0	0	0	10	1	228		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0		
ign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop		
T Channelized	-	-	None	-	-	None	-	-	None	-	-	Free		
torage Length	•	-	-	0	-	-	-	-	-	•	-	0		
eh in Median Storage,	# -	0	-	-	0	-	•	0	-	-	0	-		
rade, %	•	0	-	-	0	-	-	0	-	-	0	-		
eak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93		
leavy Vehicles, %	2	13	2	2	5	2	2	2	2	2	2	10		
fvmt Flow	0	316	0	312	973	0	0	0	0	11	1	245		
fajor/Minor M	lajor1		N	Major2					h	/linor2				
Conflicting Flow All	-	0		316	0	0			,	1755	1913	-	 	
Stage 1	_	-		010		-				1597	1597	_		
Stage 2			_	_	_	_				158	316	_		
ritical Hdwy	_	_		4.14	_	_				6.84	6.54	_		
critical Hdwy Stg 1	-	_		7.17		-				5.84	5.54	_		
critical Hdwy Stg 2			_		_	_				5.84	5.54	_		
ollow-up Hdwy	_	_	-	2.22	_					3.52	4.02	_		
ot Cap-1 Maneuver	0		0	1241		0				76	67	0		
Stage 1	0	_	0	12-71	_	0				152	164	ő		
Stage 2	0		0		_	0				854	654	0		
Platoon blocked, %	U		v			Ū				004	004	Ū		
Nov Cap-1 Maneuver	_	_		1241	_	_				57	0	-		
lov Cap-2 Maneuver	_		_	,_,,	_	-				57	0	_		
Stage 1	_	_	_	_	_	_				152	0	-		
Stage 2	-	-	-	-	-	-				640	0	-		
naraaah	ED			MD						OD.				
pproach	EB			WB						SB				
ICM Control Delay, s ICM LOS	0			2.2						84 F				
										'				
linor Lane/Major Mvmt		EBT	WBL	WBT									 	
apacity (veh/h)		-	1241	-	57	-								
CM Lane V/C Ratio		-	0.251	•	0.208	-								
ICM Control Delay (s)		-	8.9	-	84	0								
ICM Lane LOS		-	Α	-	F	Α								
ICM 95th %tile Q(veh)		-	1	-	0.7	-								

Intersection										
Int Delay, s/veh	0									
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	NEL	NER
Lane Configurations	7	44			44	7				7
Traffic Vol, veh/h	151	156	0	0	551	34	0	0	0	87
Future Vol, veh/h	151	156	0	0	551	34	0	0	0	87
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	Yield	-	•	-	Free
Storage Length	300	-	•	-	•	0	-	-	-	0
Veh in Median Storage,	# -	0	-	-	0	-	0	-	0	-
Grade, %	-	0	-	-	0	•	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	20	5	2	2	2	16	2	2	2	6
Mvmt Flow	162	168	0	0	592	37	0	0	0	94
Major/Minor M	linor2		ľ	Major2						
Conflicting Flow All	592	592	-			0				
Stage 1	592	592	-	_	-					
Stage 2	0	0	_	_	_					
Critical Hdwy	7.2	6.6		_	-					
Critical Hdwy Stg 1	6.2	5.6	-	-	-	-				
Critical Hdwy Stg 2	_	_	-	_	_					
Follow-up Hdwy	3.7	4.05	-	_	-	-				
Pot Cap-1 Maneuver	398	411	0	0	-	-				
Stage 1	469	485	0	0		•				
Stage 2		-	0	0	-	-				
Platoon blocked, %					-	-				
Mov Cap-1 Maneuver	398	0	-	-	-	-				
Mov Cap-2 Maneuver	398	0	-	-	-	-				
Stage 1	469	0	-	-	-	*				
Stage 2	-	0	-	-	-	-				
Approach	EB			WB						
HCM Control Delay, s				0						
HCM LOS	-									
Minor Lane/Major Mvmt	: 8	EBLn1 (	EBLn2 f	EBLn3	WBT	WBR				
Capacity (veh/h)		398	-	_	-	-				
HCM Lane V/C Ratio		0.408	_	_	_	-				
HCM Control Delay (s)		20.1	-		-	_				
HCM Lane LOS		С	_	-	-	-				
HCM 95th %tile Q(veh)		1.9	-	-	-	-				
, ,										

Intersection							
Int Delay, s/veh	5.4						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	'n	<b>†</b>	1		ሻ	7	
Traffic Vol, veh/h	41	153	294	20	55	233	
Future Vol, veh/h	41	153	294	20	55	233	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	225	-	-	-	0	200	
Veh in Median Storage	e,# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	87	87	87	87	87	87	
Heavy Vehicles, %	3	7	2	28	6	2	
Mvmt Flow	47	176	338	23	63	268	
Major/Minor	Major1		Major2		Minor2		
Conflicting Flow All	361	0	-	0	620	350	
Stage 1	-	-	-	-	350	-	
Stage 2	-	•	-	-	270	-	
Critical Hdwy	4.13	-	-	-	6.46	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.46	•	
Critical Hdwy Stg 2	-	-	-	-	5.46	-	
Follow-up Hdwy	2.227	-	-	-	3.554		
Pot Cap-1 Maneuver	1192	-	-	-	445	693	
Stage 1	-	-	-	-	705	-	
Stage 2	-	-	-	-	766	-	
Platoon blocked, %		-	•	•			
Mov Cap-1 Maneuver	1192	-	-	-	428	693	
Mov Cap-2 Maneuver	-	*	*	-	428	-	
Stage 1	-	-	-	-	678	-	
Stage 2	-	-	-	-	766	-	
Approach	EB		WB		SB		
HCM Control Delay, s	1.7		0		13.7		
HCM LOS					В		
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR:	SBLn1 S	BLn2
Capacity (veh/h)		1192				428	693
HCM Lane V/C Ratio		0.04		-		0.148	
HCM Control Delay (s	)	8.1	_	_	_	14.9	13.4
HCM Lane LOS	,	A		_	_	В	В
HCM 95th %tile Q(veh	1)	0.1	-	_		0.5	1.8
	,					3.0	

Intersection						
Int Delay, s/veh	0.1					
•		CDT	IAIDT	MOD	ODI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	শ	<b>†</b>	<b>}</b>		M	_
Traffic Vol, veh/h	2	206	311	1	1	3
Future Vol, veh/h	2	206	311	1	1	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None		None
Storage Length	0	-		-	0	-
Veh in Median Storage		0	0	-	0	-
Grade, %	~ı ''	0	0	-	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	2	7	2	2	2	2
Mvmt Flow	2	237	357	1	1	3
Major/Minor	Major1	٨	Major2		Minor2	
Conflicting Flow All	358	0		0	599	358
-	JU0	ď	•		358	JU0
Stage 1	•	-	-	-		-
Stage 2	4.40	-	-	-	241	-
Critical Hdwy	4.12	-	•	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	•	-	3.518	3.318
Pot Cap-1 Maneuver	1201	-	Ψ.	-	465	686
Stage 1	-	-	-	-	707	-
Stage 2	-		-	-	799	-
Platoon blocked, %		-	_	-		
Mov Cap-1 Maneuver	1201		_	_	464	686
Mov Cap-1 Maneuver	1201	_	_		464	-
	-	-	-	•		-
Stage 1	-	-	-	-	706	-
Stage 2	•	~	**	-	799	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		10.9	
HCM LOS	0.1		v		10.9 B	
HOW LOS					В	
Minor Lane/Major Mvn	nt	EBL.	EBT	WBT	WBR:	SBLn1
Capacity (veh/h)		1201		•		613
HCM Lane V/C Ratio		0.002	_		_	0.008
HCM Control Delay (s)		8	_	_		10.9
HCM Lane LOS	'		-	_	-	10.9 B
	١	A	•	•	•	
HCM 95th %tile Q(veh	)	0	-	-	-	0

Intersection						
Int Delay, s/veh	2.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>CD1</u>	ZBK	AADC	4	W	NON
Traffic Vol, veh/h	<b>T</b> 146	<b>r</b> 51	76	4 247	7 25	23
Future Vol, veh/h	146	51 51	76 76	247	25 25	23 23
			0		20	
Conflicting Peds, #/hr	0	0		0	-	O Stop
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length		150	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	7	6	4	3	13	5
Mvmt Flow	160	56	84	271	27	25
Major/Minor M	lajor1	,	Major2		Minor1	
	0	0	216	0	599	160
Conflicting Flow All		U	210			
Stage 1	-	-	-	-	160	-
Stage 2	-	-	-	-	439	-
Critical Hdwy	-	-	4.14	-	6.53	6.25
Critical Hdwy Stg 1	-	-	-	-	5.53	-
Critical Hdwy Stg 2	-	-	-	-	5.53	-
Follow-up Hdwy	•	-	2.236	-	3.617	3.345
Pot Cap-1 Maneuver	-	-	1342	-	447	877
Stage 1	-	-	-	-	843	-
Stage 2		-	-	-	627	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver			1342	_	414	877
Mov Cap-2 Maneuver	-	-	-	_	414	-
Stage 1	_	_	_	_	843	_
Stage 2					581	_
Olayo Z	•	-	-	-	J0 I	•
Approach	EB		WB		NB	
	0		1.8			
HCM Control Delay, s	U		۱.۵		12.2	
HCM LOS					8	
Minor Lane/Major Mvmt	. <u> </u>	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		554	-		1342	-
HCM Lane V/C Ratio		0.095	-	-	0.062	_
HCM Control Delay (s)		12.2	_	_	7.9	0
HCM Lane LOS					1	~
		R	_		Δ	Α
HCM 95th %tile Q(veh)		B 0.3	-	-	A 0.2	A

	*	-	7	-	4	4	<b>†</b>	1	1	<b>↓</b>	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Group Flow (vph)	21	963	32	159	1072	27	31	40	101	10	
v/c Ratio	0.17	0.33	0.03	0.51	0.32	0.24	0.13	0.17	0.60	0.01	
Control Delay	56.1	15.6	0.1	57.4	11.1	57.6	53.3	1.5	66.6	0.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	56.1	15.6	0.1	57.4	11.1	57.6	53.3	1.5	66.6	0.0	
Queue Length 50th (ft)	16	147	0	61	110	21	12	0	76	0	
Queue Length 95th (ft)	42	200	0	94	206	56	29	0	135	0	
Internal Link Dist (ft)		601			622		348			415	
Turn Bay Length (ft)	225		150	375		250		250	200		
Base Capacity (vph)	280	2877	954	543	3345	268	541	377	194	730	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.07	0.33	0.03	0.29	0.32	0.10	0.06	0.11	0.52	0.01	
Intersection Summary											

	*	<b>→</b>	*	•	+	4	4	†	~	1	<b>†</b>	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ā	ተተተ	7	<b>ሕ</b> ግ	ተተኈ		7	41	7	ሻ	₹þ	
Traffic Volume (vph)	19	886	29	146	982	5	50	4	37	93	2	7
Future Volume (vph)	19	886	29	146	982	5	50	4	37	93	2	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0	6.0	6.0	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91		0.91	0.91	1.00	1.00	0.95	
Frt	1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85	1.00	0.88	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00	0.95	1.00	
Satd. Flow (prot)	1770	5085	1583	3433	5082		1610	3249	1583	1770	3115	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00	0.95	1.00	
Satd. Flow (perm)	1770	5085	1583	3433	5082		1610	3249	1583	1770	3115	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	21	963	32	159	1067	5	54	4	40	101	2	8
RTOR Reduction (vph)	0	0	14	0	0	0	0	0	38	0	9	0
Lane Group Flow (vph)	21	963	18	159	1072	0	27	31	2	101	1	0
Turn Type	Prot	NA	Perm	Prot	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		3	3	
Permitted Phases	•		2						4			
Actuated Green, G (s)	3.4	66.7	66.7	10.9	74.2		6.9	6.9	6.9	11.5	11.5	
Effective Green, g (s)	3.4	66.7	66.7	10.9	74.2		6.9	6.9	6.9	11.5	11.5	
Actuated g/C Ratio	0.03	0.56	0.56	0.09	0.62		0.06	0.06	0.06	0.10	0.10	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	50	2826	879	311	3142		92	186	91	169	298	
v/s Ratio Prot	0.01	0.19		c0.05	c0.21		c0.02	0.01		c0.06	0.00	
v/s Ratio Perm			0.01						0.00	*****		
v/c Ratio	0.42	0.34	0.02	0.51	0.34		0.29	0.17	0.03	0.60	0.00	
Uniform Delay, d1	57.3	14.6	12.0	52.0	11.1		54.2	53.8	53.4	52.0	49.1	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	5.6	0.3	0.0	1.4	0.1		1.8	0.4	0.1	5.6	0.0	
Delay (s)	62.9	14.9	12.0	53.4	11.1		56.0	54.2	53.5	57.6	49.1	
Level of Service	E	В	В	D	В		E	D	D	E	D	
Approach Delay (s)	_	15.8	_	_	16.6		_	54.4	_	_	56.8	
Approach LOS		В			В			D			E	
Intersection Summary												
HCM 2000 Control Delay			19.6	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capa	city ratio		0.40	,,,	2.11. 2000	_0,0101	- 5, 1,00		0			
Actuated Cycle Length (s)	ung rano		120.0	S	um of lost	t time (s)			24.0			
Intersection Capacity Utiliza	ation		57.1%		CU Level				24.0 B			
Analysis Period (min)	AUVII		15	10	O LUYGI (	J. OCI VICE	,					
c Critical Lane Group			13									
5 Sittodi Cario Group												

## Intersection: 1: I-95 SB On-Ramp & Route 54

Movement	WB	B35	SB
Directions Served	L	T	LT
Maximum Queue (ft)	87	10	42
Average Queue (ft)	35	0	11
95th Queue (ft)	69	11	35
Link Distance (ft)	98	151	103
Upstream Blk Time (%)	0		
Queuing Penalty (veh)	0		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

## Intersection: 2: I-95 NB On-Ramp & Route 54

Movement	EB	E8	EB	WB	WB
Directions Served	L	T	Т	Т	Т
Maximum Queue (ft)	116	88	34	4	21
Average Queue (ft)	43	32	5	0	1
95th Queue (ft)	89	60	23	2	12
Link Distance (ft)		179	179	146	146
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)	300				
Storage Blk Time (%)					
Queuing Penalty (veh)					

## Intersection: 3: Route 54 & Woodside Lane

Movement	EB	WB	SB	SB
Directions Served	L	TR	L	R
Maximum Queue (ft)	7	15	55	76
Average Queue (ft)	0	1	18	39
95th Queue (ft)	4	9	41	63
Link Distance (ft)		23	891	
Upstream Blk Time (%)		0		
Queuing Penalty (veh)		0		
Storage Bay Dist (ft)	225			200
Storage Blk Time (%)				
Queuing Penalty (veh)				

## Intersection: 4: Route 54 & Providence Church Road

Movement	EB	_\$B	
Directions Served	T	LR	
Maximum Queue (ft)	4	4	
Average Queue (ft)	0	0	
95th Queue (ft)	4	2	
Link Distance (ft)	23	1232	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

## Intersection: 5: Goddins Hill Road & Route 54

Movement	NB
Directions Served	LR
Maximum Queue (ft)	63
Average Queue (ft)	24
95th Queue (ft)	51
Link Distance (ft)	529
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

## Intersection: 22: Route 54

Movement	SW	B24
Directions Served	R	Т
Maximum Queue (ft)	184	33
Average Queue (ft)	54	1
95th Queue (ft)	135	17
Link Distance (ft)	166	852
Upstream Blk Time (%)	1	
Queuing Penalty (veh)	0	
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 31: Route 54

Movement	EB	EB	WB	WB	WB
Directions Served	T	R	Т	Т	T
Maximum Queue (ft)	7	345	30	36	3
Average Queue (ft)	0	120	13	7	0
95th Queue (ft)	7	274	33	28	3
Link Distance (ft)	610	610	155	155	155
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 32: I-95 SB On-Ramp

Movement	SE	
Directions Served	L	
Maximum Queue (ft)	206	
Average Queue (ft)	83	
95th Queue (ft)	155	
Link Distance (ft)	215	
Upstream Blk Time (%)	0	
Queuing Penalty (veh)	1	
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	UL	Т	T	Т	R	UL	L	T	Т	TR	L	LT
Maximum Queue (ft)	60	195	274	327	124	79	104	129	144	162	15	102
Average Queue (ft)	17	81	88	162	12	23	51	57	66	74	1	40
95th Queue (ft)	47	151	225	276	64	62	89	109	122	138	13	84
Link Distance (ft)		632	632	632				610	610	610		352
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225				150	375	375				250	
Storage Blk Time (%)		0		9								
Queuing Penalty (veh)		0		3								

## Intersection: 34: Hill Carter Parkway & England Street/Route 54

Movement	NB	NB	SB	SB	SB
Directions Served	T	R	L	Ť	TR
Maximum Queue (ft)	2	56	156	25	32
Average Queue (ft)	0	16	71	3	7
95th Queue (ft)	2	40	132	16	27
Link Distance (ft)	352			429	429
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		250	200		
Storage Blk Time (%)			0		
Queuing Penalty (veh)			0		

### **Network Summary**

Network wide Queuing Penalty: 4

Intersection													
Int Delay, s/veh	2.3	•••											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		44		ሻ	44						स	7	
Traffic Vol, veh/h	0	589	0	129	1036	0	0	0	0	30	1	247	
Future Vol, veh/h	Õ	589	Ŏ	129	1036	Ŏ	Õ	Õ	Õ	30	1	247	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	
RT Channelized	-	-	None	-		None			None	-	-	Free	
torage Length	-		-	0	-	-	-	-	_	-	-	0	
eh in Median Storage	.# -	0	_	-	0	_	-	0	_		0		
Grade, %	-	0	-	_	0	_	_	0	_	-	0	-	
eak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93	
leavy Vehicles, %	2	13	2	2	5	2	2	2	2	2	2	10	
lvmt Flow	0	633	0	139	1114	0	0	0	0	32	1	266	
lajor/Minor f	Major1		N	vlajor2					ı	/linor2			
onflicting Flow All	-	0		633	0	0			<u> </u>	1709	2025		
Stage 1		•		-	-	-				1392	1392		
Stage 2	_	_	_	_	_	-				317	633	-	
itical Hdwy	_	_	_	4.14	_	_				6.84	6.54	_	
itical Hdwy Stg 1				•						5.84	5.54	_	
itical Hdwy Stg 2	_	_	_	_	_	_				5.84	5.54	-	
llow-up Hdwy	_	_	_	2.22	-	_				3.52	4.02	-	
ot Cap-1 Maneuver	0	_	0	946	-	0				82	57	0	
Stage 1	0	-	0	-	-	0				196	207	0	
Stage 2	0	-	0	-	-	0				711	472	0	
atoon blocked, %		-			-								
ov Cap-1 Maneuver	-	-	-	946	-	-				70	0	-	
ov Cap-2 Maneuver	-	-	-	-	-	-				70	0	-	
Stage 1	-	-	-	-	-	-				196	0	-	
Stage 2	-	-	-	-	-	-				606	0	-	
pproach	EB			WB						SB			
CM Control Delay, s	0			1						96.4			
CM LOS										F			
linor Lane/Major Mvm	it	EBT	WBL	WBT S	SBLn1	SBLn2							
apacity (veh/h)		-	946	-	70	-							
CM Lane V/C Ratio		-	0.147	-	0.476	-							
CM Control Delay (s)		-	9.5	-	96.4	0							
ICM Lane LOS			Α	-	F	Α							
HCM 95th %tile Q(veh)	)	-	0.5	-	1.9	-							

Synchro 11 Report Page 1 HCM 6th TWSC GS

Intersection										
Int Delay, s/veh	0									
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	NEL	NER
Lane Configurations	34	朴			44	7				7
Traffic Vol, veh/h	254	370	0	0	339	27	0	0	0	226
Future Vol, veh/h	254	370	0	0	339	27	0	0	0	226
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free
RT Channelized	•	-	None	-	•	Yield	-	-	•	Free
Storage Length	300	-	-	-	-	0	-	-	-	0
Veh in Median Storage,	# -	0	-	-	0	-	0	-	0	-
Grade, %	-	0	-	-	0	-	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	20	5	2	2	2	16	2	2	2	6
Mvmt Flow	273	398	0	0	365	29	0	0	0	243
Major/Minor M	finor2		p	Vajor2						
Conflicting Flow All	365	365	-	-	-	0				
Stage 1	365	365	_	_	_	-				
Stage 2	0	0		_	_					
Critical Hdwy	7.2	6.6	_	_	-	_				
Critical Hdwy Stg 1	6.2	5.6	-		-	-				
Critical Hdwy Stg 2	-		_	_	_	-				
Follow-up Hdwy	3.7	4.05	-	-	-	-				
Pot Cap-1 Maneuver	562	555	0	0	-					
Stage 1	622	614	0	0	-	-				
Stage 2	-	-	0	0	-	-				
Platoon blocked, %					-	•				
Mov Cap-1 Maneuver	562	0	-	-	-	-				
Mov Cap-2 Maneuver	562	0	-	-	-	-				
Stage 1	622	0	-	-	-	-				
Stage 2	-	0	-	-	-	-				
Approach	ЕВ			WB						
HCM Control Delay, s				0						
HCM LOS	-									
Minor Lane/Major Mvmt		EBLn1 (	EBLn2 F	EBLn3	WBT	WBR				
Capacity (veh/h)		562		-	-	-				
HCM Lane V/C Ratio		0.486			-					
HCM Control Delay (s)		17.3	-	_	-	_				
HCM Lane LOS		C	_	_	-	_				
HCM 95th %tile Q(veh)		2.6	-		-					

Intersection							
Int Delay, s/veh	3.6		,				
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	7	<b>†</b>	<b>1</b>	***	) T	7	
Traffic Vol, veh/h	165	317	239	10	25	105	
Future Vol, veh/h	165	317	239	10	25	105	
Conflicting Peds, #/hr		0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized		None	-	None	-	None	
Storage Length	225	-	-	-	0	200	
Veh in Median Storag	e,# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	87	87	87	87	87	87	
Heavy Vehicles, %	3	7	2	28	6	2	
Mvmt Flow	190	364	275	11	29	121	
Major/Minor	Major1	ł	Major2	1	Minor2		
Conflicting Flow All	286	0	-	0	1025	281	
Stage 1	-			•	281	-	
Stage 2	-	-	-	-	744	-	
Critical Hdwy	4.13	-	-	-	6.46	6.22	
Critical Hdwy Stg 1	-	-	•	-	5.46	•	
Critical Hdwy Stg 2	-	-	-	-	5.46	-	
Follow-up Hdwy	2.227	-	-	-	3.554	3.318	
Pot Cap-1 Maneuver	1270	-		-	256	758	
Stage 1	-	-	-	-	757	-	
Stage 2	-	-	-	-	463	-	
Platoon blocked, %		•	-	-			
Mov Cap-1 Maneuver		-	-	-	218	758	
Mov Cap-2 Maneuver	-	•	-	-	218	-	
Stage 1	-	-	-	-	643	-	
Stage 2	-	-	-	-	463	-	
Approach	EB		WB		SB		
HCM Control Delay, s			0		13.2		
HCM LOS			="		В		
					_		
Minor Long/Major Ma	mŧ	EDI	EDT	WDT	MPD	CDI 54 G	מת ום:
Minor Lane/Major Mvr	IIL	EBL	EBT	WBT	_	SBLn1 S	
Capacity (veh/h)		1270	-	-	-	218	758
HCM Control Doloy (c	١	0.149	-	-	-	0.132	
HCM Lang LOS	)	8.3	-	-	-	24 C	10.6
HCM Lane LOS HCM 95th %tile Q(veh	•1	A 0.5	-	•	•	0.4	B 0.6
HOW BOTH WHE MAKE	')	0.5	-	-	-	0.4	0.0

	-						* *
Intersection							
Int Delay, s/veh	0.1						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	1	<b>†</b>	4		A		
Traffic Vol, veh/h	1	342	246	2	1	3	
Future Vol, veh/h	1	342	246	2	1	3	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized		None		None		None	
Storage Length	0	-	-	-	0	_	
Veh in Median Storage	e,# -	0	0	_	0	_	
Grade, %	•	0	0	_	0	_	
Peak Hour Factor	87	87	87	87	87	87	
Heavy Vehicles, %	2	7	2	2	2	2	
Mvmt Flow	1	393	283	2	1	3	
Major/Minor	Major1		Major2	,	Minor2		
Conflicting Flow All	285	0	viajoiz_	0	679	284	
Stage 1	200	0		-	284	204	
Stage 2	_			_	395	_	
Critical Hdwy	4.12	-	-	-	6.42	6.22	
Critical Hdwy Stg 1	4.12	_	_	_	5.42	0.22	
Critical Hdwy Stg 2			•	•	5.42	-	
Follow-up Hdwy	2.218	-	-	-	3.518		
Pot Cap-1 Maneuver	1277	_	_	_	417	755	
Stage 1	1411	_	_	-	764	755	
Stage 2	_	_	_	_	681	_	
Platoon blocked, %					001		
Mov Cap-1 Maneuver	1277	_			417	755	
Mov Cap-2 Maneuver	1277		_		417	700	
Stage 1		_	_	_	763	_	
Stage 2		_		_	681	_	
Olage 2					QUI		
Approach	EB		WB		SB		
HCM Control Delay, s	0		0		10.8		
HCM LOS	U		U		10.8 B		
TIOW LOS					D		
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR :	SBLn1	
Capacity (veh/h)		1277		-		628	•••
HCM Lane V/C Ratio		0.001	_			0.007	
HCM Control Delay (s)	1	7.8	-	-	_	10.8	
HCM Lane LOS	,	A			_	В	
HCM 95th %tile Q(veh	1	0	_	_		0	
	,	•					

<del> </del>						
Intersection						
Int Delay, s/veh	1.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	T T	*****	4	W	HUIN
Traffic Vol, veh/h	308	34	29	221	27	56
Future Vol, veh/h	308	34	29	221	27	56
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None		None	•	None
	•	150	•	None	-	NONE
Storage Length	# 0	100	-	_	0	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	0.4	0.4	0	0	
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	7	6	4	3	13	5
Mvmt Flow	338	37	32	243	30	62
Major/Minor N	/lajor1		Major2	1	Minor1	
Conflicting Flow All	0	0	375	0	645	338
Stage 1	-	-	-	-	338	-
Stage 2					307	_
Critical Hdwy			4.14		6.53	6.25
•	-	-	4.14	•	5.53	0.20
Critical Howy Stg 1	-	-	-	-		-
Critical Hdwy Stg 2	-	-	0.000	-	5.53	0.045
Follow-up Hdwy	-	-	2.236	-	3.617	
Pot Cap-1 Maneuver	-	-	1173	-	420	697
Stage 1	•	-	-	-	698	-
Stage 2	-	-	-	-	722	-
Platoon blocked, %	-	•		•		
Mov Cap-1 Maneuver	-	-	1173	-	407	697
Mov Cap-2 Maneuver	-	_	_	_	407	_
Stage 1		-		-	698	-
Stage 2	_				699	-
-1030 =					500	
Annroach	EB		WB		NB	
Approach						
HCM Control Delay, s	0		0.9		12.6	
HCM LOS					В	
Minor Lane/Major Mvm	t I	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		566	-	-	1173	-
HCM Lane V/C Ratio		0.161			0.027	-
HCM Control Delay (s)		12.6	-	-	8.2	0
HCM Lane LOS		В	-	_	Α	Α
HCM 95th %tile Q(veh)		0.6			0.1	
		5.5				

Existing (2023) Conditions Timing Plan: PM Peak Hour

Hickory Grove
34: Hill Carter Parkway & England Street/Route 54

	Þ	-	7	*	♣	1	<b>†</b>	-	-	<b>↓</b>	
Lane Group	EBL	EBT	EBR	WBL.	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Group Flow (vph)	40	1097	107	223	1172	68	77	129	54	22	
v/c Ratio	0.31	0.40	0.12	0.60	0.38	0.47	0.26	0.48	0.38	0.08	
Control Delay	58.5	18.1	1.9	57.4	14.9	62.1	52.3	13.7	59.8	28.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	58.5	18.1	1.9	57.4	14.9	62.1	52.3	13.7	59.8	28.0	
Queue Length 50th (ft)	30	179	0	86	175	56	31	0	41	2	
Queue Length 95th (ft)	65	262	19	123	257	105	55	52	81	16	
Internal Link Dist (ft)		601			622		348			415	
Turn Bay Length (ft)	225		150	375		250		250	200		
Base Capacity (vph)	280	2768	923	543	3058	268	541	377	191	357	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.14	0.40	0.12	0.41	0.38	0.25	0.14	0.34	0.28	0.06	
Intersection Summary											

	<b></b>	۶	<b>→</b>	•	F	•	+	A.	4	†	~	<u> </u>
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		A	ተተተ	7		ā'n	ተተኈ		7	414	7	7
Traffic Volume (vph)	26	11	1009	98	61	144	1053	25	125	8	119	50
Future Volume (vph)	26	11	1009	98	61	144	1053	25	125	8	119	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0	6.0
Lane Util. Factor		1.00	0.91	1.00		0.97	0.91		0.91	0.91	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00		1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00		0.95	0.96	1.00	0.95
Satd. Flow (prot)		1770	5085	1583		3433	5068		1610	3247	1583	1770
Flt Permitted		0.95	1.00	1.00		0.95	1.00		0.95	0.96	1.00	0.95
Satd. Flow (perm)		1770	5085	1583		3433	5068		1610	3247	1583	1770
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	28	12	1097	107	66	157	1145	27	136	9	129	54
RTOR Reduction (vph)	0	0	0	50	0	0	1	0	0	0	117	0
Lane Group Flow (vph)	0	40	1097	57	0	223	1171	0	68	77	12	54
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA		Split	NA	Perm	Split
Protected Phases	5	5	2		1	1	6		. 4	4		. 3
Permitted Phases				2							4	
Actuated Green, G (s)		7.3	64.1	64.1		13.1	69.9		10.8	10.8	10.8	8.0
Effective Green, g (s)		7.3	64.1	64.1		13.1	69.9		10.8	10.8	10.8	8.0
Actuated g/C Ratio		0.06	0.53	0.53		0.11	0.58		0.09	0.09	0.09	0.07
Clearance Time (s)		6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0	6.0
Vehicle Extension (s)		3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0
Lane Grp Cap (vph)		107	2716	845		374	2952		144	292	142	118
v/s Ratio Prot		0.02	0.22			c0.06	c0.23		c0.04	0.02		c0.03
v/s Ratio Perm				0.04							0.01	
v/c Ratio		0.37	0.40	0.07		0.60	0.40		0.47	0.26	0.08	0.46
Uniform Delay, d1		54.2	16.6	13.5		50.9	13.6		51.9	50.9	50.1	53.9
Progression Factor		1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00
Incremental Delay, d2		2.2	0.4	0.2		2.6	0.1		2.4	0.5	0.2	2.8
Delay (s)		56.3	17.1	13.7		53.5	13.7		54.3	51.4	50.3	56.7
Level of Service		Ε	В	В		D	В		D	D	D	Е
Approach Delay (s)			18.0				20.0			51.6		
Approach LOS			В				С			D		
Intersection Summary												
HCM 2000 Control Delay			23.0	H	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capaci	ity ratio		0.45									
Actuated Cycle Length (s)			120.0	S	um of los	t time (s)			24.0			
Intersection Capacity Utilizati	on		60.2%	IC	U Level	of Service	2		В			
Analysis Period (min)			15									
c Critical Lane Group												

	<b>↓</b>	1
Movement	SBT	SBR
Lane Configurations	<b>†</b> }	-
Traffic Volume (vph)	6	14
Future Volume (vph)	6	14
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	6.0	
Lane Util. Factor	0.95	
Frt	0.90	
Flt Protected	1.00	
Satd. Flow (prot)	3177	
Flt Permitted	1.00	
Satd. Flow (perm)	3177	
Peak-hour factor, PHF	0.92	0.92
Adj. Flow (vph)	7	15
RTOR Reduction (vph)	14	0
Lane Group Flow (vph)	8	0
Turn Type	NA	
Protected Phases	3	
Permitted Phases		
Actuated Green, G (s)	8.0	
Effective Green, g (s)	8.0	
Actuated g/C Ratio	0.07	
Clearance Time (s)	6.0	
Vehicle Extension (s)	3.0	
Lane Grp Cap (vph)	211	
v/s Ratio Prot	0.00	
v/s Ratio Perm		
v/c Ratio	0.04	
Uniform Delay, d1	52.4	
Progression Factor	1.00	
Incremental Delay, d2	0.1	
Delay (s)	52.5	
Level of Service	D D	
Approach Delay (s)	55.5	
Approach LOS	55.5 E	
• •	_	
Intersection Summary		

## Intersection: 1: I-95 SB On-Ramp & Route 54

Movement	WB	SB	B28
Directions Served	L	LT	T
Maximum Queue (ft)	79	104	50
Average Queue (ft)	31	32	3
95th Queue (ft)	64	81	47
Link Distance (ft)	98	103	987
Upstream Blk Time (%)	0	2	
Queuing Penalty (veh)	0	0	
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

### Intersection: 2: I-95 NB On-Ramp & Route 54

Movement	EB	EB	EB	WB	WB
Directions Served	L	Т	Т	Т	Т
Maximum Queue (ft)	140	104	76	1	24
Average Queue (ft)	62	53	25	0	2
95th Queue (ft)	116	89	56	1	12
Link Distance (ft)		179	179	146	146
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)	300				
Storage Blk Time (%)					
Queuing Penalty (veh)					

### Intersection: 3: Route 54 & Woodside Lane

Movement	EB	B36	WB	SB	SB
Directions Served	L	T	TR	L.	R
Maximum Queue (ft)	18	3	34	33	58
Average Queue (ft)	1	0	2	10	29
95th Queue (ft)	11	3	17	27	44
Link Distance (ft)		146	23	891	
Upstream Blk Time (%)			0		
Queuing Penalty (veh)			0		
Storage Bay Dist (ft)	225				200
Storage Blk Time (%)					
Queuing Penalty (veh)					

### Intersection: 4: Route 54 & Providence Church Road

Movement	EB	SB
Directions Served	Т	LR
Maximum Queue (ft)	6	4
Average Queue (ft)	0	0
95th Queue (ft)	0	2
Link Distance (ft)	23	1232
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

## Intersection: 5: Goddins Hill Road & Route 54

### Intersection: 22: Route 54

Movement	EB	SW	B24
Directions Served	T	R	Т
Maximum Queue (ft)	13	261	160
Average Queue (ft)	1	114	14
95th Queue (ft)	9	235	91
Link Distance (ft)	151	166	852
Upstream Blk Time (%)		8	
Queuing Penalty (veh)		0	
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 31: Route 54

Movement	EB	WB	WB	WB
Directions Served	R	Т	T	T
Maximum Queue (ft)	298	38	47	54
Average Queue (ft)	95	17	13	2
95th Queue (ft)	225	38	40	19
Link Distance (ft)	610	155	155	155
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 32: I-95 SB On-Ramp

Movement	SE	
Directions Served	L	
Maximum Queue (ft)	111	
Verage Queue (ft)	42	
35th Queue (ft)	82	
ink Distance (ft)	215	
Jpstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection:	34: Hill Ca	arter Parkwa	/ & England	Street/Route 54
111101000110111	0 11 1 1111 00	41 COI 1 CALICATOR)		

Movement	EΒ	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	UL	Т	Т	Т	R	UL	L	T	T	TR	L	LT
Maximum Queue (ft)	94	278	261	320	190	127	132	149	164	193	140	178
Average Queue (ft)	31	140	100	151	34	63	67	68	83	99	15	91
95th Queue (ft)	71	235	220	258	110	112	115	127	145	171	80	154
Link Distance (ft)		632	632	632				610	610	610		352
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225				150	375	375				250	
Storage Blk Time (%)		1		8								
Queuing Penalty (veh)		0		8								

## Intersection: 34: Hill Carter Parkway & England Street/Route 54

Movement	NB	NB	SB	SB	SB
Directions Served	T	R	L	Т	TR
Maximum Queue (ft)	75	98	113	38	41
Average Queue (ft)	3	37	42	6	11
95th Queue (ft)	35	73	87	25	35
Link Distance (ft)	352			429	429
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		250	200		
Storage Blk Time (%)					
Queuing Penalty (veh)					

## **Network Summary**

Network wide Queuing Penalty: 9

APPENDIX D: SYNCHRO & SIMTRAFFIC OUTPUT - NO-BUILD (2027) CONDITIONS

Intersection													
Int Delay, s/veh	2.5												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	•	44		ሻ	44						सै	7	
Traffic Vol, veh/h	0	316	0	308	952	0	0	0	0	10	1	242	
Future Vol, veh/h	0	316	0	308	952	0	0	0	0	10	1	242	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	
RT Channelized	-		None			None	-	-	None	-	_	Free	
Storage Length	-	-	-	0	-	-	-	-	-	-	-	0	
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	•	0	•	
Grade, %	-	0	-	-	0	_	-	0	-	-	0	-	
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93	
Heavy Vehicles, %	2	13	2	2	5	2	2	2	2	2	2	10	
Mvmt Flow	0	340	0	331	1024	0	0	0	0	11	1	260	
Major/Minor N	Major1		ı	Major2						vlinor2			
Conflicting Flow All		0	<u> </u>	340	0	0			<u>'</u>	1856	2026		
Stage 1	_	-	_	0+0	-	-				1686	1686	_	
Stage 2		_		_	_	_				170	340		
Critical Hdwy	_			4.14	_	_				6.84	6.54	_	
Critical Hdwy Stg 1	-		_	7.17						5.84	5.54		
Critical Hdwy Stg 2	_	_	_	_	_	_				5.84	5.54	_	
Follow-up Hdwy	_	_	_	2.22	_	_				3.52	4.02		
Pot Cap-1 Maneuver	0		0	1216	_	0				65	57	0	
Stage 1	0	_	0	1210	_	0				135	149	0	
Stage 2	0		0	_	_	0				843	638	ő	
Platoon blocked, %	•		v			•				010	000	v	
Mov Cap-1 Maneuver	_			1216		_				47	0	_	
Mov Cap-2 Maneuver	_		_	1210		_				47	ő	_	
Stage 1	_	_		_	_	_				135	0	-	
Stage 2	_	_		_	_	_				614	0	_	
										V17	•		
Approach	EB			WB						SB			
HCM Control Delay, s	0			2.2						105.6			
HCM LOS	•									F			
										·			
Minor Lane/Major Mvm	it	EBT	WBL	WBT:	SBLn1	SBLn2							
Capacity (veh/h)			1216	-	47	_							
HCM Lane V/C Ratio			0.272		0.252								
HCM Control Delay (s)		_	9.1		105.6	0							
HCM Lane LOS		-	A		F	Ä							
HCM 95th %tile Q(veh)	ì	_	1.1		0.8	-							
HOW ADIU WING M(AGU)	)	-	1.1	-	U.8	-							

Intersection												
Int Delay, s/veh	0										 	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	NEL	NER		
Lane Configurations	ሻ	朴			朴	7				7		
Traffic Vol, veh/h	160	166	0	0	585	36	0	0	0	92		
Future Vol., veh/h	160	166	0	0	585	36	0	0	0	92		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0		
Sign Control	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free		
RT Channelized		-	None	•	-	Yield	_	-	-	Free		
Storage Length	300	-	-	-	-	0	-	-	-	0		
Veh in Median Storage,	# -	0	-	-	0	-	0	-	0	-		
Grade, %	-	0	-	-	0	-	0	-	0	-		
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93		
Heavy Vehicles, %	20	5	2	2	2	16	2	2	2	6		
Mvmt Flow	172	178	0	0	629	39	0	0	0	99		
Major/Minor M	linor2			Major2								 
Conflicting Flow All	629	629	-	-	-	0						 
Stage 1	629	629		-	-	-						
Stage 2	0	0	-	_	-	-						
Critical Hdwy	7.2	6.6	-	-	-	-						
Critical Hdwy Stg 1	6.2	5.6	•	-	-	-						
Critical Hdwy Stg 2	-	-	-	-	-	-						
Follow-up Hdwy	3.7	4.05	-	-	-	-						
ot Cap-1 Maneuver	375	391	0	0	-	-						
Stage 1	447	466	0	0	-	-						
Stage 2	-	-	0	0	-	-						
Platoon blocked, %					-	-						
Nov Cap-1 Maneuver	375	0	-	-	-	-						
Mov Cap-2 Maneuver	375	0	-		-	-						
Stage 1	447	0	-	-	-	-						
Stage 2	-	0	-	-	-	-						
Approach	EB			WB								 
HCM Control Delay, s				0								
HCM LOS	-											
6 4 Co		EDI 4:		-0: ^	MOT	Wee						
Minor Lane/Major Mvmt		EBLn1 E	BLN2 b	:BLN3	WBT	WBR						
Capacity (veh/h)		375	-	-	-	-						
HCM Lane V/C Ratio		0.459	-	-	-	-						
HCM Control Delay (s)		22.5	-	-	-	-						
HCM Lane LOS		С	•	-	-	-						
HCM 95th %tile Q(veh)		2.3										

Intersection							
Int Delay, s/veh	5.7	-	-				
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	ሻ	<b>^</b>	7		ሻ	7	
Traffic Vol, veh/h	44	162	312	21	59	248	
Future Vol, veh/h	44	162	312	21	59	248	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	225	-	-	-	0	200	
Veh in Median Storage	e,# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	87	87	87	87	87	87	
Heavy Vehicles, %	3	7	2	28	6	2	
Mvmt Flow	51	186	359	24	68	285	
Major/Minor	Major1	Ŋ	Major2	ı	Minor2		
Conflicting Flow All	383	0	-	0	659	371	
Stage 1		-	-	-	371	-	
Stage 2	-	-	-	-	288	-	
Critical Hdwy	4.13	-	-	-	6.46	6.22	
Critical Hdwy Stg 1	•	•	•	•	5.46	-	
Critical Hdwy Stg 2	-	-	-	-	5.46	-	
Follow-up Hdwy	2.227	-	-	-		3.318	
Pot Cap-1 Maneuver	1170	•	-	-	422	675	
Stage 1	-	-	-	-	689	-	
Stage 2	•	-	•	-	752	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver		-	-	-	403	675	
Mov Cap-2 Maneuver	-	-	-	-	403	-	
Stage 1	-	-	-	-	659	-	
Stage 2	-	-	-	-	752		
Approach	EB		WB		SB		
HCM Control Delay, s	1.8		0		14.5		
HCM LOS					В		
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	SBLn1S	BLn2
Capacity (veh/h)	,	1170			*****	403	675
HCM Lane V/C Ratio		0.043	_	_		0.168	
HCM Control Delay (s	)	8.2	_	_	_	15.7	14.2
HCM Lane LOS	,	Α			-	C	В
HCM 95th %tile Q(veh	1)	0.1	_	_	_	0.6	2.1
	'/	3.1				3.0	

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7	<b>A</b>	4	*****	N/	3311
Traffic Vol, veh/h	2	219	330	1	1 1	3
Future Vol, veh/h	2	219	330	1	1	3
Conflicting Peds, #/hr	0	0	0	0	Ó	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	riee .	None	Liee	None	Stop	None
	0		-	NOTIE		None
Storage Length		-	-	-	0	-
Veh in Median Storage	∋,# -	0	0	-	0	-
Grade, %	•	0	0	07	0	07
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	2	7	2	2	2	2
Mvmt Flow	2	252	379	1	1	3
Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	380	0	viajoiz -	0	636	380
Stage 1	300	U	-	-	380	300
•	-	-	-			-
Stage 2	440	•	•	-	256	e 00
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	•	-	5.42	-
Critical Hdwy Stg 2	•	-	-	•	5.42	
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1178	-	•	-	442	667
Stage 1	-	-	-	-	691	-
Stage 2	-	-	-	-	787	-
Platoon blocked, %			•	-		
Mov Cap-1 Maneuver	1178	-	_	-	441	667
Mov Cap-2 Maneuver		_	_	_	441	_
Stage 1	_	_		_	690	
Stage 2	_				787	_
Glaye Z	-	-	-	-	101	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		11.1	
HCM LOS					В	
					·	
Minor Lane/Major Mun	nt	EBL	EBT	WBT	WBR :	SBI n1
Minor Lane/Major Mvn	R		ĖD1	VVDI	VV DIK	
Capacity (veh/h)		1178	-	-	-	591
HCM Lane V/C Ratio		0.002	•	•	•	0.008
HCM Control Delay (s)	1	8.1	-	-	-	11.1
HCM Lane LOS		Α	-	-	-	В
HCM 95th %tile Q(veh	)	0	-	•	-	0

2.2					
	EDD	MOL	MOT	NDI	MPD
		WBL	_		NBR
		0.4			O.E.
					25
					25
-	-			-	0
rree				•	Stop
-		-			None
		-	-		-
	-	-			-
	-	-			-
					91
					5
170	59	89	288	30	27
Maior1	N	Maior2		Minor1	
					170
	U	229			
-	-	-			-
-	-	-			6.05
•	-	4.14			6.25
-	-	-	-		-
-	-	-	-		-
•	-		-		
-	-	1327	-		866
-	-	-	-		-
-	-	-	-	609	•
-	-		-		
-		1327	-	391	866
-	-	-	-	391	-
-	-	_	-	834	_
-	_	-	-		-
				500	
		14.00		A	
0		1.9			
				В	
nt N	NBLn1	€BT	EBR	WBL	WBT
	531		-	1327	
	0.108	-	_	0.067	_
		_	-	0.007	_
		_	_	7.0	Λ
)	12.6	-	•	7.9	0
		-	-	7.9 A 0.2	0 A
	- - - EB 0	155 54 155 54 0 0 Free Free - None - 150 4, # 0 - 91 91 7 6 170 59  Major1 1 0 0	155 54 81 0 0 0 0 Free Free Free - None - 150 150 150 91 91 91 7 6 4 170 59 89  Major1 Major2 0 0 229 4.14 2.236 - 1327	155 54 81 262 155 54 81 262 0 0 0 0 0 Free Free Free Free Free - None - None - 150 150 0 91 91 91 91 7 6 4 3 170 59 89 288  Major1 Major2  0 0 229 0 4.14 2.236 1327 1327 1327 1327 1327 1327 1327 1327 1327 1327 1327 1327 1327 1327 1327	155 54 81 262 27 155 54 81 262 27 0 0 0 0 0 0 0  Free Free Free Free Stop - None - None 150 0 0 0 0 0 91 91 91 91 91 91 7 6 4 3 13 170 59 89 288 30  Major1 Major2 Minor1 0 0 229 0 636 170 466 - 4.14 - 6.53 466 - 4.14 - 6.53 5.53 2.236 - 3.617 - 1327 - 425 1327 - 425 1327 - 425 1327 - 391 1327 - 391 1327 - 391 391 391 1327 - 391 391 834 834 834 560  EB WB NB 0 1.9 12.6 B

No-Build (2027) Conditions Timing Plan: AM Peak Hour

	*	-	7	•	4		<b>†</b>	1	-	<b>↓</b>	
Lane Group	EBL	EBT	EBR	WBL.	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Group Flow (vph)	22	1023	33	168	1140	29	34	42	108	11	
v/c Ratio	0.15	0.40	0.04	0.47	0.37	0.22	0.12	0.15	0.61	0.02	
Control Delay	45.3	17.6	0.1	46.5	12.5	46.6	43.2	1.1	57.6	0.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	45.3	17.6	0.1	46.5	12.5	46.6	43.2	1.1	57.6	0.0	
Queue Length 50th (ft)	13	153	0	53	115	19	11	0	66	0	
Queue Length 95th (ft)	38	208	0	83	215	49	27	0	123	0	
Internal Link Dist (ft)		601			622		348			415	
Turn Bay Length (ft)	225		150	375		250		250	200		
Base Capacity (vph)	230	2541	873	446	3077	289	585	419	194	718	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.10	0.40	0.04	0.38	0.37	0.10	0.06	0.10	0.56	0.02	
Intersection Summary											

	A	-	7	•	4	4	1	1	~	1	1	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ã	<b>^</b>	7	<b>ሕ</b> ግ	ተተኈ		ሽ	41	7	7	₹ħ	
Traffic Volume (vph)	20	941	30	155	1042	6	53	5	39	99	2	8
Future Volume (vph)	20	941	30	155	1042	6	53	5	39	99	2	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0	6.0	6.0	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91		0.91	0.91	1.00	1.00	0.95	
Frt	1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85	1.00	0.88	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00	0.95	1.00	
Satd. Flow (prot)	1770	5085	1583	3433	5081		1610	3252	1583	1770	3105	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00	0.95	1.00	
Satd. Flow (perm)	1770	5085	1583	3433	5081		1610	3252	1583	1770	3105	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	22	1023	33	168	1133	7	58	5	42	108	2	9
RTOR Reduction (vph)	0	0	17	0	0	0	0	0	39	0	10	0
Lane Group Flow (vph)	22	1023	16	168	1140	Õ	29	34	3	108	1	0
Turn Type	Prot	NA	Perm	Prot	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2	1 (1111	1	6		4	4	1 OIIII	3	3	
Permitted Phases	Ŭ	_	2	'	v		-	7	4	·	•	
Actuated Green, G (s)	3.3	48.8	48.8	10.3	55.8		6.8	6.8	6.8	10.1	10.1	
Effective Green, g (s)	3.3	48.8	48.8	10.3	55.8		6.8	6.8	6.8	10.1	10.1	
Actuated g/C Ratio	0.03	0.49	0.49	0.10	0.56		0.07	0.07	0.07	0.10	0.10	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	
	5.0	2481	772	353	2835	·	109	221	107	178	313	
Lane Grp Cap (vph)	0.01		112	c0.05	c0.22		c0.02	0.01	107	c0.06	0.00	
v/s Ratio Prot	0.01	0.20	0.04	CU.U5	CU.22		CU.U2	0.01	0.00	CU.U6	0.00	
v/s Ratio Perm	0.00	0.44	0.01	0.40	0.40		0.07	0.45	0.00	0.64	0.00	
v/c Ratio	0.38	0.41	0.02	0.48	0.40		0.27	0.15	0.03	0.61	0.00	
Uniform Delay, d1	47.3	16.4	13.2	42.3	12.6		44.2	43.9	43.5	43.0	40.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	4.1	0.5	0.0	1.0	0.1		1.3	0.3	0.1	5.7	0.0	
Delay (s)	51.5	16.9	13.3	43.3	12.7		45.5	44.2	43.6	48.8	40.4	
Level of Service	D	В	В	D	В		D	D	D	D	D	
Approach Delay (s)		17.5			16.6			44.3			48.0	
Approach LOS		В			В			D			D	
Intersection Summary												
HCM 2000 Control Delay			19.5	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capa	city ratio		0.44						_			
Actuated Cycle Length (s)			100.0		um of lost				24.0			
Intersection Capacity Utiliza	ıtion		58.2%	IC	CU Level	of Service			В			
Analysis Period (min)			15									
c Critical Lane Group												

Intersection: 1: I-95 SB On-Ramp & Route 54

Movement	WB	B33	B35	SB
Directions Served	L	Т	Т	LT
Maximum Queue (ft)	106	5	10	49
Average Queue (ft)	38	0	0	12
95th Queue (ft)	84	6	8	39
Link Distance (ft)	98	206	151	103
Upstream Blk Time (%)	1			
Queuing Penalty (veh)	3			
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 2: I-95 NB On-Ramp & Route 54

Movement	EB	EB	EΒ	WB	WB
Directions Served	L	T	Т	Т	Т
Maximum Queue (ft)	109	76	38	6	28
Average Queue (ft)	45	31	5	0	2
95th Queue (ft)	89	57	25	4	14
Link Distance (ft)		179	179	146	146
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)	300				
Storage Blk Time (%)					
Queuing Penalty (veh)					

## Intersection: 3: Route 54 & Woodside Lane

Movement	EB	WB	SB	\$B
Directions Served	L	TR	L	R
Maximum Queue (ft)	6	27	60	71
Average Queue (ft)	0	1	20	39
95th Queue (ft)	4	14	43	62
Link Distance (ft)		23	891	
Upstream Blk Time (%)		0		
Queuing Penalty (veh)		0		
Storage Bay Dist (ft)	225			200
Storage Blk Time (%)				
Queuing Penalty (veh)				

### Intersection: 4: Route 54 & Providence Church Road

Movement	EB	SB
Directions Served	T	LR
Maximum Queue (ft)	10	3
Average Queue (ft)	0	0
95th Queue (ft)	5	2
Link Distance (ft)	23	1232
Upstream Blk Time (%)	0	
Queuing Penalty (veh)	0	
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

#### Intersection: 6: Goddins Hill Road & Route 54

Movement	NB
Directions Served	LR
Maximum Queue (ft)	64
Average Queue (ft)	26
95th Queue (ft)	52
Link Distance (ft)	529
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

## Intersection: 22: Route 54

Movement	SW	B24
Directions Served	R	Т
Maximum Queue (ft)	180	6
Average Queue (ft)	58	0
95th Queue (ft)	141	7
Link Distance (ft)	166	852
Upstream Blk Time (%)	1	
Queuing Penalty (veh)	0	
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 31: Route 54

Movement	EΒ	EB	WB	WB	WB
Directions Served	Т	R	Т	Т	T
Maximum Queue (ft)	7	391	33	41	19
Average Queue (ft)	0	149	13	7	1
95th Queue (ft)	8	297	33	29	16
Link Distance (ft)	610	610	155	155	155
Upstream Blk Time (%)		0			0
Queuing Penalty (veh)		0			0
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 32: I-95 SB On-Ramp

Movement	SE	=
Directions Served	L	_
Maximum Queue (ft)	199	9
Average Queue (ft)	94	1
95th Queue (ft)	172	2
Link Distance (ft)	215	5
Upstream Blk Time (%)	1	1
Queuing Penalty (veh)	2	2
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection:	34: Hill	Carter	Parkway	& England	Street/Route 54
			,		

Movement	EB	EB	ЕВ	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	UL	Т	Т	Т	R	UL	L	T	Т	TR	L	LT
Maximum Queue (ft)	65	182	270	331	170	92	109	158	163	188	23	98
Average Queue (ft)	17	86	99	180	18	30	55	61	68	85	1	39
95th Queue (ft)	48	154	232	285	87	72	98	121	127	155	17	81
Link Distance (ft)		632	632	632				610	610	610		352
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225				150	375	375				250	
Storage Blk Time (%)		0		12								
Queuing Penalty (veh)		0		4								

## Intersection: 34: Hill Carter Parkway & England Street/Route 54

Movement	NB	SB	SB	SB
Directions Served	R	L	Т	TR
Maximum Queue (ft)	58	153	25	31
Average Queue (ft)	17	71	1	7
95th Queue (ft)	43	128	12	26
Link Distance (ft)			429	429
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	250	200		
Storage Blk Time (%)		0		
Queuing Penalty (veh)		0		

### **Network Summary**

Network wide Queuing Penalty: 9

Intersection													
Int Delay, s/veh	3.1												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		44		ሻ	ተተ						र्स	7	
Traffic Vol, veh/h	0	631	0	137	1103	0	0	0	0	32	1	262	
Future Vol, veh/h	0	631	0	137	1103	0	0	0	0	32	1	262	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	Free	
Storage Length	-	-	-	0	-	-	-	-	-	-	•	0	
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93	
Heavy Vehicles, %	2	13	2	2	5	2	2	2	2	2	2	10	
Mvmt Flow	0	678	0	147	1186	0	0	0	0	34	1	282	
	Major1		ľ	/lajor2					N	/linor2			
Conflicting Flow All	-	0	-	678	0	0				1819	2158	•	
Stage 1	•	-	-	-	-	-				1480	1480	-	
Stage 2	-	-	-	-	-	-				339	678	-	
Critical Hdwy	•	-	-	4.14	-	•				6.84	6.54	-	
Critical Hdwy Stg 1	-	-	-	-	-	-				5.84	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-				5.84	5.54	-	
Follow-up Hdwy	•	•	•	2.22	-	-				3.52	4.02	-	
Pot Cap-1 Maneuver	0	-	0	910	-	0				69	47	0	
Stage 1	0	-	0	-	-	0				175	188	0	
Stage 2	0	•	0	•	•	0				693	450	0	
Platoon blocked, %		-		010	-					50	Λ		
Mov Cap-1 Maneuver	-	-	•	910	•					58 58	0	-	
Mov Cap-2 Maneuver	-	-	-	-	-	-				175	0	-	
Stage 1 Stage 2	-	-	•	-	-	-				581	0	-	
Stage 2	•	-	-	-	-	-				301	U	-	
Approach	EB			WB						SB			
HCM Control Delay, s	0			1.1					•	136.9		· ·	
HCM LOS	-									F			
Minor Lane/Major Mvm	t	EBT	WBL	WBT	SBLn1	SBLn2							
Capacity (veh/h)		-	910	-	58	-							
		-	0.162	-	0.612	-							
HCM Lane V/C Ratio						Λ							
HCM Lane V/C Ratio HCM Control Delay (s)		-	9.7	•	136.9	0							
		-	9. <i>7</i>	-	130.9 F	A							

				·						
Intersection										
Int Delay, s/veh	0									
Movement	EBL	EBT	E8R	WBL	WBT	WBR	SBL	SBR	NEL	NER
Lane Configurations	<b>P</b>	朴			<b>*</b>	7				7
Traffic Vol, veh/h	270	393	0	0	360	29	0	0	0	213
Future Vol, veh/h	270	393	0	0	360	29	0	0	0	213
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	Yield	-	-	-	Free
Storage Length	300	-	-	-	-	0	•	-	-	0
Veh in Median Storage,	# -	0	-	- 1	0	-	0	-	0	-
Grade, %	-	0	-	-	0	-	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	20	5	2	2	2	16	2	2	2	6
Mvmt Flow	290	423	0	0	387	31	0	0	0	229
Major/Minor N	linor2		٨	√lajor2						
Conflicting Flow All	387	387	-	-		0				
Stage 1	387	387	_	_	_	-				
Stage 2	0	0	_	_	_	-				
Critical Hdwy	7.2	6.6								
Critical Hdwy Stg 1	6.2	5.6	-	_	_	-				
Critical Hdwy Stg 2	-	-	_	-	_	_				
Follow-up Hdwy	3.7	4.05	_	-		_				
Pot Cap-1 Maneuver	544	539	0	0	_	-				
Stage 1	606	601	0	0	_	-				
Stage 2			Õ	Ô	_	_				
Platoon blocked, %			•	•	_	-				
Mov Cap-1 Maneuver	544	0	-	_		-				
Mov Cap-2 Maneuver	544	0	-	_	_	-				
Stage 1	606	0		_	_	-				
Stage 2	-	0	-	_	-	-				
3		•								
Annroach	EB			WB						
Approach										
HCM Control Delay, s				0						
HCM LOS	-									
Minor Lane/Major Mvmt	E	BLn1 E	EBLn2 E	EBLn3	WBT	WBR				
Capacity (veh/h)		544	-	-	-	-				
HCM Lane V/C Ratio		0.534	-	-	-	-				
HCM Control Delay (s)		18.9		-	•	•				
HCM Lane LOS		С	-	-	-	-				
HCM 95th %tile Q(veh)		3.1	-	-	-	-				

Intersection										
Int Delay, s/veh	3.7							 		
Movement	EBL	EBT	WBT	WBR	SBL	SBR				
Lane Configurations	7	<b>†</b>	\$		4	7		 	 	
Traffic Vol, veh/h	175	336	254	11	27	111				
Future Vol, veh/h	175	336	254	11	27	111				
Conflicting Peds, #/hr	0	0	0	0	0	0				
Sign Control	Free	Free	Free	Free	Stop	Stop				
RT Channelized	-	None	-	None	-	None				
Storage Length	225	-	-	-	0	200				
Veh in Median Storage	e,# -	0	0	-	0	-				
Grade, %	-	0	0	-	0	-				
Peak Hour Factor	87	87	87	87	87	87				
Heavy Vehicles, %	3	7	2	28	6	2				
Mvmt Flow	201	386	292	13	31	128				
Major/Minor	Major1	ľ	Major2	ı	Minor2					
Conflicting Flow All	305	0	-	0	1087	299				
Stage 1	-	-	-	-	299					
Stage 2	-	-	-	_	788	_				
Critical Hdwy	4.13	-			6.46	6.22				
Critical Hdwy Stg 1	-	-	-	-	5.46	-				
Critical Hdwy Stg 2	-	_	-	-	5.46	_				
Follow-up Hdwy	2.227	-		-	3.554	3.318				
Pot Cap-1 Maneuver	1250	-	-	-	235	741				
Stage 1	-	-	-	-	743	-				
Stage 2	-	-	-	-	441	-				
Platoon blocked, %		-	-	-						
Mov Cap-1 Maneuver	1250	-	-	-	197	741				
Mov Cap-2 Maneuver	-	-	-	-	197	-				
Stage 1	-	-	-	-	623	-				
Stage 2	-	-	-	-	441	-				
Approach	EB		WB		SB					
HCM Control Delay, s	2.9		0		14					
HCM LOS					В					
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR:	SBLn1 S	BLn2			
Capacity (veh/h)		1250	-	-	-	197	741	 		
HCM Lane V/C Ratio		0.161	-	-	-	0.450				
HCM Control Delay (s)	)	8.4	-	-	-	26.7	10.9			
HCM Lane LOS		Α	-	-	-	D	8			
HCM 95th %tile Q(veh	)	0.6	-	-	-	0.5	0.6			
•										

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
				VVDR		אסג
Lane Configurations	7	202	<b>\$</b>	2	W	2
Traffic Vol, veh/h	1	363	262	2	1	3
Future Vol, veh/h	1	363	262	2	1	3
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	_
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	2	7	2	2	2	2
Mvmt Flow	1	417	301	2	1	3
14141111 I I I I I I I I I I I I I I I I	'	-3/11	001	_	1	3
Major/Minor	Major1	N	/lajor2		Minor2	
Conflicting Flow All	303	0	-	0	721	302
Stage 1		_	_	-	302	-
Stage 2	-	_	_	_	419	_
Critical Hdwy	4.12	_	_		6.42	6.22
Critical Hdwy Stg 1	7.12			_	5.42	0.22
	-	-	-	-	5.42	-
Critical Hdwy Stg 2	2 240	-	-	-		2.240
Follow-up Hdwy	2.218	-	-	•	3.518	
Pot Cap-1 Maneuver	1258	-	-	-	394	738
Stage 1	-	-	-	-	750	-
Stage 2	-	•	-	•	664	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1258	-	-	-	394	738
Mov Cap-2 Maneuver	-	-	-	-	394	-
Stage 1	_	-	_	_	749	_
Stage 2	_	_	_	_	664	_
Olage 2		_	_	_	004	_
Approach	EB		WB		\$B	
HCM Control Delay, s	0		0		11	
HCM LOS					В	
TOM LOO						
A 42 A B A A A		EBL	EBT	WBT	WBR	SBLn1
Minor Lane/Major Mvm	I					606
	ıτ	1258	-	-	•	000
Capacity (veh/h)	nt	1258	-	•	-	
Capacity (veh/h) HCM Lane V/C Ratio	••	1258 0.001	-	-	-	0.008
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	••	1258 0.001 7.9	-	-	-	0.008
Capacity (veh/h) HCM Lane V/C Ratio		1258 0.001	-	-	-	0.008

Intersection						
Int Delay, s/veh	2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>A</b>	7		र्स	W	112011
Traffic Vol, veh/h	327	36	31	235	29	59
Future Vol, veh/h	327	36	31	235	29	59
Conflicting Peds, #/hr	0	0	0	233	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None		None	Stob.	None
	-		-			
Storage Length	щ A	150	-	-	0	-
Veh in Median Storage		-	•	0	0	•
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	7	6	4	3	13	5
Mvmt Flow	359	40	34	258	32	65
Major/Minor N	Anior1		Major?		Minor1	
	/ajor1		Major2			250
Conflicting Flow All	0	0	399	0	685	359
Stage 1	-	•	•	•	359	-
Stage 2	-	-	-	-	326	-
Critical Hdwy	-	-	4.14	-	6.53	6.25
Critical Hdwy Stg 1	•	•	-	-	5.53	-
Critical Hdwy Stg 2	-	-	-	-	5.53	-
Follow-up Hdwy	-	-	2.236	-	3.617	3.345
Pot Cap-1 Maneuver	-		1149	_	397	679
Stage 1	-	-	-	-	683	-
Stage 2		_		-	707	_
Platoon blocked, %				_	, 01	
Mov Cap-1 Maneuver	_		1149	-	383	679
	-	-	1149			019
Mov Cap-2 Maneuver				-	383	-
Stage 1	-	-	-	-	683	-
Stage 2	-	-	*		682	*0
Approach	EB		WB		NB	
HCM Control Delay, s	0		1		13.1	
HCM LOS	-				В	
Minor Lane/Major Mvm	+ 1	NBLn1	EBT	EBR	WBL	WBT
	, 1					
Capacity (veh/h)		541	-	-	1149	-
HCM Lane V/C Ratio		0.179	-	-	0.03	-
HCM Control Delay (s)		13.1	-	-	8.2	0
HCM Lane LOS		В	•	•	Α	Α
HCM 95th %tile Q(veh)		0.6	-	-	0.1	-

	*	-	7	-	-		1	-	1	ļ	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Group Flow (vph)	43	1164	113	237	1244	72	82	137	58	23	
v/c Ratio	0.32	0.43	0.12	0.61	0.41	0.49	0.27	0.51	0.40	0.08	
Control Delay	58.7	19.1	2.4	57.3	15.7	62.2	52.1	15.0	60.2	27.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	58.7	19.1	2.4	57.3	15.7	62.2	52.1	15.0	60.2	27.6	
Queue Length 50th (ft)	32	197	0	91	193	59	32	0	44	2	
Queue Length 95th (ft)	69	287	24	129	283	109	58	59	86	16	
Internal Link Dist (ft)		601			622		348			415	
Turn Bay Length (ft)	225		150	375		250		250	200		
Base Capacity (vph)	280	2728	912	543	3031	268	540	378	191	357	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.15	0.43	0.12	0.44	0.41	0.27	0.15	0.36	0.30	0.06	
Intersection Summary											

	<b></b>	۶	<b>→</b>	*	F	1	+	4	4	†	-	1
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		Ā	ተተተ	7		ሽኘ	ተተኈ		ሻ	41	7	7
Traffic Volume (vph)	28	12	1071	104	65	153	1118	27	133	8	126	53
Future Volume (vph)	28	12	1071	104	65	153	1118	27	133	8	126	53
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0	6.0
Lane Util. Factor		1.00	0.91	1.00		0.97	0.91		0.91	0.91	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00		1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00		0.95	0.96	1.00	0.95
Satd. Flow (prot)		1770	5085	1583		3433	5068		1610	3246	1583	1770
Flt Permitted		0.95	1.00	1.00		0.95	1.00		0.95	0.96	1.00	0.95
Satd. Flow (perm)		1770	5085	1583		3433	5068		1610	3246	1583	1770
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	30	13	1164	113	71	166	1215	29	145	9	137	58
RTOR Reduction (vph)	0	0	0	54	0	0	1	0	0	Õ	124	0
Lane Group Flow (vph)	0	43	1164	59	0	237	1243	0	72	82	13	58
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA		Split	NA	Perm	Split
Protected Phases	5	5	2		1	1	6		4	4		3
Permitted Phases				2							4	
Actuated Green, G (s)		7.5	63.1	63.1		13.6	69.2		11.1	11.1	11.1	8.2
Effective Green, g (s)		7.5	63.1	63.1		13.6	69.2		11.1	11.1	11.1	8.2
Actuated g/C Ratio		0.06	0.53	0.53		0.11	0.58		0.09	0.09	0.09	0.07
Clearance Time (s)		6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0	6.0
Vehicle Extension (s)		3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0
Lane Grp Cap (vph)		110	2673	832		389	2922		148	300	146	120
v/s Ratio Prot		0.02	0.23	002		c0.07	c0.25		c0.04	0.03		c0.03
v/s Ratio Perm		0.02	0.20	0.04		00.01	00.20		00.0	0.00	0.01	00.00
v/c Ratio		0.39	0.44	0.07		0.61	0.43		0.49	0.27	0.09	0.48
Uniform Delay, d1		54.1	17.5	14.0		50.7	14.2		51.7	50.7	49.8	53.9
Progression Factor		1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00
Incremental Delay, d2		2.3	0.5	0.2		2.7	0.1		2.5	0.5	0.3	3.0
Delay (s)		56.3	18.0	14.2		53.4	14.3		54.2	51.2	50.1	56.9
Level of Service		E	В	В		00.4 D	В		D	D	D	E
Approach Delay (s)		L	18.9			U	20.6		U	51.4		_
Approach LOS			10.3 B				20.0 C			D D		
Intersection Summary							Ü			J		
HCM 2000 Control Delay			23.6	Н	CM 2000	Level of	Service		С	•		
HCM 2000 Volume to Capac	ity ratio		0.47	• • •					-			
Actuated Cycle Length (s)	.,		120.0	Si	um of los	time (s)			24.0			
Intersection Capacity Utilizat	ion		61.8%			of Service	<u> </u>		В			
Analysis Period (min)			15		J = 31011				_			
c Critical Lane Group												

	<b>↓</b>	1
Movement	SBT	SBR
LaneConfigurations	<b>ተ</b> ጉ	
Traffic Volume (vph)	6	15
Future Volume (vph)	6	15
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	6.0	
Lane Util. Factor	0.95	
Frt	0.90	
Flt Protected	1.00	
Satd. Flow (prot)	3170	
Flt Permitted	1.00	
Satd. Flow (perm)	3170	
Peak-hour factor, PHF	0.92	0.92
Adj. Flow (vph)	7	16
RTOR Reduction (vph)	15	0
Lane Group Flow (vph)	8	0
Turn Type	NA	
Protected Phases	3	
Permitted Phases		
Actuated Green, G (s)	8.2	
Effective Green, g (s)	8.2	
Actuated g/C Ratio	0.07	
Clearance Time (s)	6.0	
Vehicle Extension (s)	3.0	
Lane Grp Cap (vph)	216	
v/s Ratio Prot	0.00	
v/s Ratio Perm		
v/c Ratio	0.04	
Uniform Delay, d1	52.2	

Progression Factor

Level of Service

Approach LOS

Approach Delay (s)

Intersection Summary

Delay (s)

Incremental Delay, d2

1.00

0.1

52.3

55.6

D

Ε

Intersection: 1: I-95 SB On-Ramp & Route 54

Movement	EB	WB	SB	SB	B28
Directions Served	Т	L	LT	R	Т
Maximum Queue (ft)	8	85	109	73	134
Average Queue (ft)	0	33	37	6	10
95th Queue (ft)	6	66	94	59	104
Link Distance (ft)	155	98	103	103	987
Upstream Blk Time (%)		0	4		
Queuing Penalty (veh)		1	0		
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

### Intersection: 2: I-95 NB On-Ramp & Route 54

Movement	EB	EB	EB	WB
Directions Served	L	Т	Т	Т
Maximum Queue (ft)	145	139	74	18
Average Queue (ft)	69	53	27	1
95th Queue (ft)	129	94	57	11
Link Distance (ft)		179	179	146
Upstream Blk Time (%)	0	0		
Queuing Penalty (veh)	0	0		
Storage Bay Dist (ft)	300			
Storage Blk Time (%)	0	0		
Queuing Penalty (veh)	0	0		

### Intersection: 3: Route 54 & Woodside Lane

Movement	EB	W8	SB	SB
Directions Served	L	TR	L	R
Maximum Queue (ft)	18	37	51	63
Average Queue (ft)	1	2	13	29
95th Queue (ft)	9	19	36	46
Link Distance (ft)		23	891	
Upstream Blk Time (%)		0		
Queuing Penalty (veh)		0		
Storage Bay Dist (ft)	225			200
Storage Blk Time (%)				
Queuing Penalty (veh)				

### Intersection: 4: Route 54 & Providence Church Road

Movement	EB	SB	
Directions Served	T	LR	
Maximum Queue (ft)	2	4	
Average Queue (ft)	0	0	
95th Queue (ft)	2	2	
Link Distance (ft)	23	1232	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

#### Intersection: 6: Goddins Hill Road & Route 54

Movement	NB
Directions Served	LR
Maximum Queue (ft)	70
Average Queue (ft)	34
95th Queue (ft)	56
Link Distance (ft)	529
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

## Intersection: 22: Route 54

EB	SW	B24
Т	R	Т
27	268	323
1	146	45
13	274	216
151	166	852
	17	
	0	
	T 27 1 13	T R 27 268 1 146 13 274 151 166 17

Intersection: 31: Route 54

Movement	EB	EB	WB	WB	WB		
Directions Served	Т	R	Т	Т	T		
Maximum Queue (ft)	50	307	36	42	40		
Average Queue (ft)	2	122	18	14	2		
95th Queue (ft)	37	255	39	42	19		
Link Distance (ft)	610	610	155	155	155		
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)							
Storage Blk Time (%)							
Queuing Penalty (veh)							

Intersection: 32: I-95 SB On-Ramp

Movement	SE
Directions Served	L
Maximum Queue (ft)	106
Average Queue (ft)	45
95th Queue (ft)	84
Link Distance (ft)	215
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 34: Hill Carter Parkway & England Street/Route 54

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	UL	Т	Т	Т	R	UL	L	Ŧ	Т	TR	L	LT
Maximum Queue (ft)	92	305	287	322	249	141	135	147	177	216	138	174
Average Queue (ft)	36	155	130	172	42	67	70	77	94	115	17	96
95th Queue (ft)	77	255	253	279	138	117	118	137	158	189	85	160
Link Distance (ft)		632	632	632				610	610	610		352
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225				150	375	375				250	
Storage Blk Time (%)		2		12								
Queuing Penalty (veh)		1		13								

Intersection: 34: Hill Carter Parkway & England Street/Route 54

Movement	NB	NB	SB	SB	SB
Directions Served	T	R	L	Т	TR
Maximum Queue (ft)	59	115	111	40	42
Average Queue (ft)	2	43	46	6	12
95th Queue (ft)	28	89	92	27	36
Link Distance (ft)	352			429	429
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		250	200		
Storage Blk Time (%)					
Queuing Penalty (veh)					

#### **Network Summary**

Network wide Queuing Penalty: 15

APPENDIX E: SYNCHRO & SIMTRAFFIC OUTPUT - BUILD (2027) CONDITIONS

ntersection														
it Delay, s/veh	3													
lovement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
ane Configurations		44		ሻ	44						4	7		
raffic Vol, veh/h	0	319	0	323	957	0	0	0	0	14	1	242		
uture Vol, veh/h	0	319	0	323	957	0	0	0	0	14	1	242		
onflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0		
gn Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop		
T Channelized	-	-	None	-	_	None	-		None		-	Free		
orage Length		-	-	0			-	-	-	-	-	0		
eh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-		
ade, %		0	-	-	0		-	0		-	0			
eak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93		
eavy Vehicles, %	2	13	2	2	5	2	2	2	2	2	2	10		
vmt Flow	0	343	0	347	1029	0	0	0	0	15	1	260		
=														
<del></del>	vlajor1		- 1	Major2						/linor2				
onflicting Flow All	-	0	-	343	0	0				1895	2066	-		
Stage 1	-	-	-	-	-	-				1723	1723	-		
Stage 2	•	-	-		-	•				172	343	•		
ritical Hdwy	-	-	-	4.14	-	-				6.84	6.54	-		
ritical Hdwy Stg 1	-	-	-	-	-	-				5.84	5.54	-		
ritical Hdwy Stg 2	•	•	-		-	•				5.84	5.54	•		
ollow-up Hdwy	-	-	-	2.22	-	-				3.52	4.02	-		
ot Cap-1 Maneuver	0	-	0	1213	-	0				61	54	0		
Stage 1	0	•	0	•	•	0				129	142	0		
Stage 2	0	-	0	-	-	0				841	636	0		
latoon blocked, %		-		4040	-					4.4	0			
lov Cap-1 Maneuver	-	-	-	1213	-	-				44	0	-		
ov Cap-2 Maneuver	-	-	-	-	-	-				44	0	-		
Stage 1	-	-	-	-	-	-				129	0	-		
Stage 2	-	-	-	-	-	-				600	0	-		
pproach	ЕВ			WB						SB			 	
CM Control Delay, s	0			2.3						128.2				
CM LOS										F				
linor Lane/Major Mvm	+	EBT	WBL	\A/DT (	SBLn1	SBI 22								
	ı	LDI	1213	VVDI	44	JULIIZ								
apacity (veh/h) CM Lane V/C Ratio		-		-		-								
		-	0.286		0.367	-								
CM Control Delay (s)		-	9.2	-		0								
CM Lane LOS		-	Α	-	F	Α								
CM 95th %tile Q(veh)			1.2		1.3									

												_
Intersection												
Int Delay, s/veh	0			•								
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	NEL	NER		
Lane Configurations	7	44			44	7				7		
Traffic Vol, veh/h	160	173	0	0	605	45	0	0	0	99		
Future Vol, veh/h	160	173	0	0	605	45	0	0	0	99		
Conflicting Peds, #/hr	0	0	0	0	0	0	Ō	Õ	Ō	0		
Sign Control	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free		
RT Channelized	-	- 10 -	None	-	-	Yield	_	_	_	Free		
Storage Length	300		-	-		0	_		_	0		
Veh in Median Storage,		0	-	-	0	-	0	_	0	_		
Grade, %	_	0	-	-	0	_	0		0	-		
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93		
Heavy Vehicles, %	20	5	2	2	2	16	2	2	2	6		
Mvmt Flow	172	186	0	0	651	48	0	0	0	106		
Major/Minor M	inor2		1	Major2								
Conflicting Flow All	651	651		-	-	0						
Stage 1	651	651	-	-	-	-						
Stage 2	0	0	-	-	-	-						
Critical Hdwy	7.2	6.6	-	-	-	_						
Critical Hdwy Stg 1	6.2	5.6	-	-	-	-						
Critical Hdwy Stg 2	-		-	-	_	-						
Follow-up Hdwy	3.7	4.05	_	-	-	-						
Pot Cap-1 Maneuver	363	380	0	0	-	-						
Stage 1	435	455	0	0	•	-						
Stage 2	-	-	0	0	-	-						
Platoon blocked, %					-	-						
Mov Cap-1 Maneuver	363	0	-	-	-	-						
Mov Cap-2 Maneuver	363	0	-	-	-	-						
Stage 1	435	0	-	-	-	-						
Stage 2	-	0	-	-	-	-						
Approach	EΒ			WB								
HCM Control Delay, s				0								
HCM LOS	-											
Minor Lane/Major Mvmt	8		EBLn2 l	EBLn3	WBT	WBR					 	
Capacity (veh/h)		363	-	-	-	-						
HCM Lane V/C Ratio		0.474	-	-	-	-						
HCM Control Delay (s)		23.5	-	-	•	•						
HCM Lane LOS		С	-	-	-	_						
HCM 95th %tile Q(veh)		2.5										

					-		
Intersection							
Int Delay, s/veh	5.7						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	ሻ	<b>†</b>	1		ሻ	7	
Traffic Vol, veh/h	44	175	341	23	60	248	
Future Vol, veh/h	44	175	341	23	60	248	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free		Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	225	•	-	-	0	200	
Veh in Median Storage	e,# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	87		87	87	87	87	
Heavy Vehicles, %	3		2	28	6	2	
Mvmt Flow	51	201	392	26	69	285	
	Major1		Major2		Minor2		
Conflicting Flow All	418	0	•	0	708	405	
Stage 1	-	-	-	-	405	-	
Stage 2	-	-	-	-	303	-	
Critical Hdwy	4.13	-	-	-	6.46	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.46	-	
Critical Hdwy Stg 2		-	•	-	5.46		
Follow-up Hdwy	2.227	-	-	-		3.318	
Pot Cap-1 Maneuver	1136	-	-	-	395	646	
Stage 1	•	-	•	-	665	-	
Stage 2	-	-	-	-	740	-	
Platoon blocked, %	1126	-	-	-	277	646	
Mov Cap-1 Maneuver Mov Cap-2 Maneuver	1136	-	-	-	377 377	040	
Stage 1	-	-	-	-	635	-	
Stage 2	•	_	-	-	740	-	
Glaye Z	-	-	-	-	140	-	
Annraach	ED		\AID		ĈD.		
Approach	EB	· · · <u>-</u> · · ·	WB		SB 45.2		
HCM Control Delay, s	1.7		0		15.3		
HCM LOS					С		
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR:	SBLn1 S	
Capacity (veh/h)		1136	-	-	-	377	646
HCM Lane V/C Ratio		0.045	-	-	-	0.183	
HCM Control Delay (s)	)	8.3	-	-	-	16.7	14.9
HCM Lane LOS		Α	-	-	-	С	В
HCM 95th %tile Q(veh	)	0.1	-	-	-	0.7	2.3

Intersection							
Int Delay, s/veh	0.4						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	75	<b>1</b>	1		W		· · · · · · · · · · · · · · · · · · ·
Traffic Vol, veh/h	7	228	350	1	1	14	
Future Vol, veh/h	7	228	350	1	1	14	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized		None	-	None	Otop	None	
Storage Length	200	140110	_	-	0	140110	
Veh in Median Storage		0	0	_	0	_	
Grade, %	η π - •	0	0	-	0		
Peak Hour Factor		87	87		87		
	87			87		87	
Heavy Vehicles, %	2	7	2	2	2	2	
Mvmt Flow	8	262	402	1	1	16	
Major/Minor	Major1	N	Major2	1	Minor2		
Conflicting Flow All	403	0	-	0	681	403	
Stage 1	-	_	_		403	-	
Stage 2			-	-	278	_	
Critical Hdwy	4.12	_	_	_	6.42	6.22	
Critical Hdwy Stg 1	-	_	_		5.42	-	
Critical Hdwy Stg 2		_			5.42	_	
Follow-up Hdwy	2.218	_	_	_		3.318	
Pot Cap-1 Maneuver	1156		_	-	416	647	
Stage 1	1100	_			675	047	
Stage 2	_	_	_	-	769	-	
	-	_	-		109	-	
Platoon blocked, %	1150	•	•	-	413	647	
Mov Cap-1 Maneuver	1156	-	-	-			
Mov Cap-2 Maneuver	-	-	-	-	413	-	
Stage 1	-	-	-	-	670	-	
Stage 2	-	-	-	-	769	-	
Approach	EB		WB		SB		
HCM Control Delay, s	0.2		0		10.9		
HCM LOS					В		
Minor Lane/Major Mvm	ıt	EBL	EBT	WBT	\A/DD	SBLn1	
	ıı		LOI	AADI	AADIA		
Capacity (veh/h)		1156	-	-	-	623	
HCM Lane V/C Ratio		0.007	•	-	-	0.028	
HCM Control Delay (s)		8.1	-	-	-	10.9	
HCM Lane LOS		Α	-	-	-	В	
HCM 95th %tile Q(veh)	)	0	•	-	-	0.1	

Intersection						
Int Delay, s/veh	0.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	T T	<b>A</b>	\$	AADIA	Tyf	ODIN
Traffic Vol, veh/h	า 9	<b>T</b> 220	331	3	<b>T</b> 5	20
Future Vol, veh/h	9	220	331	3	5	20
Conflicting Peds, #/hr	0	0	331	ა 0	0	0
Sign Control	Free		Free	Free	Stop	
RT Channelized	rree -	None		None	Stop	Stop None
			-			
Storage Length	200	-	_	-	0	-
Veh in Median Storage		0	0	-	0	•
Grade, %	-	0	0	- 00	0	- 02
Peak Hour Factor	92		92	92		92
Heavy Vehicles, %	2		2	2	2	2
Mvmt Flow	10	239	360	3	5	22
Major/Minor N	/lajor1	N	Major2		Minor2	
Conflicting Flow All	363		-	0	621	362
Stage 1	303	-		-	362	502
Stage 2	-	_		_	259	-
Critical Hdwy	4.12	_	•		6.42	6.22
Critical Hdwy Stg 1	4.12	•	•	•	5.42	0.22
	-	-	-	-	5.42	•
Critical Hdwy Stg 2	2.218	-	-		3.518	2 240
		-	•	-		
Pot Cap-1 Maneuver	1196	-	-	-	451	683
Stage 1	-	-	-	-	704	-
Stage 2	-	-	-	-	784	-
Platoon blocked, %	4400	-	-	-	,	000
Mov Cap-1 Maneuver	1196	-	-	-	447	683
Mov Cap-2 Maneuver	-	-	-	-	447	-
Stage 1	-	-	-	-	698	-
Stage 2	-	-	-	-	784	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.3	_	0		11.1	
HCM LOS			_		В	
Minor Long/Major Mare	•	EDI	EDT	WOT	MIDD	CDI ~4
Minor Lane/Major Mvm	ί	EBL	EBT	MRI	WBR:	
Capacity (veh/h)		1196	-	-	-	618
HCM Lane V/C Ratio		0.008	-	-	-	0.044
HCM Control Delay (s)		8	•	•	-	11.1
HCM Lane LOS		Α	-	-	-	В
HCM 95th %tile Q(veh)		0	-	-	-	0.1

Intersection						
Int Delay, s/veh	2.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	T.	,,,,,,	4	W	.,
Traffic Vol, veh/h	160	54	81	265	27	25
Future Vol, veh/h	160	54	81	265	27	25
Conflicting Peds, #/hr	0	0	0	0	0	0
<del>-</del>	Free	Free	Free	Free	Stop	Stop
RT Channelized		None		None	-	None
Storage Length	-	150	_	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	_
Grade, %	0		_	0	0	
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	7	6	4	3	13	5
Mymt Flow	176	59	89	291	30	27
WHILL ION	170	00	03	201	50	21
	ajor1		Vlajor2		Minor1	
Conflicting Flow All	0	0	235	0	645	176
Stage 1	-	-	-	-	176	-
Stage 2	-	-	-	-	469	-
Critical Hdwy	-	-	4.14	-	6.53	6.25
Critical Hdwy Stg 1	•	•	-	-	5.53	-
Critical Hdwy Stg 2	-	-	-	-	5.53	-
Follow-up Hdwy	-	-	2.236	-	3.617	3.345
Pot Cap-1 Maneuver	•	•	1321	-	420	859
Stage 1	-	-	-	-	829	-
Stage 2	-	-	-	-	607	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1321	-	386	859
Mov Cap-2 Maneuver	-	-	-	-	386	-
Stage 1	-	-	-	-	829	-
Stage 2	-	_	-	-	558	-
-						
Approach	ЕВ		WB		NB	
HCM Control Delay, s	0		1.9		12.7	
HCM LOS	U		1.3		12.7 B	
TION LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		525	-	-	1321	-
HCM Lane V/C Ratio		0.109	-	-	0.067	-
HCM Control Delay (s)		12.7	-	-	7.9	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.4	•	-	0.2	-

Build (2027) Conditions Timing Plan: AM Peak Hour

34: Hill Carter Parkway & England Street/Route 54

	*	-	*	1	•	4	<b>†</b>	-	1	<b>↓</b>	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Group Flow (vph)	22	1026	33	168	1145	29	34	42	108	11	
v/c Ratio	0.15	0.40	0.04	0.47	0.37	0.22	0.12	0.15	0.61	0.02	
Control Delay	45.3	17.6	0.1	46.5	12.5	46.6	43.2	1.1	57.6	0.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	45.3	17.6	0.1	46.5	12.5	46.6	43.2	1.1	57.6	0.0	
Queue Length 50th (ft)	13	154	0	53	116	19	11	0	66	0	
Queue Length 95th (ft)	38	208	0	83	217	49	27	0	123	0	
Internal Link Dist (ft)		601			622		348			415	
Turn Bay Length (ft)	225		150	375		250		250	200		
Base Capacity (vph)	230	2541	873	446	3077	289	585	419	194	718	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.10	0.40	0.04	0.38	0.37	0.10	0.06	0.10	0.56	0.02	
Intersection Summary										<u>.</u>	

	*	<b>-</b>	>	•	<b>←</b>	4	4	<b>†</b>	-	-	<b></b>	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ä	<b>†</b> ††	7	<b>ሕ</b> ካ	4 <sup>†</sup>		ሻ	44	1	N.	作	
Traffic Volume (vph)	20	944	30	155	1047	6	53	5	39	99	2	8
Future Volume (vph)	20	944	30	155	1047	6	53	5	39	99	2	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0	6.0	6.0	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91		0.91	0.91	1.00	1.00	0.95	
Frt	1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85	1.00	0.88	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00	0.95	1.00	
Satd. Flow (prot)	1770	5085	1583	3433	5081		1610	3252	1583	1770	3105	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00	0.95	1.00	
Satd. Flow (perm)	1770	5085	1583	3433	5081		1610	3252	1583	1770	3105	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	22	1026	33	168	1138	7	58	5	42	108	2	9
RTOR Reduction (vph)	0	0	17	0	0	0	0	0	39	0	10	0
Lane Group Flow (vph)	22	1026	16	168	1145	0	29	34	3	108	1	0
Turn Type	Prot	NA	Perm	Prot	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		3	3	
Permitted Phases			2						4			
Actuated Green, G (s)	3.3	48.8	48.8	10.3	55.8		6.8	6.8	6.8	10.1	10.1	
Effective Green, g (s)	3.3	48.8	48.8	10.3	55.8		6.8	6.8	6.8	10.1	10.1	
Actuated g/C Ratio	0.03	0.49	0.49	0.10	0.56		0.07	0.07	0.07	0.10	0.10	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	58	2481	772	353	2835		109	221	107	178	313	
v/s Ratio Prot	0.01	0.20		c0.05	c0.23		c0.02	0.01		c0.06	0.00	
v/s Ratio Perm			0.01						0.00			
v/c Ratio	0.38	0.41	0.02	0.48	0.40		0.27	0.15	0.03	0.61	0.00	
Uniform Delay, d1	47.3	16.4	13.2	42.3	12.6		44.2	43.9	43.5	43.0	40.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	4.1	0.5	0.0	1.0	0.1		1.3	0.3	0.1	5.7	0.0	
Delay (s)	51.5	16.9	13.3	43.3	12.7		45.5	44.2	43.6	48.8	40.4	
Level of Service	D	В	В	D	В		D	D	D	D	D	
Approach Delay (s)		17.5			16.6			44.3			48.0	
Approach LOS		В			В			D			D	
Intersection Summary												
HCM 2000 Control Delay		-	19.5	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capac	city ratio		0.44									
Actuated Cycle Length (s)	*		100.0	S	um of lost	time (s)			24.0			
Intersection Capacity Utilizat	ion		58.2%			of Service			В			
Analysis Period (min)			15									
c Critical Lane Group												

Intersection: 1: I-95 SB On-Ramp & Route 54

Movement	EB	WB	B33	B35	B35	B35	SB
Directions Served	Т	Ł	Т	Т	Т	Т	LT
Maximum Queue (ft)	3	156	41	13	12	9	53
Average Queue (ft)	0	54	2	0	1	0	14
95th Queue (ft)	3	118	29	9	11	9	40
Link Distance (ft)	155	98	206	151	151	151	103
Upstream Blk Time (%)		4	0				
Queuing Penalty (veh)		17	0				
Storage Bay Dist (ft)							
Storage Blk Time (%)							
Queuing Penalty (veh)							

## Intersection: 2: I-95 NB On-Ramp & Route 54

Movement	EB	EB	EB	WB	WB
Directions Served	L	T	Т	T	T
Maximum Queue (ft)	146	98	51	6	32
Average Queue (ft)	60	35	8	0	3
95th Queue (ft)	118	70	34	3	18
Link Distance (ft)		179	179	146	146
Upstream Blk Time (%)	0	0			
Queuing Penalty (veh)	0	0			
Storage Bay Dist (ft)	300				
Storage Blk Time (%)	0	0			
Queuing Penalty (veh)	0	0			

## Intersection: 3: Route 54 & Woodside Lane

Movement	EB	SB	\$B
Directions Served	L	L	R
Maximum Queue (ft)	37	69	142
Average Queue (ft)	12	22	55
95th Queue (ft)	34	48	101
Link Distance (ft)		891	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	225		200
Storage Blk Time (%)			0
Queuing Penalty (veh)			0

## Intersection: 4: Route 54 & Providence Church Road

Movement	EB	SB
Directions Served	L	LR
Maximum Queue (ft)	24	29
Average Queue (ft)	2	11
95th Queue (ft)	13	32
Link Distance (ft)		1008
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	200	
Storage Blk Time (%)		
Queuing Penalty (veh)		

## Intersection: 5: Route 54 & Site Driveway

Movement	EB	SB
Directions Served	L	LR
Maximum Queue (ft)	23	33
Average Queue (ft)	2	17
95th Queue (ft)	14	41
Link Distance (ft)		539
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	200	
Storage Blk Time (%)		
Queuing Penalty (veh)		

## Intersection: 6: Goddins Hill Road & Route 54

Movement	EB	WB	NB
Directions Served	R	LT	LR
Maximum Queue (ft)	3	84	74
Average Queue (ft)	0	17	28
95th Queue (ft)	3	56	59
Link Distance (ft)		1778	529
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	150		
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 22: Route 54

Movement	EB	SW	B24
Directions Served	Т	R	T
Maximum Queue (ft)	5	190	26
Average Queue (ft)	0	62	1
95th Queue (ft)	4	147	18
Link Distance (ft)	151	166	852
Upstream Blk Time (%)		1	
Queuing Penalty (veh)		0	
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 31: Route 54

Movement	EB	EB	WB	WB	WB
Directions Served	Т	R	Т	Т	Т
Maximum Queue (ft)	41	400	33	37	15
Average Queue (ft)	1	155	13	7	1
95th Queue (ft)	38	312	33	30	9
Link Distance (ft)	610	610	155	155	155
Upstream Blk Time (%)		0			
Queuing Penalty (veh)		0			
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

# Intersection: 32: I-95 SB On-Ramp

Movement	SE
Directions Served	L
Maximum Queue (ft)	229
Average Queue (ft)	138
95th Queue (ft)	240
Link Distance (ft)	215
Upstream Blk Time (%)	5
Queuing Penalty (veh)	16
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Movement	EB	EB	EB	ЕΒ	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	UL	T	T	T	R	UL	I I	T	T	TR	I	LT
Maximum Queue (ft)	64	185	294	357	212	89	114	163	170	185	5	93
Average Queue (ft)	16	93	109	189	24	36	58	75	80	84	0	41
95th Queue (ft)	45	163	258	309	118	75	96	136	144	152	3	82
Link Distance (ft)		632	632	632				610	610	610		352
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225				150	375	375				250	
Storage Blk Time (%)		0		14								
Queuing Penalty (veh)		0		4								

# Intersection: 34: Hill Carter Parkway & England Street/Route 54

Movement	3	NB	NB	SB	SB	SB
Directions Served		Т	R	L	Т	TR
Maximum Queue (ft)		2	58	152	25	36
Average Queue (ft)		0	17	68	2	8
95th Queue (ft)		2	41	127	13	30
Link Distance (ft)		352			429	429
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)			250	200		
Storage Blk Time (%)						
Queuing Penalty (veh)						

## **Network Summary**

Network wide Queuing Penalty: 37

ntersection nt Delay, s/veh	5.1												
•													
· · · · · · · · · · · · · · · · · · ·	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	•	<b>*</b>		<b>\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\</b>	<b>*</b>	•				4.4	र्स	7	
Traffic Vol, veh/h	0	637	0	147	1106	0	0	0	0	41	1	262	
Future Vol, veh/h	0	637	0	147	1106	0	0	0	0	41	1	262	
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	_ 0	_ 0	_ 0	_ 0	_ 0	0	0	0	
•	Free	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	
RT Channelized	•	-	None	~	•	None	-	-	None	•	•	Free	
Storage Length	-	-	-	0	-	-	-	-	-	-	-	0	
Veh in Median Storage, #		0	-	-	0	-	-	0	-	-	0	-	
Grade, %	02	0	-	00	0	00	- 02	0	- 00	- 02	0	02	
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93	
Heavy Vehicles, %	2	13	2	2	5	2	2	2	2	2	2	10	
Mvmt Flow	0	685	0	158	1189	0	0	0	0	44	1	282	
Major/Minor Ma	ajor1		N	Major2					ı	dinor2			
Conflicting Flow All		0		685	0	0				1848	2190	_	
Stage 1	_			•		-				1505	1505	_	
Stage 2	_		_	_	_	_				343	685	_	
Critical Hdwy	_	_	_	4.14	_	125				6.84	6.54	_	
Critical Hdwy Stg 1										5.84	5.54	-	
Critical Hdwy Stg 2	_	_	_	_	_	_				5.84	5.54		
Follow-up Hdwy	-	_	_	2.22	-	_				3.52	4.02		
Pot Cap-1 Maneuver	0		0	904		0				66	45	0	
Stage 1	Ō	_	0	_	_	0				170	182	Ō	
Stage 2	0	_	0	_		0				690	447	0	
Platoon blocked, %	•		•		_	_							
Mov Cap-1 Maneuver	-	_	_	904		_				54	0	-	
Mov Cap-2 Maneuver	-	_	-	-	-	-				54	Ō		
Stage 1	-	-	-	_	-	-				170	0		
Stage 2	-	-	-	-	-	-				569	0	-	
A	<b>-</b> -			14.05						65			
Approach	EB			WB						SB			
HCM Control Delay, s	0			1.2						197.5			
HCM LOS										F			
Minor Lane/Major Mvmt		EBT	WBL	WBT 9	SBLn1	SBLn2							
Capacity (veh/h)			904	_	54								
HCM Lane V/C Ratio		_	0.175	_	0.836								
HCM Control Delay (s)		-	9.8		197.5	0							
			2.4			-							
HCM Lane LOS		-	Α	-	F	Α							

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	NEL	NER		
Lane Configurations	7	<b>^</b>			44	7				7		
Traffic Vol, veh/h	270	408	0	0	373	35	0	0	0	228		
Future Vol, veh/h	270	408	0	0	373	35	0	0	0	228		
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0		
	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free		
RT Channelized		•	None	-	-	Yield	-	•	-	Free		
Storage Length	300	-	-	-	-	0	-	-	-	0		
Veh in Median Storage,	# -	0	-	-	0	-	0	-	0	-		
Grade, %	-	0	-	-	0	-	0	-	0	•		
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93		
Heavy Vehicles, %	20	5	2	2	2	16	2	2	2	6		
Mvmt Flow	290	439	0	0	401	38	0	0	0	245		
Major/Minor M	linor2		N	/lajor2								
Conflicting Flow All	401	401	-	-	-	0						
Stage 1	401	401	-	-	-							
Stage 2	0	0	-			-						
Critical Hdwy	7.2	6.6	-	-	-	-						
Critical Hdwy Stg 1	6.2	5.6	-	-	-	•						
Critical Hdwy Stg 2	-	-	-	-	-	-						
Follow-up Hdwy	3.7	4.05	-	-	-	-						
Pot Cap-1 Maneuver	532	529	0	0	-	•						
Stage 1	595	592	0	0	-	-						
Stage 2	-	-	0	0	-	-						
Platoon blocked, %					-	-						
Mov Cap-1 Maneuver	532	0	-	-	-	-						
Mov Cap-2 Maneuver	532	0	-	•	-							
Stage 1	595	0	-	-	-	-						
Stage 2	-	0	-	-	-	-						
Approach	EB			WB								
HCM Control Delay, s				0								
HCM LOS	-											
Minor Lane/Major Mvmt	Е	EBLn1 8	EBLn2 E	EBLn3	WBT	WBR						
		532	-	-	_	-						
Capacity (veh/h)				_								
Capacity (veh/h) HCM Lane V/C Ratio		0.546	•	-	_							
		19.6	-	-	-	-						
HCM Lane V/C Ratio			-	-	-	-						

Intersection							
Int Delay, s/veh	3.7						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	7	<u> </u>	1		ሻ	7	
Traffic Vol, veh/h	175	366	273	12	28	111	
Future Vol, veh/h	175	366	273	12	28	111	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	•	
RT Channelized	•	None	•	None	-	None	
Storage Length	225	-	-	-	0	200	
Veh in Median Storage		0	0	-	0	-	
Grade, %		0	0	•	0		
Peak Hour Factor	87	87	87	87	87	87	
Heavy Vehicles, %	3	7	2	28	6	2	
Mvmt Flow	201	421	314	14	32	128	
	Major1		Major2		Minor2		
Conflicting Flow All	328	0	-	0	1144	321	
Stage 1	-	-	-	-	321	-	
Stage 2	•	•	-	-	823		
Critical Hdwy	4.13	-	-	-	6.46	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.46	-	
Critical Hdwy Stg 2		-	-	-	5.46	-	
Follow-up Hdwy	2.227	-	-	-	3.554		
Pot Cap-1 Maneuver	1226	•	•	-	217	720	
Stage 1	-	-	-	-	726	-	
Stage 2	-	-	-	-	425	-	
Platoon blocked, %	4000	•	•	-	404	700	
Mov Cap-1 Maneuver		-	-	-	181	720	
Mov Cap-2 Maneuver	-	-	-	-	181 607	**	
Stage 1 Stage 2	-	-	-	-	425	-	
Staye 2	•	-	-	-	420	-	
Approach	EB		WB		SB		
HCM Control Delay, s	2.8		0		14.7		
HCM LOS					В		
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1 SE	3Ln2
Capacity (veh/h)		1226	-	-	-	181	720
HCM Lane V/C Ratio		0.164	-	-	-	0.178 0	.177
HCM Control Delay (s	)	8.5	-	-	-		11.1
HCM Lane LOS		Α	-	-	-	D	В
HCM 95th %tile Q(veh	1)	0.6	-	-	-	0.6	0.6
•							

# Hickory Grove 4: Route 54 & Providence Church Road

Intersection							
Int Delay, s/veh	0.3						· <del>-</del>
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	7	<b>†</b>	1>	•	N.		
Traffic Vol, veh/h	12	383	275	2	1	10	
Future Vol, veh/h	12	383	275	2	1	10	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None		None		None	
Storage Length	200	-	-	-	0	-	
/eh in Median Storage		0	0	_	0		
Grade, %		0	0	-	0	-	
Peak Hour Factor	87	87	87	87	87	87	
Heavy Vehicles, %	2	7	2	2	2	2	
Mvmt Flow	14	440	316	2	1	11	
Major/Minor N	Major1	N	Major2	1	Minor2		
Conflicting Flow All	318	0	-	0	785	317	
Stage 1	-	-	-	_	317	-	
Stage 2		-			468	-	
Critical Hdwy	4.12	-	-	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2		-	-	-	5.42	-	
Follow-up Hdwy	2.218	-	-	-	3.518	3.318	
Pot Cap-1 Maneuver	1242	-	-	-	361	724	
Stage 1	-	-	-	-	738	-	
Stage 2	-	-	-	-	630	-	
Platoon blocked, %		-		-			
Mov Cap-1 Maneuver	1242	-	-	-	357	724	
Mov Cap-2 Maneuver	-	-	-	-	357	_	
Stage 1	-	-	-	-	730	-	
Stage 2	-	-	-	-	630	-	
Approach	EB		WB		SB		
HCM Control Delay, s	0.2		0		10.5		
HCM LOS					В		
Minor Lana/Major Ma	4	EDI	CRT	VAIDT	MPD	CDI ∽4	
Minor Lane/Major Mvm	ι	EBL	EBT	VV D I	WBR		
Capacity (veh/h)		1242	-	-	-	662	
HCM Control Dolor (a)		0.011	-	-	-	0.019	
HCM Control Delay (s)		7.9	-	-	-	10.5	
HCM Lane LOS HCM 95th %tile Q(veh)		A 0	-	•	•	B 0.1	

Intersection						
Int Delay, s/veh	0.5					-
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	7	<u> </u>	₩ <u>₩</u>	VVDIV	JU!	אוםט.
Traffic Vol, veh/h		<b>T</b> 364	264	c		13
	20 20		264	6	4	
Future Vol, veh/h		364		6	4	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	200	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	22	396	287	7	4	14
				•	•	
4.4 1 44.41						
	Major1		vlajor2		Minor2	4 -
Conflicting Flow All	294	0	-	0	731	291
Stage 1	•	•	-	-	291	-
Stage 2	-	-	-	-	440	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-			5.42	
Critical Hdwy Stg 2	-	_	_	-	5.42	-
Follow-up Hdwy	2.218	_	_	_	3.518	3 318
Pot Cap-1 Maneuver	1268	-		_	389	748
•	1200	-	•	-	759	170
Stage 1	-	-	-	-		-
Stage 2	-	-	•	-	649	-
Platoon blocked, %	45.55	-	-	-		
Mov Cap-1 Maneuver	1268	-	-	-	382	748
Mov Cap-2 Maneuver	-	-	-	-	382	-
Stage 1	-	-	-	-	746	-
Stage 2		-	-	-	649	-
3						
Approach	EB		W8		SB	
HCM Control Delay, s	0.4		0		11.1	
HCM LOS					В	
Minor Lane/Major Myn	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		1268		•	-	610
HCM Lane V/C Ratio		0.017	_	-	_	0.03
HCM Control Delay (s	)	7.9	-	_	_	11.1
HCM Lane LOS	,	Α	-	_	_	В
HCM 95th %tile Q(veh	١,	0.1				0.1
TION SOUL WILL OF CALL	7	0.1	-	-	-	U, I

Intersection						
Int Delay, s/veh	1.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>1</b>	7		4	W	
Traffic Vol, veh/h	331	36	31	241	29	59
Future Vol, veh/h	331	36	31	241	29	59
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None	-	None	- -	None
Storage Length	_	150	_	-	0	-
Veh in Median Storage,	# 0			0	Ŏ	
Grade, %	0	_	_	0	0	_
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	7	6	4	3	13	5
Mymt Flow	364	40	34	265	32	65
MATHER TOW	004	70	07	200	J2	00
	lajor1	ľ	Vlajor2		Minor1	
Conflicting Flow All	0	0	404	0	697	364
Stage 1	•	-	•	•	364	-
Stage 2	-	-	-	-	333	-
Critical Hdwy	-	-	4.14	-	6.53	6.25
Critical Hdwy Stg 1	-	-	-	-	5.53	-
Critical Hdwy Stg 2	-	-	-	-	5.53	-
Follow-up Hdwy	-	-	2.236	-	3.617	3.345
Pot Cap-1 Maneuver	-	-	1144	-	391	674
Stage 1	-	-	-	-	679	-
Stage 2	•	-	-	•	702	
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1144	-	377	674
Mov Cap-2 Maneuver	-	-	-	-	377	-
Stage 1	-	-	-	-	679	-
Stage 2	-	*	-		677	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.9		13.2	
HCM LOS	-				В	
					_	
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL.	WBT
Capacity (veh/h)		535			1144	
HCM Lane V/C Ratio		0.181	-	_	0.03	-
HCM Control Delay (s)		13.2		_	8.2	0
HCM Lane LOS		В	-		Α	Ā
HCM 95th %tile Q(veh)		0.7	-	_	0.1	-

	*	-	*	•	<b>←</b>	4	1	-	-	Į.	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Group Flow (vph)	43	1171	113	237	1247	72	82	137	58	23	
v/c Ratio	0.32	0.43	0.12	0.61	0.41	0.49	0.27	0.51	0.40	0.08	
Control Delay	58.7	19.2	2.4	57.3	15.7	62.2	52.1	15.0	60.2	27.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	58.7	19.2	2.4	57.3	15.7	62.2	52.1	15.0	60.2	27.6	
Queue Length 50th (ft)	32	199	0	91	194	59	32	0	44	2	
Queue Length 95th (ft)	69	289	24	129	283	109	58	59	86	16	
Internal Link Dist (ft)		601			622		348			415	
Turn Bay Length (ft)	225		150	375		250		250	200		
Base Capacity (vph)	280	2728	912	543	3031	268	540	378	191	357	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.15	0.43	0.12	0.44	0.41	0.27	0.15	0.36	0.30	0.06	
Intersection Summary											

34: Hill Carter Parkway & England Street/Route 54

	<b></b>	۶	<b>→</b>	*	F	•	+	4	4	†	-	1
Movement	EBU	EBL	_EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		Ā	ተተተ	7		ሽኘ	ተተኈ		ሻ	44	7	ሻ
Traffic Volume (vph)	28	12	1077	104	65	153	1121	27	133	8	126	53
Future Volume (vph)	28	12	1077	104	65	153	1121	27	133	8	126	53
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0	6.0
Lane Util. Factor		1.00	0.91	1.00		0.97	0.91		0.91	0.91	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00		1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00		0.95	0.96	1.00	0.95
Satd. Flow (prot)		1770	5085	1583		3433	5068		1610	3246	1583	1770
Flt Permitted		0.95	1.00	1.00		0.95	1.00		0.95	0.96	1.00	0.95
Satd. Flow (perm)		1770	5085	1583		3433	5068		1610	3246	1583	1770
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	30	13	1171	113	71	166	1218	29	145	9	137	58
RTOR Reduction (vph)	0	0	0	54	0	0	1	0	0	0	124	0
Lane Group Flow (vph)	0	43	1171	59	0	237	1246	0	72	82	13	58
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA		Split	NA	Perm	Split
Protected Phases	5	5	2		1	1	6		4	4		3
Permitted Phases				2							4	
Actuated Green, G (s)		7.5	63.1	63.1		13.6	69.2		11.1	11.1	11.1	8.2
Effective Green, g (s)		7.5	63.1	63.1		13.6	69.2		11.1	11.1	11.1	8.2
Actuated g/C Ratio		0.06	0.53	0.53		0.11	0.58		0.09	0.09	0.09	0.07
Clearance Time (s)		6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0	6.0
Vehicle Extension (s)		3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0
Lane Grp Cap (vph)		110	2673	832		389	2922		148	300	146	120
v/s Ratio Prot		0.02	0.23			c0.07	c0.25		c0.04	0.03		c0.03
v/s Ratio Perm				0.04							0.01	
v/c Ratio		0.39	0.44	0.07		0.61	0.43		0.49	0.27	0.09	0.48
Uniform Delay, d1		54.1	17.5	14.0		50.7	14.3		51.7	50.7	49.8	53.9
Progression Factor		1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00
Incremental Delay, d2		2.3	0.5	0.2		2.7	0.1		2.5	0.5	0.3	3.0
Delay (s)		56.3	18.1	14.2		53.4	14.4		54.2	51.2	50.1	56.9
Level of Service		Е	В	В		D	В		D	D	D	Ε
Approach Delay (s)			19.0				20.6			51.4		
Approach LOS			В				С			D		
Intersection Summary												
HCM 2000 Control Delay			23.6	H	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capac	city ratio		0.47									
Actuated Cycle Length (s)			120.0		um of lost				24.0			
Intersection Capacity Utiliza	tion		61.9%	IC	:U Level	of Service	<del>)</del>		В			
Analysis Period (min)			15									
c Critical Lane Group												

# 34: Hill Carter Parkway & England Street/Route 54

	<b>‡</b>	1
Movement	SBT	SBR
Laneronfigurations	<b>ተ</b> ኈ	
Traffic Volume (vph)	6	15
Future Volume (vph)	6	15
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	6.0	
Lane Util. Factor	0.95	
Frt	0.90	
Flt Protected	1.00	
Satd. Flow (prot)	3170	
Flt Permitted	1.00	
Satd. Flow (perm)	3170	
Peak-hour factor, PHF	0.92	0.92
Adj. Flow (vph)	7	16
RTOR Reduction (vph)	15	0
Lane Group Flow (vph)	8	0
Turn Type	NA	
Protected Phases	3	
Permitted Phases		
Actuated Green, G (s)	8.2	
Effective Green, g (s)	8.2	
Actuated g/C Ratio	0.07	
Clearance Time (s)	6.0	
Vehicle Extension (s)	3.0	
Lane Grp Cap (vph)	216	
v/s Ratio Prot	0.00	
v/s Ratio Perm		
v/c Ratio	0.04	
Uniform Delay, d1	52.2	
Progression Factor	1.00	
Incremental Delay, d2	0.1	
Delay (s)	52.3	
Level of Service	D	
Approach Delay (s)	55.6	
Approach LOS	E	
Intersection Summary		

## Intersection: 1: I-95 SB On-Ramp & Route 54

Movement	WB	WB	B33	B35_	B35	SB	SB	B28
Directions Served	L	T	Т	Т	Т	LT	R	Т
Maximum Queue (ft)	108	6	5	13	2	152	151	430
Average Queue (ft)	42	0	0	0	0	69	25	88
95th Queue (ft)	85	4	5	13	2	150	127	489
Link Distance (ft)	98	98	206	151	151	103	103	987
Upstream Blk Time (%)	1					23		4
Queuing Penalty (veh)	3					0		0
Storage Bay Dist (ft)								
Storage Blk Time (%)								
Queuing Penalty (veh)								

# Intersection: 2: I-95 NB On-Ramp & Route 54

Movement	EB	EB	EB	WB	WB
Directions Served	L	Т	Т	Ţ	Т
Maximum Queue (ft)	158	179	83	1	30
Average Queue (ft)	79	58	27	0	2
95th Queue (ft)	141	112	60	1	17
Link Distance (ft)		179	179	146	146
Upstream Blk Time (%)	0	0			
Queuing Penalty (veh)	0	1			
Storage Bay Dist (ft)	300				
Storage Blk Time (%)	0	0			
Queuing Penalty (veh)	0	1			

## Intersection: 3: Route 54 & Woodside Lane

Movement	EB	WB	SB	SB
Directions Served	L,	TR	L	R
Maximum Queue (ft)	72	2	51	68
Average Queue (ft)	26	0	14	32
95th Queue (ft)	57	2	37	53
Link Distance (ft)		529	891	
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	225			200
Storage Blk Time (%)				
Queuing Penalty (veh)				

# Intersection: 4: Route 54 & Providence Church Road

Movement	EB	SB
Directions Served	L	LR
Maximum Queue (ft)	18	33
Average Queue (ft)	2	7
95th Queue (ft)	11	27
Link Distance (ft)		1008
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	200	
Storage Blk Time (%)		
Queuing Penalty (veh)		

# Intersection: 5: Route 54 & Site Driveway

Movement	EB	SB
Directions Served	L	LR
Maximum Queue (ft)	29	38
Average Queue (ft)	3	13
95th Queue (ft)	17	39
Link Distance (ft)		539
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	200	
Storage Blk Time (%)		
Queuing Penalty (veh)		

## Intersection: 6: Goddins Hill Road & Route 54

Movement	EB	WB	NB
Directions Served	R	LT	LR
Maximum Queue (ft)	10	63	93
Average Queue (ft)	0	10	38
95th Queue (ft)	6	40	69
Link Distance (ft)		1778	529
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	150		
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 22: Route 54

Movement	EB	SW	B24
Directions Served	T	R	T
Maximum Queue (ft)	35	269	306
Average Queue (ft)	1	150	37
95th Queue (ft)	17	273	187
Link Distance (ft)	151	166	852
Upstream Blk Time (%)		16	
Queuing Penalty (veh)		0	
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 31: Route 54

Movement	EB	EB	WB	WB	WB
Directions Served	Т	R	Т	Т	Т
Maximum Queue (ft)	7	350	36	45	42
Average Queue (ft)	0	123	17	15	2
95th Queue (ft)	7	260	39	43	20
Link Distance (ft)	610	610	155	155	155
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)					
Storage Blk Time (%)					
Queuing Penalty (veh)					

Intersection: 32: I-95 SB On-Ramp

Movement	SE
Directions Served	L
Maximum Queue (ft)	152
Average Queue (ft)	57
95th Queue (ft)	116
Link Distance (ft)	215
Upstream Blk Time (%)	0
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 34: Hill Carter Parkway	& England Street/Route 54
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Movement	EB	EB	EB	E8	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	UL	Т	Т	Т	R	UL	L	Т	Т	TR	L	LT
Maximum Queue (ft)	86	283	271	333	215	162	154	203	222	230	146	178
Average Queue (ft)	34	163	133	176	44	80	80	96	112	124	21	96
95th Queue (ft)	74	256	252	289	150	137	134	172	192	211	96	163
Link Distance (ft)		632	632	632				610	610	610		352
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225				150	375	375				250	
Storage Blk Time (%)		2		12								0
Queuing Penalty (veh)		1		13								0

# Intersection: 34: Hill Carter Parkway & England Street/Route 54

Movement	NB	NB	SB	SB	SB
Directions Served	Т	R	L	Т	TR
Maximum Queue (ft)	18	119	109	46	39
Average Queue (ft)	1	42	44	7	12
95th Queue (ft)	14	87	90	29	35
Link Distance (ft)	352			429	429
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		250	200		
Storage Blk Time (%)					
Queuing Penalty (veh)					

## **Network Summary**

Network wide Queuing Penalty: 19

	-	-	4-	<b>↓</b>	1
Lane Group	EBT	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	434	402	1063	109	260
v/c Ratio	0.35	0.82	0.43	0.40	0.79
Control Delay	16.0	47.2	6.9	40.3	37.3
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	16.0	47.2	6.9	40.3	37.3
Queue Length 50th (ft)	55	238	121	63	85
Queue Length 95th (ft)	60	319	214	104	160
Internal Link Dist (ft)	190		68	110	
Turn Bay Length (ft)					
Base Capacity (vph)	1237	619	2492	426	445
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.35	0.65	0.43	0.26	0.58
Intersection Summary					

	*	-	-	1	-	4	4	<b>†</b>	-	-	<b>↓</b>	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>		ħ	<b>†</b> †						લી	7
Traffic Volume (vph)	0	404	0	374	989	0	0	0	0	100	1	242
Future Volume (vph)	0	404	0	374	989	0	0	0	0	100	1	242
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0		6.0	6.0						6.0	6.0
Lane Util. Factor		0.95		1.00	0.95						1.00	1.00
Frt		1.00		1.00	1.00						1.00	0.85
Flt Protected		1.00		0.95	1.00						0.95	1.00
Satd. Flow (prot)		3195		1770	3438						1775	1468
Flt Permitted		1.00		0.95	1.00						0.95	1.00
Satd. Flow (perm)		3195		1770	3438						1775	1468
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	0	434	0	402	1063	0	0	0	0	108	1	260
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	103
Lane Group Flow (vph)	0	434	0	402	1063	0	0	0	0	0	109	157
Heavy Vehicles (%)	2%	13%	2%	2%	5%	2%	2%	2%	2%	2%	2%	10%
Turn Type		NA		Prot	NA					Split	NA	Perm
Protected Phases		2		1	6					. 4	4	
Permitted Phases												4
Actuated Green, G (s)		38.7		27.8	72.5						15.5	15.5
Effective Green, g (s)		38.7		27.8	72.5						15.5	15.5
Actuated g/C Ratio		0.39		0.28	0.72						0.16	0.16
Clearance Time (s)		6.0		6.0	6.0						6.0	6.0
Vehicle Extension (s)		3.0		3.0	3.0						3.0	3.0
Lane Grp Cap (vph)		1236		492	2492						275	227
v/s Ratio Prot		0.14		c0.23	c0.31						0.06	
v/s Ratio Perm												c0.11
v/c Ratio		0.35		0.82	0.43						0.40	0.69
Uniform Delay, d1		21.7		33.7	5.5						38.0	40.0
Progression Factor		0.61		1.00	1.00						1.00	1.00
Incremental Delay, d2		0.7		10.1	0.5						0.9	8.8
Delay (s)		14.0		43.9	6.0						39.0	48.7
Level of Service		В		D	Α						D	D
Approach Delay (s)		14.0			16.4			0.0			45.9	
Approach LOS		В			8			Α			D	
Intersection Summary												
HCM 2000 Control Delay			20.7	Н	ICM 2000	Level of S	Service		С			
HCM 2000 Volume to Capa	city ratio		0.62									
Actuated Cycle Length (s)			100.0		um of lost				18.0			
Intersection Capacity Utiliza	tion		52.5%	IC	CU Level of	of Service			Α			
Analysis Period (min)			15									
c Critical Lane Group												

Intersection											
Int Delay, s/veh	0										
Movement	EBL	EBT	E8R	WBL	WBT	WBR	SBL	SBR	NEL	NER	
Lane Configurations	7	<b>^</b>			44	7				7	
Traffic Vol, veh/h	160		0	0	688	82	0	0	0	241	
Future Vol, veh/h	160		0	0	688	82	Ŏ	Õ	Õ	241	
Conflicting Peds, #/hr	0		0	0	0	0	0	0	ő	0	
Sign Control	Stop		Stop	Free	Free	Free	Free	Free	Free	Free	
RT Channelized	Otop	Otop -	None	1100	1100	Yield	1100	1166	1100	Free	
Storage Length	300		NONE	-	_	0	-	•	•	0	
Veh in Median Storage			-	-	0	-	0	-	0	U	
•		0	-	-			0		0	-	
Grade, %	- 02	-	- 02	- 02	0	- 00		- 00		00	
Peak Hour Factor	93		93	93	93	93	93	93	93	93	
Heavy Vehicles, %	20		2	2	2	16	2	2	2	6	
Mvmt Flow	172	370	0	0	740	88	0	0	0	259	
B 4 3 46 82											
	Minor2		ا	Major2		_					
Conflicting Flow All	740		-	-	-	0					
Stage 1	740		-	-	-	-					
Stage 2	0		-	•	•	-					
Critical Hdwy	7.2	6.6	-	-	-	-					
Critical Hdwy Stg 1	6.2	5.6	-	-	-	-					
Critical Hdwy Stg 2	-	-	-	-	-	-					
Follow-up Hdwy	3.7	4.05	-	-	-	-					
Pot Cap-1 Maneuver	316	~ 337	0	0	-	-					
Stage 1	388	414	0	0	-	-					
Stage 2	-	_	0	0	_	-					
Platoon blocked, %					-	•					
Mov Cap-1 Maneuver	316	0	_	_	_	-					
Mov Cap-2 Maneuver			_	_	_	_					
Stage 1	388		_	_	_	_					
Stage 2	-	0	_	_	_	_					
Stage 2	_	U	_	_	_	_					
Approach	EB			WB							
HCM Control Delay, s				0							- · · - · · · · · · · · · · · · · · · ·
HCM LOS				U							
HOW LOS	-										
Minor Lane/Major Mvn	nt	EBLn1	ERI no l	EBI 52	\ <i>\\</i>   <b>Q</b> T	\M/QD					
	iit.				AADI	AADIZ					
Capacity (veh/h)		316	-	-	-	-					
HCM Cantral Dalay (a		0.544	-	•	•	-					
HCM Control Delay (s	)	29.2	-	-	-	-					
HCM Lane LOS		D	-	-	-	-					
HCM 95th %tile Q(veh	1)	3.1	-	•	-	-					
Notes											
~: Volume exceeds ca	pacity	\$: De	elay exc	ceeds 3	00s	+: Com	putation	Not D	efined	*: All	major volume in platoon

				-		
5.7						
EBL	EBT	WBT	WBR	SBL	SBR	
**	<b>^</b>	1		ሻ	7	
44	182	360	23	60	248	
44	182	360	23	60	248	
0	0	0	0	0	0	
Free		Free		Stop		
•	None	-	None	-		
	-	-	-		200	
e,# -	0	0	-		-	
•	-		-		-	
51	209	414	26	69	285	
Major1		Major2		Minor2		
440	0	-	0	738	427	
-	-	-	-	427	-	
-	•	-	•	311		
4.13	-	-	-	6.46	6.22	
-	-	-	-		-	
•	-	•	-		•	
	-	-	-			
1115	-	-	-		628	
-	-	-	-		•	
-	-	-	-	734	-	
4445	-	-	-		000	
1115	-	-	-		628	
-	-	-			-	
-	-	-	-		-	
-	-	-	-	/34	-	
EB		WB		SB		
1.6		0		15.8		
				С		
nt	EBL	EBT	WBT	WBR	SBLn1	SBLn2
-						628
			-	_		
)		-	_	_		15.4
,		-	-	_		C
1)		-		_		2.4
•						
	EBL  44  44  0 Free  225 e, # -  87  3 51  Major1  440  -  4.13  -  1115  -  1115  -  116  EBB  1.6	EBL EBT  44 182 44 182 0 0 Free Free - None 225 e, # - 0 87 87 3 7 51 209  Major1	EBL EBT WBT  44 182 360 44 182 360 0 0 0 Free Free Free - None 225 2, # - 0 0 87 87 87 3 7 2 51 209 414  Major1 Major2  440 0 1115 1115	EBL EBT WBT WBR  44 182 360 23 44 182 360 23 0 0 0 0 0 Free Free Free Free - None - None 225 0 0 - 87 87 87 87 3 7 2 28 51 209 414 26  Major1 Major2  440 0 - 0 1115 1115 1115 1115 1115 1115	EBL         EBT         WBT         WBR         SBL           44         182         360         23         60           44         182         360         23         60           0         0         0         0         0           Free         Free         Free         Stop         -           None         -         None         -         0           225         -         -         0         -         0           87         87         87         87         87         87           3         7         2         28         6         6           51         209         414         26         69           Major1         Major2         Minor2         Minor2           440         0         -         0         738           -         -         -         427         -         427           -         -         -         427         -         427           -         -         -         -         5.46           2.227         -         -         3.554           1115         -	EBL EBT WBT WBR SBL SBR  44 182 360 23 60 248 0 0 0 0 0 0 0 0 Free Free Free Free Stop Stop - None - None - None 225 0 200 e, # - 0 0 - 0 - 0 87 87 87 87 87 87 87 3 7 2 28 6 2 51 209 414 26 69 285  Major1 Major2 Minor2  440 0 - 0 738 427 427 - 311 - 4.13 - 6.46 6.22 5.46 - 2.227 - 3.554 3.318 1115 5.46 - 15.46 5.46 - 15.46 734 - 1115 362 628 734 - 1115 362 628 1.6 0 15.8 C  mt EBL EBT WBT WBR SBLn1 9 0.045 0.191 0.84 17.3 A C

HCM 6th TWSC

Synchro 11 Report Page 4

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations				VVDI		SDR
Traffic Vol, veh/h	7	225	360	4	W	4.4
	7	235	369	1	1	14
Future Vol, veh/h	7	235	369	1	1	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	200	•	•	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	2	7	2	2	2	2
Mvmt Flow	8	270	424	1	1	16
					·	
h.d.=5==66.45====	Matasa.		A-to-O		h.ii	
	Major1		Major2		Minor2	
Conflicting Flow All	425	0	•	0		425
Stage 1	-	-	-	-	425	-
Stage 2	-	-	-	-	286	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-		_	5.42	-
Follow-up Hdwy	2.218	-	-	_	3.518	3.318
Pot Cap-1 Maneuver	1134	_	_	_	400	629
Stage 1					659	
Stage 2	_	_	_	_	763	_
Platoon blocked, %	-	_	_	_	700	
_	4494	-	-		207	620
Mov Cap-1 Maneuver	1134	-	-	-	397	629
Mov Cap-2 Maneuver	-	-	-	-	397	-
Stage 1	-	-	-	-	654	-
Stage 2	-	-	-	-	763	-
Approach	ЕΒ		WB		SB	
HCM Control Delay, s	0.2		0		11.1	
HCM LOS					В	
				141		on
Minor Lane/Major Mvn	ıt .	EBL	EBT	WBT	WBR	
Capacity (veh/h)		1134	-	-	-	605
HCM Lane V/C Ratio		0.007	-	-	-	0.028
HCM Control Delay (s)	)	8.2	-	-	-	11.1
HCM Lane LOS		Α	-	-	-	В
HCM 95th %tile Q(veh	)	0		_	_	0.1
	•					

ntersection													
nt Delay, s/veh	0.8												
Novement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL.	SBT	SBR	
ane Configurations	ħ	<b>\$</b>		ሻ	<b>\$</b>			4			4		
raffic Vol, veh/h	9	227	0	17	350	3	0	0	0	5	0	20	
uture Vol, veh/h	9	227	0	17	350	3	0	0	0	5	0	20	
onflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
ign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
T Channelized	-	-	None	-	-	None	-	-	None	_	-	None	
torage Length	200	-	-	200		-	-	-	-	-			
eh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-	
rade, %	_	0	-	-	0	-	-	0	-	_	0	_	
eak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	
eavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
vmt Flow	10	247	0	18	380	3	0	0	0	5	0	22	
lajor/Minor l	Major1	r1 Major2				!	Minor1	Minor2					
onflicting Flow All	383	0	0	247	0	0	696	686	247	685	685	382	
Stage 1	-	-	-	-	-	-	267	267	-	418	418	-	
Stage 2	-	-	-	-	-	-	429	419	_	267	267	-	
ritical Hdwy	4.12	-		4.12		-	7.12	6.52	6.22	7.12	6.52	6.22	
ritical Hdwy Stg 1	_	-	_	-	-	-	6.12	5.52	-	6.12	5.52	_	
ritical Hdwy Stg 2	_	_	_	_	_	-	6.12	5.52	_	6.12	5.52	_	
ollow-up Hdwy	2.218	-	_	2.218		-	3.518	4.018	3.318	3.518	4.018	3.318	
ot Cap-1 Maneuver	1175	_	-	1319	-	-	356	370	792	362	371	665	
Stage 1	_	_	-	-	-	_	738	688	_	612	591	_	
Stage 2		-	-	_	-	-	604	590	-	738	688	-	
latoon blocked, %		-	_		-	-							
ov Cap-1 Maneuver	1175		_	1319	-	-	339	361	792	356	362	665	
lov Cap-2 Maneuver		_	_	-	-		339	361	-	356	362	-	
Stage 1	_	_	_	_	_	_	731	682	_	606	583	_	
Stage 2	-	-	-	-	-	-	576	582	-	732	682	-	
-													
pproach	EB			WB			NB			SB			
CM Control Delay, s	0.3			0.4			0			11.7			
CM LOS							Α			В			
Almond and 46 Anton 6.4		IDI 4	E0:	r.p.z.	FDD	14851	LAUDT	MDD	001 4				
linor Lane/Major Mvm	IL N	IBLn1	EBL	EBT	EBR		WRI	WBR :				_	
apacity (veh/h)		-	1175	-	-	1319	•	-	567				
CM Lane V/C Ratio		-	0.008	-	-	0.014	-	-	0.048				
ICM Control Delay (s)		0	8.1	•	-	7.8	-	•	11.7				
ICM Lane LOS		Α	Α	-	-	Α	-	-	В				
ICM 95th %tile Q(veh)	1	_	0	_	_	0	_	-	0.2				

	۶	-	*	*	<b>—</b>	4	1	1	-	<b>↓</b>	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Group Flow (vph)	22	1118	33	168	1180	29	34	42	108	11	
v/c Ratio	0.15	0.44	0.04	0.47	0.38	0.22	0.12	0.15	0.61	0.02	
Control Delay	45.3	18.0	0.1	49.7	9.8	46.6	43.2	1.1	57.6	0.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	45.3	18.0	0.1	49.7	9.8	46.6	43.2	1.1	57.6	0.0	
Queue Length 50th (ft)	13	171	0	42	112	19	11	0	66	0	
Queue Length 95th (ft)	38	230	0	88	197	49	27	0	123	0	
Internal Link Dist (ft)		601			622		348			415	
Turn Bay Length (ft)	225		150	375		250		250	200		
Base Capacity (vph)	230	2541	873	446	3077	289	585	419	194	717	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.10	0.44	0.04	0.38	0.38	0.10	0.06	0.10	0.56	0.02	
Intersection Summary											

	*	-	7	•	<b>—</b>	*	1	<b>†</b>	-	-	1	1
Movement	EBL	EBT	EBR	WBL.	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ä	ተተተ	7	ሽኘ	444	,	Ť	41	7	7	₫₽	
Traffic Volume (vph)	20	1029	30	155	1079	6	53	5	39	99	2	8
Future Volume (vph)	20	1029	30	155	1079	6	53	5	39	99	2	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0	6.0	6.0	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91		0.91	0.91	1.00	1.00	0.95	
Frt	1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85	1.00	0.88	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00	0.95	1.00	
Satd. Flow (prot)	1770	5085	1583	3433	5081		1610	3252	1583	1770	3105	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	0.96	1.00	0.95	1.00	
Satd. Flow (perm)	1770	5085	1583	3433	5081		1610	3252	1583	1770	3105	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	22	1118	33	168	1173	7	58	5	42	108	2	9
RTOR Reduction (vph)	0	0	17	0	0	0	0	0	39	0	10	0
Lane Group Flow (vph)	22	1118	16	168	1180	0	29	34	3	108	1	0
Turn Type	Prot	NA	Perm	Prot	NA		Split	NA	Perm	Split	NA	
Protected Phases	5	2		1	6		4	4		3	3	
Permitted Phases	_	_	2	•	_		-		4	_	_	
Actuated Green, G (s)	3.3	48.8	48.8	10.3	55.8		6.8	6.8	6.8	10.1	10.1	
Effective Green, g (s)	3.3	48.8	48.8	10.3	55.8		6.8	6.8	6.8	10.1	10.1	
Actuated g/C Ratio	0.03	0.49	0.49	0.10	0.56		0.07	0.07	0.07	0.10	0.10	
Clearance Time (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	58	2481	772	353	2835		109	221	107	178	313	
v/s Ratio Prot	0.01	c0.22	*	c0.05	c0.23		c0.02	0.01		c0.06	0.00	
v/s Ratio Perm			0.01		00.20				0.00			
v/c Ratio	0.38	0.45	0.02	0.48	0.42		0.27	0.15	0.03	0.61	0.00	
Uniform Delay, d1	47.3	16.8	13.2	42.3	12.7		44.2	43.9	43.5	43.0	40.4	
Progression Factor	1.00	1.00	1.00	1.09	0.77		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	4.1	0.6	0.0	0.9	0.1		1.3	0.3	0.1	5.7	0.0	
Delay (s)	51.5	17.4	13.3	46.9	9.9		45.5	44.2	43.6	48.8	40.4	
Level of Service	D	В	В	D	A		D	D	D	D	D	
Approach Delay (s)		17.9		_	14.5			44.3	_		48.0	
Approach LOS		В			В			D			D	
Intersection Summary												
HCM 2000 Control Delay			18.6	Н	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capacity ratio			0.46									
Actuated Cycle Length (s)			100.0	S	um of lost	time (s)			24.0			
Intersection Capacity Utilization			59.9%		U Level o				В			
Analysis Period (min)			15									

Intersection: 1: I-95 SB On-Ramp & Route 54

Movement	EB	EB	WB	WB	WB	B33	B33	B33	B35	B35	B35	SB
Directions Served	Т	Т	L	Т	Т	Т	Т	Т	Т	Т	Т	LT
Maximum Queue (ft)	181	159	193	141	157	279	5	15	146	148	105	150
Average Queue (ft)	120	72	163	53	62	104	0	1	17	14	13	75
95th Queue (ft)	190	158	199	118	132	268	4	10	94	102	95	134
Link Distance (ft)	155	155	98	98	98	206	206	206	151	151	151	103
Upstream Blk Time (%)	9	1	55	1	2	11			3	2	2	5
Queuing Penalty (veh) Storage Bay Dist (ft)	18	1	249	5	8	38			12	9	7	0

Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 1: I-95 SB On-Ramp & Route 54

Movement	SB	B28
Directions Served	R	T
Maximum Queue (ft)	59	42
Average Queue (ft)	3	3
95th Queue (ft)	41	26
Link Distance (ft)	103	987
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 2: I-95 NB On-Ramp & Route 54

Movement	EB	EB	EB	WB	WB
Directions Served	L	Т	T	T	T
Maximum Queue (ft)	158	159	116	8	55
Average Queue (ft)	74	57	26	1	7
95th Queue (ft)	141	111	67	4	33
Link Distance (ft)		179	179	146	146
Upstream Blk Time (%)	0	0	0		
Queuing Penalty (veh)	0	0	0		
Storage Bay Dist (ft)	300				
Storage Blk Time (%)	0	0			
Queuing Penalty (veh)	0	0			

# Intersection: 3: Route 54 & Woodside Lane

Movement	EB	SB	SB
Directions Served	L	L	R
Maximum Queue (ft)	48	72	126
Average Queue (ft)	12	23	57
95th Queue (ft)	35	52	101
Link Distance (ft)		891	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	225		200
Storage Blk Time (%)			
Queuing Penalty (veh)			

# Intersection: 4: Route 54 & Providence Church Road

Movement	EB	SB
Directions Served	L	LR
Maximum Queue (ft)	26	31
Average Queue (ft)	2	10
95th Queue (ft)	12	32
Link Distance (ft)		1008
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	200	
Storage Blk Time (%)		
Queuing Penalty (veh)		

# Intersection: 5: Route 54 & Site Driveway

Movement	EB	WB	SB
Directions Served	L	L	LTR
Maximum Queue (ft)	22	26	40
Average Queue (ft)	2	2	18
95th Queue (ft)	13	14	43
Link Distance (ft)			540
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	200	200	
Storage Blk Time (%)			
Queuing Penalty (veh)			

# Intersection: 6: Goddins Hill Road & Route 54

Movement	EB	WB	NB	
Directions Served	R	LT	LR	
Maximum Queue (ft)	2	64	64	
Average Queue (ft)	0	17	27	
95th Queue (ft)	2	50	55	
Link Distance (ft)		1778	529	
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	150			
Storage Blk Time (%)				
Queuing Penalty (veh)				

# Intersection: 22: Route 54

Movement	EB	EB	WB	WB	SW	B24
Directions Served	Т	Т	Ŧ	Т	R	T
Maximum Queue (ft)	26	6	52	43	219	113
Average Queue (ft)	1	0	4	5	70	11
95th Queue (ft)	14	6	43	50	174	130
Link Distance (ft)	151	151	179	179	166	852
Upstream Blk Time (%)			0	0	3	
Queuing Penalty (veh)			0	1	0	
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

## Intersection: 31: Route 54

Movement	EB	E8	EB	WB	WB	WB
Directions Served	T	T	R	T	T	T
Maximum Queue (ft)	168	119	342	38	51	31
Average Queue (ft)	27	9	146	14	9	1
95th Queue (ft)	108	62	285	36	37	15
Link Distance (ft)	610	610	610	155	155	155
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 32: I-95 SB On-Ramp

Directions Served L  Maximum Queue (ft) 230  Average Queue (ft) 204  95th Queue (ft) 267  Link Distance (ft) 215  Upstream Blk Time (%) 12
Average Queue (ft) 204 95th Queue (ft) 267 Link Distance (ft) 215
95th Queue (ft) 267 Link Distance (ft) 215
Link Distance (ft) 215
Upstream Blk Time (%) 12
Queuing Penalty (veh) 46
Storage Bay Dist (ft)
Storage Blk Time (%)
Queuing Penalty (veh)

# Intersection: 34: Hill Carter Parkway & England Street/Route 54

Movement	EΒ	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	UL	Т	Т	Т	R	ŲL	L	Т	Ť	TR	L	LT
Maximum Queue (ft)	56	226	302	335	162	90	106	180	189	196	8	98
Average Queue (ft)	19	111	114	183	16	37	63	68	76	81	0	37
95th Queue (ft)	46	191	259	301	81	77	99	143	158	162	4	79
Link Distance (ft)		632	632	632				610	610	610		352
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225				150	375	375				250	
Storage Blk Time (%)		0		13				0				
Queuing Penalty (veh)		0		4				0				

# Intersection: 34: Hill Carter Parkway & England Street/Route 54

Movement	NB	NB	SB	\$B	SB
Directions Served	Т	R	L	Т	TR
Maximum Queue (ft)	4	61	141	20	32
Average Queue (ft)	0	17	66	1	7
95th Queue (ft)	2	43	120	11	27
Link Distance (ft)	352			429	429
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		250	200		
Storage Blk Time (%)			0		
Queuing Penalty (veh)			0		

# **Network Summary**

Network wide Queuing Penalty: 398

	<b>→</b>	•	4-	<b>↓</b>	1
Lane Group	EBT	WBL	WBT	SBT	SBR
Lane Group Flow (vph)	727	303	1281	86	282
v/c Ratio	0.50	0.80	0.52	0.27	0.85
Control Delay	38.8	60.6	9.3	42.3	55.2
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	38.8	60.6	9.3	42.3	55.2
Queue Length 50th (ft)	297	224	212	57	153
Queue Length 95th (ft)	376	304	318	99	243
Internal Link Dist (ft)	190		68	110	
Turn Bay Length (ft)					
Base Capacity (vph)	1454	486	2470	428	416
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.50	0.62	0.52	0.20	0.68
Intersection Summary					

1: I-95 SB On-Ramp & Route 54

1. 1-33 OB OH-I Kamp	Q 1 (O	ato o-	•							iting i iai		
	*	-	7	•	<b>—</b>	*	4	<b>†</b>	-	-	<b>↓</b>	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>^</b>		ሻ	<b>1</b>						લ	To the same
Traffic Volume (vph)	0	676	0	282	1191	0	0	0	0	79	1	262
Future Volume (vph)	0	676	0	282	1191	0	0	0	0	79	1	262
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0		6.0	6.0						6.0	6.0
Lane Util. Factor		0.95		1.00	0.95						1.00	1.00
Frt		1.00		1.00	1.00						1.00	0.85
Flt Protected		1.00		0.95	1.00						0.95	1.00
Satd. Flow (prot)		3195		1770	3438						1775	1468
Flt Permitted		1.00		0.95	1.00						0.95	1.00
Satd. Flow (perm)		3195		1770	3438						1775	1468
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	0	727	0	303	1281	0	0	0	0	85	1	282
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	67
Lane Group Flow (vph)	0	727	0	303	1281	0	0	0	0	0	86	215
Heavy Vehicles (%)	2%	13%	2%	2%	5%	2%	2%	2%	2%	2%	2%	10%
Turn Type		NA		Prot	NA					Split	NA	Perm
Protected Phases		2		1	6					4	4	
Permitted Phases												4
Actuated Green, G (s)		54.6		25.6	86.2						21.8	21.8
Effective Green, g (s)		54.6		25.6	86.2						21.8	21.8
Actuated g/C Ratio		0.46		0.21	0.72						0.18	0.18
Clearance Time (s)		6.0		6.0	6.0						6.0	6.0
Vehicle Extension (s)		3.0		3.0	3.0						3.0	3.0
Lane Grp Cap (vph)		1453		377	2469						322	266
v/s Ratio Prot		0.23		c0.17	c0.37						0.05	200
v/s Ratio Perm		0.20		00.11	00.01						0.00	c0.15
v/c Ratio		0.50		0.80	0.52						0.27	0.81
Uniform Delay, d1		23.1		44.8	7.6						42.2	47.1
Progression Factor		1.46		1.00	1.00						1.00	1.00
Incremental Delay, d2		1.1		11.7	0.8						0.4	16.3
Delay (s)		34.9		56.5	8.4						42.7	63.4
Level of Service		C		E	A						D	E
Approach Delay (s)		34.9		_	17.6			0.0			58.5	_
Approach LOS		C			В			A			E	
Intersection Summary												
HCM 2000 Control Delay			27.9	H	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacity	y ratio		0.66									
Actuated Cycle Length (s)			120.0	S	um of lost	t time (s)			18.0			
Intersection Capacity Utilizatio	ก		59.1%		CU Level o		:		В			
Analysis Period (min)			15									
c Critical Lane Group												

Intersection													
Int Delay, s/veh	0												
Movement	EBL.	EBT	EBR	WBL	WBT	WBR	SBL	SBR	NEL	NER			
Lane Configurations	7	44			44	77				7			
Traffic Vol, veh/h	270	485	0	0	593	125	0	0	0	295			
Future Vol, veh/h	270	485	0	0	593	125	0	0	0	295			
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0			
Sign Control	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free			
RT Channelized	-	-	None	-	-	Yield	-	-	-	Free			
Storage Length	300				-	0	-		-	0			
Veh in Median Storage	.,# -	0	-	-	0	-	0	-	0	-			
Grade, %	_	0	-	-	0	-	0	-	0	-			
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93			
Heavy Vehicles, %	20	5	2	2	2	16	2	2	2	6			
Mvmt Flow	290	522	0	0	638	134	0	0	0	317			
Major/Minor I	Minor2		ı	Major2									
Conflicting Flow All	638	638				0		** *					
Stage 1	638	638	-	-	-	_							
Stage 2	0	0	_	-	-	_							
Critical Hdwy	7.2	6.6	-	-		-							
Critical Hdwy Stg 1	6.2	5.6	-	-	-	-							
Critical Hdwy Stg 2	-	_	_	-	-	_							
Follow-up Hdwy	3.7	4.05			_	-							
Pot Cap-1 Maneuver	370	~ 387	0	0	-	-							
Stage 1	442	~ 462	0	0	-	-							
Stage 2	-	-	0	0	_	-							
Platoon blocked, %					-	-							
Mov Cap-1 Maneuver	370	0	-	-	-	-							
Mov Cap-2 Maneuver	370	0	-	-	-	-							
Stage 1	442	0	-	-	-	-							
Stage 2	-	0	-	-	-	-							
Approach	EB			WB									
HCM Control Delay, s				0									
HCM LOS													
Minor Lane/Major Mvm	nt I	EBLn1 l	EBLn2 I	EBLn3	WBT	WBR							
Capacity (veh/h)		370	-	_		_							
HCM Lane V/C Ratio		0.785	-	_	_	_							
HCM Control Delay (s)		42.3				-							
HCM Lane LOS		E	-	-	-	-							
HCM 95th %tile Q(veh)	)	6.6	-	-	-	-							
, ,	•												
Notes		A C	.1	^.	00-			Maria	. C 1	4. 40		alata a c	
~: Volume exceeds cap	pacity	\$: De	elay exc	eeds 3	UUS	+: Com	putation	I NOT D	etined	r: All i	najor volume in p	olatoon	

Intersection							
Int Delay, s/veh	3.6						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	T T	<u> </u>	1	VVDIX	JOE 1	7 JUNE	-
Traffic Vol, veh/h	175	384	282	12	28	111	
Future Vol, veh/h	175	384	282	12	28	111	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	225	-			0	200	
Veh in Median Storage		0	0	-	0	-	
Grade, %		0	0	-	0	_	
Peak Hour Factor	87	87	87	87	87	87	
Heavy Vehicles, %	3	7	2	28	6	2	
Mvmt Flow	201	441	324	14	32	128	
Major/Minor	Major1		Major2	1	Minor2		
Conflicting Flow All	338	0	viajoi z	0		331	
Stage 1	-	-		-	331	-	
Stage 2		-		_	843	_	
Critical Hdwy	4.13	-		_	6.46	6.22	
Critical Hdwy Stg 1	-	-	-	_	5.46	-	
Critical Hdwy Stg 2		-		-	5.46	_	
Follow-up Hdwy	2.227	_	-	_	3.554	3.318	
Pot Cap-1 Maneuver	1216	_	-	-	208	711	
Stage 1	-	-	•	•	719	-	
Stage 2	-	-	-	-	416	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1216	-	-	-	174	711	
Mov Cap-2 Maneuver	-	-	-	-	174	-	
Stage 1	-			-	600	-	
Stage 2	-	-	-	-	416	-	
Approach	EB		WB		SB		
HCM Control Delay, s	2.7	-	0		15		
HCM LOS	2.1		0		C		
841 1 184 1 24				LAPOT	14155	0DL 4.5	VDI A
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1 S	
Capacity (veh/h)		1216	-	-	-	174	711
HCM Lane V/C Ratio		0.165	-	-	-	0.185	
HCM Control Delay (s	)	8.5	-	-	-	30.3	11.2
HCM Lane LOS	,	A	-	-	-	D	В
HCM 95th %tile Q(veh	))	0.6	-	-	-	0.7	0.7

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
				VVDIX		SDR
Lane Configurations	<b>1</b> 3	401	7	2	W	40
Traffic Vol, veh/h	12	401	284	2	1	10
Future Vol, veh/h	12	401	284	2	1	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None	•	None	•	None
Storage Length	200	-	-	-	0	-
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	2	7	2	2	2	2
Mvmt Flow	14	461	326	2	1	11
Major/Minor !	Major1	٨	Major2	- 1	Minor2	
Conflicting Flow All	328	0	-	0	816	327
Stage 1	320	-	_	-	327	J21 -
Stage 2	-	•	•	-	489	-
Critical Hdwy	4.12	-	-		6.42	6.22
Critical Hdwy Stg 1	4.12	-	•	-	5.42	0.22
	•	-	•	•	5.42	-
Critical Hdwy Stg 2	2 240	-	-	-		2 240
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1232	-	•	•	347	714
Stage 1	-	-	-	-	731	-
Stage 2	-	-	-	-	616	-
Platoon blocked, %	4000	44	•	-	0.40	744
Mov Cap-1 Maneuver	1232	-	-	-	343	714
Mov Cap-2 Maneuver	~	-	4	-	343	-
Stage 1	-	-	-	-	723	-
Stage 2	-	-	-	-	616	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.2		0		10.6	
HCM LOS	0.2		v		В	
TOW LOO					D	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	
Capacity (veh/h)		1232	-	-	-	650
HCM Lane V/C Ratio		0.011	-	•	•	0.019
HCM Control Delay (s)	)	8	-	-	-	10.6
HCM Lane LOS		Α	-	-	-	В
HCM 95th %tile Q(veh)	)	0	-		-	0.1
, ,						

			<del>-</del>					<del></del>					
Intersection	0.0												
Int Delay, s/veh	8.0												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	7	B		7	Þ			4			4		
Traffic Vol, veh/h	20	382	0	7	273	6	0	0	16	4	0	13	
Future Vol, veh/h	20	382	0	7	273	6	0	0	16	4	0	13	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	•	None		-	None	-	-	None	-	-	None	
Storage Length	200	-	-	200	-	-	-	-	-	-	-	-	
Veh in Median Storage	,# -	0	-	-	0	-	-	0	•	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	22	415	0	8	297	7	0	0	17	4	0	14	
Major/Minor I	Major1		1	Major2		1	Minor1			Minor2			
Conflicting Flow All	304	0	0	415	0	0	783	779	415	785	776	301	
Stage 1	004	-	-	710	0	•	459	459	-	317	317	-	
Stage 2	_	_	_	_	_	_	324	320	_	468	459	-	
Critical Hdwy	4.12	_		4.12	_	_	7.12	6.52	6.22	7.12	6.52	6.22	
Critical Hdwy Stg 1	7.12			7.12			6.12	5.52	0.22	6.12	5.52	0.22	
Critical Hdwy Stg 2	_	_	_	_	_	_	6.12	5.52	_	6.12	5.52	_	
Follow-up Hdwy	2.218	_	_	2.218	_		3.518	4.018	3.318	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1257			1144			311	327	637	310	328	739	
Stage 1	-	_	-	-			582	566	-	694	654	-	
Stage 2	_	-	_	_			688	652	_	575	566	_	
Platoon blocked, %		_			_	_	***				•••		
Mov Cap-1 Maneuver	1257	-	-	1144	-	-	299	319	637	296	320	739	
Mov Cap-2 Maneuver	-	-	-	-	-		299	319	-	296	320	-	
Stage 1	-	-	-	-	-	-	572	556	-	682	649	-	
Stage 2	-	-	-	-	-	-	670	647	-	550	556	-	
				10.4									
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0.4			0.2			10.8			11.8			
HCM LOS							В			В			
Minor Lane/Major Mvm	nt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	-	637	1257			1144	-	-	547				
HCM Lane V/C Ratio			0.017		_				0.034				
HCM Control Delay (s)		10.8	7.9	_		8.2	_	-	11.8				
HCM Lane LOS		В	A		-	Α	-	_	В				
HCM 95th %tile Q(veh)	}	0.1	0.1	_	_	0	-	-	0.1				
	,												

Intersection						
Int Delay, s/veh	1.9					
• .			MIST	LUDT	NO	NEC
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>↑</b>	7		र्ब	Y	
Traffic Vol, veh/h	365	36	31	256	29	59
Future Vol, veh/h	365	36	31	256	29	59
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	150	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	7	6	4	3	13	5
Mymt Flow	401	40	34	281	32	65
MAIIII LIOM	401	40	34	201	32	00
Major/Minor Ma	ajor1		Major2	1	Minor1	
Conflicting Flow All	0	0	441	0	750	401
Stage 1	-	-	-	-	401	
Stage 2	_	_	_	-	349	
Critical Hdwy	_	_	4.14		6.53	6.25
Critical Hdwy Stg 1	_	_	7.17		5.53	0.20
, ,	-	-	-			
Critical Hdwy Stg 2	-	-	0.000	-	5.53	2 245
Follow-up Hdwy	•	•	2.236	-	3.617	
Pot Cap-1 Maneuver	-	-	1108	-	364	643
Stage 1	-	-	-	-	653	-
Stage 2	•	-	•	•	690	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1108		351	643
Mov Cap-2 Maneuver	-	-	-	-	351	-
Stage 1	_	_	_		653	_
Stage 2	_	_	-		665	
Glaye Z	-	-	•	•	000	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.9		13.8	
HCM LOS	-				В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		505	-	-	1108	-
HCM Lane V/C Ratio		0.191	_		0.031	_
HCM Control Delay (s)		13.8			8.4	0
HCM Lane LOS		8	_	_	A	Ă
HCM 95th %tile Q(veh)		0.7		-	0.1	
HOM SOME ON ACH		0.7	-	-	0.1	_

	*	-	*	*	<b>—</b>	4	†	-	1	↓	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	
Lane Group Flow (vph)	43	1213	113	237	1340	72	82	137	58	23	
v/c Ratio	0.32	0.44	0.12	0.61	0.44	0.49	0.27	0.51	0.40	0.08	
Control Delay	58.7	19.4	2.4	52.2	20.7	62.2	52.1	15.0	60.2	27.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	58.7	19.4	2.4	52.2	20.7	62.2	52.1	15.0	60.2	27.6	
Queue Length 50th (ft)	32	209	0	92	188	59	32	0	44	2	
Queue Length 95th (ft)	69	302	24	131	388	109	58	59	86	16	
Internal Link Dist (ft)		601			622		348			415	
Turn Bay Length (ft)	225		150	375		250		250	200		
Base Capacity (vph)	280	2728	912	543	3031	268	540	378	191	357	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.15	0.44	0.12	0.44	0.44	0.27	0.15	0.36	0.30	0.06	
Intersection Summary											

	₫	*	<b>→</b>	*	F	1	4-	4	4	†	-	" /
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		Ä	ተተተ	7		ট্রী	ተተኈ		7	414	74	
Traffic Volume (vph)	28	12	1116	104	65	153	1206	27	133	8	126	53
Future Volume (vph)	28	12	1116	104	65	153	1206	27	133	8	126	53
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0	6.0
Lane Util. Factor		1.00	0.91	1.00		0.97	0.91		0.91	0.91	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00		1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00		0.95	0.96	1.00	0.95
Satd. Flow (prot)		1770	5085	1583		3433	5069		1610	3246	1583	1770
Flt Permitted		0.95	1.00	1.00		0.95	1.00		0.95	0.96	1.00	0.95
Satd. Flow (perm)		1770	5085	1583		3433	5069		1610	3246	1583	1770
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	30	13	1213	113	71	166	1311	29	145	9	137	58
RTOR Reduction (vph)	0	0	0	54	0	0	1	0	0	0	124	(
Lane Group Flow (vph)	0	43	1213	59	0	237	1339	0	72	82	13	58
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA		Split	NA	Perm	Spli
Protected Phases	5	5	2		1	1	6		4	4		:
Permitted Phases				2							4	
Actuated Green, G (s)		7.5	63.1	63.1		13.6	69.2		11.1	11.1	11.1	8.2
Effective Green, g (s)		7.5	63.1	63.1		13.6	69.2		11.1	11.1	11.1	8.2
Actuated g/C Ratio		0.06	0.53	0.53		0.11	0.58		0.09	0.09	0.09	0.07
Clearance Time (s)		6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0	6.0
Vehicle Extension (s)		3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0
Lane Grp Cap (vph)		110	2673	832		389	2923		148	300	146	120
v/s Ratio Prot		0.02	0.24			c0.07	c0.26		c0.04	0.03		c0.03
v/s Ratio Perm				0.04							0.01	
v/c Ratio		0.39	0.45	0.07		0.61	0.46		0.49	0.27	0.09	0.48
Uniform Delay, d1		54.1	17.7	14.0		50.7	14.6		51.7	50.7	49.8	53.9
Progression Factor		1.00	1.00	1.00		0.92	1.30		1.00	1.00	1.00	1.00
Incremental Delay, d2		2.3	0.6	0.2		2.3	0.1		2.5	0.5	0.3	3.0
Delay (s)		56.3	18.3	14.2		48.9	19.1		54.2	51.2	50.1	56.9
Level of Service		E	В	В		D	В		D	D	D	Ε
Approach Delay (s)			19.1				23.5			51.4		
Approach LOS			В				С			D		
Intersection Summary												
HCM 2000 Control Delay			24.9	Н	CM 2000	Level of	Service		С			
UCM 2000 Valuma to Cana	-14		0.50									

HCM 2000 Volume to Capacity ratio 0.50 Actuated Cycle Length (s) 120.0 Sum of lost time (s) 24.0 Intersection Capacity Utilization 62.7% ICU Level of Service В Analysis Period (min) 15

c Critical Lane Group

Movement   SBT   SBR   Lane Configurations   Traffic Volume (vph)   6   15     Itulare Volume (vph)   1900   1900     Ideal Flow (vphpl)   1900   1900     Ideal Flow (port)   1000   1900     Ideal Flow (port)   1000   1900     Ideal Flow (port)   1900   1900     Ideal Flow (port)   1900   1900     Ideal Flow (perm)   3170     Peak-hour factor, PHF   0.92   0.92     Adj. Flow (vph)   7   16     RTOR Reduction (vph)   15   0     Lane Group Flow (vph)   8   0     Turn Type   NA     Protected Phases   3     Permitted Phases   3     Permitted Phases   3     Actuated Green, G (s)   8.2     Effective Green, G (s)   8.2     Effective Green, G (s)   8.2     Actuated Gree		ţ	4	
Traffic Volume (vph) 6 15 Future Volume (vph) 6 15 Ideal Flow (vphpl) 1900 1900 Total Lost time (s) 6.0 Lane Util. Factor 0.95 Fit 7 0.90 Fit Protected 1.00 Satd. Flow (prot) 3170 Fit Permitted 1.00 Satd. Flow (prot) 3170 Peak-hour factor, PHF 0.92 0.92 Adj. Flow (pm) 7 16 RTOR Reduction (vph) 15 0 Lane Group Flow (vph) 8 0 Turm Type NA Protected Phases 3 Permitted Phases Actuated Green, G (s) 8.2 Effective Green, g (s) 8.2 Actuated g/C Ratio 0.07 Clearance Time (s) 6.0 Vehicle Extension (s) 3.0 Lane Grp Cap (vph) 216 v/s Ratio Prot 0.00 v/s Ratio Prot 0.00 v/s Ratio Prot 0.00 v/s Ratio Prot 0.04 Uniform Delay, d1 52.2 Progression Factor 1.00 Incremental Delay, d2 0.1 Delay (s) 52.3 Level of Service D Approach LoS E	Movement	SBT	SBR	
Traffic Volume (vph) 6 15 Future Volume (vph) 6 15 Ideal Flow (vphpl) 1900 1900 Total Lost time (s) 6.0 Lane Util. Factor 0.95 Fit 7 0.90 Fit Protected 1.00 Satd. Flow (prot) 3170 Fit Permitted 1.00 Satd. Flow (prot) 3170 Peak-hour factor, PHF 0.92 0.92 Adj. Flow (pm) 7 16 RTOR Reduction (vph) 15 0 Lane Group Flow (vph) 8 0 Turm Type NA Protected Phases 3 Permitted Phases Actuated Green, G (s) 8.2 Effective Green, g (s) 8.2 Actuated g/C Ratio 0.07 Clearance Time (s) 6.0 Vehicle Extension (s) 3.0 Lane Grp Cap (vph) 216 v/s Ratio Prot 0.00 v/s Ratio Prot 0.00 v/s Ratio Prot 0.00 v/s Ratio Prot 0.04 Uniform Delay, d1 52.2 Progression Factor 1.00 Incremental Delay, d2 0.1 Delay (s) 52.3 Level of Service D Approach LoS E	Lane Configurations	<b>†</b> %		
Ideal Flow (vphpl)   1900   1900   1900   Total Lost time (s)   6.0   Lane Util. Factor   0.95   Frt   0.90   Fit Protected   1.00   Satd. Flow (prot)   3170   Fit Permitted   1.00   Satd. Flow (perm)   7   16   Repair (permitted)   15   0   Canada (permitted)   15   Canada (permitted)	Traffic Volume (vph)	_	15	
Total Lost time (s) 6.0 Lane Util. Factor 0.95 Frt 0.90 Fit Protected 1.00 Satd. Flow (prot) 3170 Fit Permitted 1.00 Satd. Flow (perm) 3170 Peak-hour factor, PHF 0.92 Adj. Flow (yph) 7 16 RTOR Reduction (yph) 15 0 Lane Group Flow (yph) 8 0 Turn Type NA Protected Phases 3 Permitted Phases Actuated Green, G (s) 8.2 Effective Green, g (s) 8.2 Effective Green, g (s) 8.2 Effective Green, g (s) 6.0 Vehicle Extension (s) 3.0 Lane Grp Cap (yph) 216 v/s Ratio Prot 0.00 v/s Ratio Prot 0.00 v/s Ratio Prot 0.00 Uniform Delay, d1 52.2 Progression Factor 1.00 Incremental Delay, d2 0.1 Delay (s) 52.3 Level of Service D Approach Delay (s) 55.6 Approach LOS E	Future Volume (vph)	6	15	
Lane Util. Factor 0.95 Frt 0.90 Frt 0.90 Fit Protected 1.00 Satd. Flow (prot) 3170 Fit Permitted 1.00 Satd. Flow (perm) 3170 Peak-hour factor, PHF 0.92 0.92 Adj. Flow (yph) 7 16 RTOR Reduction (yph) 15 0 Lane Group Flow (yph) 8 0 Turn Type NA Protected Phases 3 Permitted Phases Actuated Green, G (s) 8.2 Effective Green, g (s) 8.2 Effective Green, g (s) 8.2 Actuated g/C Ratio 0.07 Clearance Time (s) 6.0 Vehicle Extension (s) 3.0 Lane Grp Cap (yph) 216 v/s Ratio Prot 0.00 v/s Ratio Prot 0.00 v/s Ratio Prot 0.04 Uniform Delay, d1 52.2 Progression Factor 1.00 Incremental Delay, d2 0.1 Delay (s) 52.3 Level of Service D Approach LOS E	Ideal Flow (vphpl)	1900	1900	
Frit Protected 1.00 Satd. Flow (prot) 3170 Fit Permitted 1.00 Satd. Flow (perm) 3170 Peak-hour factor, PHF 0.92 0.92 Adj. Flow (vph) 7 16 RTOR Reduction (vph) 15 0 Lane Group Flow (vph) 8 0 Turn Type NA Protected Phases 3 Permitted Phases Actuated Green, G (s) 8.2 Effective Green, g (s) 8.2 Actuated g/C Ratio 0.07 Clearance Time (s) 6.0 Vehicle Extension (s) 3.0 Lane Gro Zap (vph) 216 v/s Ratio Prot 0.00 v/s Ratio Perm v/c Ratio 0.04 Uniform Delay, d1 52.2 Progression Factor 1.00 Incremental Delay, d2 0.1 Delay (s) 52.3 Level of Service D Approach LOS E	Total Lost time (s)	6.0		
Fit Protected 1.00 Satd. Flow (prot) 3170 Fit Permitted 1.00 Satd. Flow (perm) 3170 Peak-hour factor, PHF 0.92 0.92 Adj. Flow (vph) 7 16 RTOR Reduction (vph) 15 0 Lane Group Flow (vph) 8 0 Turn Type NA Protected Phases 3 Permitted Phases Actuated Green, G (s) 8.2 Effective Green, g (s) 8.2 Actuated g/C Ratio 0.07 Clearance Time (s) 6.0 Vehicle Extension (s) 3.0 Lane Grp Cap (vph) 216 v/s Ratio Prot 0.00 v/s Ratio Port 0.00 v/s Ratio Perm v/c Ratio 0.04 Uniform Delay, d1 52.2 Progression Factor 1.00 Incremental Delay, d2 0.1 Delay (s) 52.3 Level of Service D Approach Delay (s) 55.6 Approach LOS E	Lane Util. Factor	0.95		
Satd. Flow (prot)       3170         Flt Permitted       1.00         Satd. Flow (perm)       3170         Peak-hour factor, PHF       0.92         Adj. Flow (vph)       7       16         RTOR Reduction (vph)       15       0         Lane Group Flow (vph)       8       0         Turn Type       NA         Protected Phases       3         Actuated Green, G (s)       8.2         Effective Green, g (s)       8.2         Actuated g/C Ratio       0.07         Clearance Time (s)       6.0         Vehicle Extension (s)       3.0         Lane Grp Cap (vph)       216         v/s Ratio Prot       0.00         v/s Ratio Perm       v/c Ratio         Uniform Delay, d1       52.2         Progression Factor       1.00         Incremental Delay, d2       0.1         Delay (s)       52.3         Level of Service       D         Approach Delay (s)       55.6         Approach LOS       E	Frt	0.90		
Fit Permitted       1.00         Satd. Flow (perm)       3170         Peak-hour factor, PHF       0.92       0.92         Adj. Flow (vph)       7       16         RTOR Reduction (vph)       15       0         Lane Group Flow (vph)       8       0         Turn Type       NA         Protected Phases       3         Permitted Phases       Actuated Green, G (s)       8.2         Effective Green, g (s)       8.2         Actuated g/C Ratio       0.07         Clearance Time (s)       6.0         Vehicle Extension (s)       3.0         Lane Grp Cap (vph)       216         v/s Ratio Prot       0.00         v/s Ratio Perm       v/c Ratio         v/c Ratio       0.04         Uniform Delay, d1       52.2         Progression Factor       1.00         Incremental Delay, d2       0.1         Delay (s)       52.3         Level of Service       D         Approach Delay (s)       55.6         Approach LOS       E	Flt Protected	1.00		
Fit Permitted         1.00           Satd. Flow (perm)         3170           Peak-hour factor, PHF         0.92         0.92           Adj. Flow (vph)         7         16           RTOR Reduction (vph)         15         0           Lane Group Flow (vph)         8         0           Turn Type         NA           Protected Phases         3           Actuated Green, G (s)         8.2           Effective Green, g (s)         8.2           Actuated g/C Ratio         0.07           Clearance Time (s)         6.0           Vehicle Extension (s)         3.0           Lane Grp Cap (vph)         216           v/s Ratio Prot         0.00           v/s Ratio Perm         v/c Ratio           Uniform Delay, d1         52.2           Progression Factor         1.00           Incremental Delay, d2         0.1           Delay (s)         52.3           Level of Service         D           Approach Delay (s)         55.6           Approach LOS         E	Satd. Flow (prot)	3170		
Peak-hour factor, PHF         0.92         0.92           Adj. Flow (vph)         7         16           RTOR Reduction (vph)         15         0           Lane Group Flow (vph)         8         0           Turn Type         NA         NA           Protected Phases         3         Permitted Phases           Actuated Green, G (s)         8.2         Effective Green, g (s)         8.2           Effective Green, g (s)         8.2         Actuated g/C Ratio         0.07           Clearance Time (s)         6.0         Vehicle Extension (s)         3.0           Lane Grp Cap (vph)         216         v/s Ratio Prot         0.00           v/s Ratio Prot         0.00         v/s Ratio Prot         0.00           v/s Ratio Perm         v/c Ratio         0.04         Uniform Delay, d1         52.2           Progression Factor         1.00         Incremental Delay, d2         0.1         Delay (s)         52.3           Level of Service         D         Approach Delay (s)         55.6         Approach Delay (s)         55.6		1.00		
Peak-hour factor, PHF         0.92         0.92           Adj. Flow (vph)         7         16           RTOR Reduction (vph)         15         0           Lane Group Flow (vph)         8         0           Turn Type         NA         NA           Protected Phases         3         Permitted Phases           Actuated Green, G (s)         8.2         Effective Green, g (s)         8.2           Effective Green, g (s)         8.2         Actuated g/C Ratio         0.07           Clearance Time (s)         6.0         Vehicle Extension (s)         3.0           Lane Grp Cap (vph)         216         v/s Ratio Prot         0.00           v/s Ratio Prot         0.00         v/s Ratio Prot         0.00           v/s Ratio Perm         v/c Ratio         0.04         Uniform Delay, d1         52.2           Progression Factor         1.00         Incremental Delay, d2         0.1           Delay (s)         52.3         Level of Service         D           Approach Delay (s)         55.6         Approach LOS         E	Satd. Flow (perm)			
Adj. Flow (vph)       7       16         RTOR Reduction (vph)       15       0         Lane Group Flow (vph)       8       0         Turn Type       NA         Protected Phases       3         Permitted Phases       Actuated Green, G (s)       8.2         Effective Green, g (s)       8.2         Actuated g/C Ratio       0.07         Clearance Time (s)       6.0         Vehicle Extension (s)       3.0         Lane Grp Cap (vph)       216         v/s Ratio Prot       0.00         v/s Ratio Perm       v/c Ratio         V/c Ratio       0.04         Uniform Delay, d1       52.2         Progression Factor       1.00         Incremental Delay, d2       0.1         Delay (s)       52.3         Level of Service       D         Approach Delay (s)       55.6         Approach LOS       E	Peak-hour factor, PHF	0.92	0.92	
RTOR Reduction (vph)         15         0           Lane Group Flow (vph)         8         0           Turn Type         NA           Protected Phases         3           Actuated Phases         3           Actuated Green, G (s)         8.2           Effective Green, g (s)         8.2           Actuated g/C Ratio         0.07           Clearance Time (s)         6.0           Vehicle Extension (s)         3.0           Lane Grp Cap (vph)         216           v/s Ratio Prot         0.00           v/s Ratio Perm         v/c Ratio           v/c Ratio         0.04           Uniform Delay, d1         52.2           Progression Factor         1.00           Incremental Delay, d2         0.1           Delay (s)         52.3           Level of Service         D           Approach Delay (s)         55.6           Approach LOS         E	•			
Lane Group Flow (vph)         8         0           Turn Type         NA           Protected Phases         3           Actuated Phases         8.2           Actuated Green, G (s)         8.2           Actuated g/C Ratio         0.07           Clearance Time (s)         6.0           Vehicle Extension (s)         3.0           Lane Grp Cap (vph)         216           v/s Ratio Prot         0.00           v/s Ratio Perm         v/c Ratio           v/c Ratio         0.04           Uniform Delay, d1         52.2           Progression Factor         1.00           Incremental Delay, d2         0.1           Delay (s)         52.3           Level of Service         D           Approach Delay (s)         55.6           Approach LOS         E				
Turn Type				
Protected Phases       3         Actuated Green, G (s)       8.2         Effective Green, g (s)       8.2         Actuated g/C Ratio       0.07         Clearance Time (s)       6.0         Vehicle Extension (s)       3.0         Lane Grp Cap (vph)       216         v/s Ratio Prot       0.00         v/s Ratio Perm       v/c Ratio         Uniform Delay, d1       52.2         Progression Factor       1.00         Incremental Delay, d2       0.1         Delay (s)       52.3         Level of Service       D         Approach Delay (s)       55.6         Approach LOS       E		NA		
Permitted Phases       Actuated Green, G (s)       8.2         Effective Green, g (s)       8.2         Actuated g/C Ratio       0.07         Clearance Time (s)       6.0         Vehicle Extension (s)       3.0         Lane Grp Cap (vph)       216         v/s Ratio Prot       0.00         v/s Ratio Perm       v/c Ratio         Uniform Delay, d1       52.2         Progression Factor       1.00         Incremental Delay, d2       0.1         Delay (s)       52.3         Level of Service       D         Approach Delay (s)       55.6         Approach LOS       E		3		
Effective Green, g (s) 8.2  Actuated g/C Ratio 0.07  Clearance Time (s) 6.0  Vehicle Extension (s) 3.0  Lane Grp Cap (vph) 216  v/s Ratio Prot 0.00  v/s Ratio Perm  v/c Ratio 0.04  Uniform Delay, d1 52.2  Progression Factor 1.00  Incremental Delay, d2 0.1  Delay (s) 52.3  Level of Service D  Approach Delay (s) 55.6  Approach LOS E				
Effective Green, g (s) 8.2  Actuated g/C Ratio 0.07  Clearance Time (s) 6.0  Vehicle Extension (s) 3.0  Lane Grp Cap (vph) 216  v/s Ratio Prot 0.00  v/s Ratio Perm  v/c Ratio 0.04  Uniform Delay, d1 52.2  Progression Factor 1.00  Incremental Delay, d2 0.1  Delay (s) 52.3  Level of Service D  Approach Delay (s) 55.6  Approach LOS E	Actuated Green, G (s)	8.2		
Clearance Time (s)       6.0         Vehicle Extension (s)       3.0         Lane Grp Cap (vph)       216         v/s Ratio Prot       0.00         v/s Ratio Perm       0.04         Uniform Delay, d1       52.2         Progression Factor       1.00         Incremental Delay, d2       0.1         Delay (s)       52.3         Level of Service       D         Approach Delay (s)       55.6         Approach LOS       E	Effective Green, g (s)	8.2		
Vehicle Extension (s)         3.0           Lane Grp Cap (vph)         216           v/s Ratio Prot         0.00           v/s Ratio Perm         v/c Ratio           Uniform Delay, d1         52.2           Progression Factor         1.00           Incremental Delay, d2         0.1           Delay (s)         52.3           Level of Service         D           Approach Delay (s)         55.6           Approach LOS         E	Actuated g/C Ratio	0.07		
Lane Grp Cap (vph)       216         v/s Ratio Prot       0.00         v/s Ratio Perm       0.04         Uniform Delay, d1       52.2         Progression Factor       1.00         Incremental Delay, d2       0.1         Delay (s)       52.3         Level of Service       D         Approach Delay (s)       55.6         Approach LOS       E	Clearance Time (s)	6.0		
Lane Grp Cap (vph)       216         v/s Ratio Prot       0.00         v/s Ratio Perm       0.04         Uniform Delay, d1       52.2         Progression Factor       1.00         Incremental Delay, d2       0.1         Delay (s)       52.3         Level of Service       D         Approach Delay (s)       55.6         Approach LOS       E		3.0		
v/s Ratio Prot0.00v/s Ratio Perm0.04V/c Ratio0.04Uniform Delay, d152.2Progression Factor1.00Incremental Delay, d20.1Delay (s)52.3Level of ServiceDApproach Delay (s)55.6Approach LOSE		216		
v/s Ratio Perm v/c Ratio 0.04 Uniform Delay, d1 52.2 Progression Factor 1.00 Incremental Delay, d2 0.1 Delay (s) 52.3 Level of Service D Approach Delay (s) 55.6 Approach LOS E				
Uniform Delay, d1       52.2         Progression Factor       1.00         Incremental Delay, d2       0.1         Delay (s)       52.3         Level of Service       D         Approach Delay (s)       55.6         Approach LOS       E	v/s Ratio Perm			
Uniform Delay, d1       52.2         Progression Factor       1.00         Incremental Delay, d2       0.1         Delay (s)       52.3         Level of Service       D         Approach Delay (s)       55.6         Approach LOS       E	v/c Ratio	0.04		
Progression Factor 1.00 Incremental Delay, d2 0.1 Delay (s) 52.3 Level of Service D Approach Delay (s) 55.6 Approach LOS E				
Incremental Delay, d2         0.1           Delay (s)         52.3           Level of Service         D           Approach Delay (s)         55.6           Approach LOS         E	•			
Delay (s) 52.3 Level of Service D Approach Delay (s) 55.6 Approach LOS E	•			
Level of Service D Approach Delay (s) 55.6 Approach LOS E		52.3		
Approach Delay (s) 55.6 Approach LOS E				
Approach LOS E				
THE DECIDING V	Intersection Summary			

Intersection: 1: I-95 SB On-Ramp & Route 54

Movement	EB	EB	WB	WB	WB	B33	B33	B33	B35	B35	B35	SB
Directions Served	Т	T	L	Т	T	Т	T	Ţ	Т	Т	Т	LT
Maximum Queue (ft)	196	168	188	137	152	205	19	14	41	25	24	137
Average Queue (ft)	122	71	155	57	66	52	0	1	2	1	1	66
95th Queue (ft)	209	172	200	118	135	152	4	9	21	16	18	121
Link Distance (ft)	155	155	98	98	98	206	206	206	151	151	151	103
Upstream Blk Time (%)	9	2	42	1	2	1				0		2
Queuing Penalty (veh)	31	7	207	6	11	3				0		0
Storage Bay Dist (ft)												

Storage Blk Time (%)
Queuing Penalty (veh)

Intersection: 1: I-95 SB On-Ramp & Route 54

Intersection: 2: I-95 NB On-Ramp & Route 54

Movement	EB	EB	EB	WB	WB
Directions Served	L	Т	Т	Т	T
Maximum Queue (ft)	175	221	115	16	70
Average Queue (ft)	106	88	37	1	16
95th Queue (ft)	177	175	81	9	46
Link Distance (ft)		179	179	146	146
Upstream Blk Time (%)	1	1	0		
Queuing Penalty (veh)	0	4	0		
Storage Bay Dist (ft)	300				
Storage Blk Time (%)	1	1			
Queuing Penalty (veh)	2	3			

# Intersection: 3: Route 54 & Woodside Lane

Movement	EB	WB	SB	SB
Directions Served	L	TR	L	R
Maximum Queue (ft)	89	4	46	68
Average Queue (ft)	30	0	14	33
95th Queue (ft)	67	3	34	55
Link Distance (ft)		529	891	
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	225			200
Storage Blk Time (%)				
Queuing Penalty (veh)				

# Intersection: 4: Route 54 & Providence Church Road

Movement	EB	SB
Directions Served	L	LR
Maximum Queue (ft)	24	28
Average Queue (ft)	2	8
95th Queue (ft)	14	28
Link Distance (ft)		1008
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	200	
Storage Blk Time (%)		
Queuing Penalty (veh)		

# Intersection: 5: Route 54 & Site Driveway

Movement	EB	WB	NB	SB
Directions Served	L	L	LTR	LTR
Maximum Queue (ft)	29	26	29	33
Average Queue (ft)	3	2	12	13
95th Queue (ft)	16	14	35	38
Link Distance (ft)			408	540
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	200	200		
Storage Blk Time (%)				
Queuing Penalty (veh)				

# Intersection: 6: Goddins Hill Road & Route 54

		14.15	110
Movement	EB	WB	NB
Directions Served	R	LT	LR
Maximum Queue (ft)	7	56	96
Average Queue (ft)	0	10	40
95th Queue (ft)	5	40	73
Link Distance (ft)		1778	529
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)	150		
Storage Blk Time (%)			
Queuing Penalty (veh)			

# Intersection: 22: Route 54

Movement	EB	EB	B35	B33	WB	SW	B24	B25
Directions Served	Т	Т	Т	Ţ	Т	R	Т	Т
Maximum Queue (ft)	131	30	11	10	2	267	291	16
Average Queue (ft)	19	1	1	0	0	150	44	1
95th Queue (ft)	90	16	12	6	2	272	270	17
Link Distance (ft)	151	151	206	98	179	166	852	358
Upstream Blk Time (%)	0					17	0	
Queuing Penalty (veh)	1					0	0	
Storage Bay Dist (ft)								
Storage Blk Time (%)								
Queuing Penalty (veh)								

# Intersection: 31: Route 54

Movement	EB	EB	E8	WB	WB	WB
Directions Served	T	Т	R	T	Т	Ŧ
Maximum Queue (ft)	305	267	306	45	49	57
Average Queue (ft)	71	35	123	18	15	4
95th Queue (ft)	229	158	254	41	45	28
Link Distance (ft)	610	610	610	155	155	155
Upstream Blk Time (%)						
Queuing Penalty (veh)						
Storage Bay Dist (ft)						
Storage Blk Time (%)						
Queuing Penalty (veh)						

Intersection: 32: I-95 SB On-Ramp

Movement	SE	
Directions Served	L	
Maximum Queue (ft)	228	
Average Queue (ft)	128	
95th Queue (ft)	247	
Link Distance (ft)	215	
Upstream Blk Time (%)	3	
Queuing Penalty (veh)	10	
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 34: Hill Carter Parkway & England Street/Route 54

Movement	EB	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB
Directions Served	ŲŁ	Т	Т	T	R	UL	L	Т	T	TR	L	LT
Maximum Queue (ft)	94	301	294	344	250	157	146	222	227	236	144	190
Average Queue (ft)	33	176	135	183	50	77	78	107	116	129	14	91
95th Queue (ft)	73	281	269	295	164	135	124	180	191	205	79	156
Link Distance (ft)		632	632	632				610	610	610		352
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	225				150	375	375				250	
Storage Blk Time (%)		3		14								0
Queuing Penalty (veh)		1		15								0

Intersection: 34: Hill Carter Parkway & England Street/Route 54

Movement	NB	NB	SB	SB	SB
Directions Served	Т	R	L	T	TR
Maximum Queue (ft)	49	110	116	33	34
Average Queue (ft)	2	41	46	6	13
95th Queue (ft)	30	86	96	24	36
Link Distance (ft)	352			429	429
Upstream Blk Time (%)					
Queuing Penalty (veh)					
Storage Bay Dist (ft)		250	200		
Storage Blk Time (%)					
Queuing Penalty (veh)					

# **Network Summary**

Network wide Queuing Penalty: 302

# **APPENDIX F: VDOT TURN LANE WARRANT ANALYSIS**



General Project	Information			Enter a valu	e for all inpu	t cells		
Project Name:	Hickory Grov	e - Rte 54 at	Realigned Prov	vidence Chur	ch Rd - Build	2027 AM		
County:	Hanover Cou	nty						
Reviewer:	GS				Date:	5/17/2023		
Adjacent Roads	way Data							
Adjacent Road I		Rte 54				Number of L	anes: 2	
Posted Speed:		55	mph *	С	lassification:	Minor Rural	Arterial	
AADT:	N/A	VPD D:	N/A *Use Design S	k: peed if availa		enter N/A if j	factors are un	known
Trip Generation	1			115				
Generated Trips	s:	836	VPD	% Trucks in I	Entrance:	2	%	
Right In:	0	VPH		Advancing V	olume:	235	VPH	
Left In:	7	VPH	3%	Opposing Vo			VPH**	
(0)	() L			**Also used	as Approach	ing Volume fo	or Rt. Turns	
Entrance Criter	ia		Entrance is a	Standard Cor	nmercial Ent	rance		
Entrance Type:		Full Access E	ntrance					
Minimum Spaci	ng:	555	ft	SDL:	610	ft SDR:	610	ft
Left Turn Lane \	Warrant: Adva	ancing Volum	ne ≥	423	VPH	No Left Turn	Lane	
Right Turn Tape	er Warrant: Rt	. Turn Volum	ne ≥	35	VPH	No Taper Re	quired	
Right Turn Lane	Warrant: Rt.	Turn Volume	2 ≥	73	VPH	No Right Tur	n Lane Requi	red
Wa	errant for Left- 2-Lane	Turn Storage Highway	Lane	S 100			r Right Turn T Lane Highway	
長 700				S=100	120	1 +		
> 600				——S=150	T 400			Taper Turn Lane
000 sing Volume, 400 solume, 4	8			——S=175	s, 80 80			
9 300 = 5 9 200 = 5	N N			S=200	40			
100	VAR.			S=250	20 Es 20	1		
d 0 1	200 400	600 80	0 1000 12			0 200	400	600 800
		ing Volume,					proach Total,	
1. The minimur								nph
2. Left turn land								3.0
W 8	S = 100'	eft Turn Stor	rage Length Inc	rease Requir	s = 200'	S = 250'	s = 300'	- 11 11, 4.1
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General Projec	t Information			Enter a value	for all inp	ut cells		
Project Name:	Hickory Grov	e - Rte 54 at	Realigned Prov	vidence Churc	h Rd - Buik	d 2027 PM		
County:	Hanover Cou	inty						
Reviewer:	GS				Date:	5/17/2023		
Adjacent Road	way Data	<i>P</i> 1 4 24						
Adjacent Road	Name:	Rte 54				Number of L	anes: 2	
Posted Speed:		55	mph *	CI	assification	: Minor Rural	Arterial	
AADT:	N/A	VPD D:	N/A *Use Design S	k:	N/A ible	enter N/A if	factors are ur	nknown
Trip Generatio	n W. W. Law							
Generated Trip	s:	836	VPD	% Trucks in E	ntrance:	2	%	
Right In:	2	VPH		Advancing V	olume:	395	VPH	
Left In:	12	VPH	3%	Opposing Vo		277 hing Volume f	VPH** or Rt. Turns	
Entrance Criter	ia		Entrance is a	Standard Con	nmercial En	trance		
Entrance Type:		Unsignalized	d Intersection/	Crossover				
Minimum Spac	ing:	1050	ft	SDL:	610	_ft SDR:	610	_ft
Left Turn Lane	Warrant: Adv	ancing Volun	ne ≥	460	VPH	No Left Turn	Lane	
Right Turn Tape	er Warrant: Ri	t. Turn Volun	ne ≥	42	VPH	No Taper Re	quired	
Right Turn Lane	Warrant: Rt.	Turn Volum	e≥	83	VPH	No Right Tu	rn Lane Requi	ired
W	arrant for Left 2-Lane	-Turn Storage Highway	e Lane	S=100			r Right Turn T Lane Highway	
₹ 800				——S=125	120			Taper
Opposing Volume, VPH  000 000 000 000 000 000 000 000 000				S=150	1 0 On	7		—Turn Lane
400	<u>a</u>			—S=175	. sr. 60			
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100	WAR			S=300	म्हा 20 मुख्य 20	-		
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		ing Volume,					proach Total	
1. The minimu	m warranted l	eft turn lane	length shall be	100' for spec	eds ≤ 40 mp	oh and 200' for	speeds ≥ 45	mph
2. Left turn lan								1
		Left Turn Sto	rage Length Inc	crease Requir	ed for Truc	k Ratio (in Fee	t)	
	S = 100'	S = 125'	S = 150'	S = 175'	S = 200'	S = 250'	S = 300'	8 110

Version 2.1 - Based on July 2012 Appendix F - VDOT Road Design Manual 8-1-12



General Project Information Enter a value for all input cells								
Project Name: Hickory Grove - Rte 54 at Site Driveway - Build 2027 AM								
County:	Hanover Cou	nty	_					
Reviewer:	GS			. +	Date:	5/17/2023		
Adjacent Roads	way Data	a contiguosis		W.Y.				
Adjacent Road Name: Rte 54						Number of L	anes: 2	
Posted Speed: 55 mph * Classification: Minor Rural Arterial								
AADT:	N/A	VPD D:	N/A *Use Design S	k: peed if availa	N/A able	enter N/A if	factors are un	known
Trip Generation					- 71			
Generated Trips	s:	836	VPD	% Trucks in E	Entrance:	2	%	
Right In:	3 VPH Advancing Volume: 229 VPH							
Left In:	9	VPH	4%	Opposing Vo		334 ling Volume fo	VPH** or Rt. Turns	
Entrance Criter	ia		Entrance is a	Standard Con	nmercial Ent	rance		
Entrance Type:		Full Access E	intrance					
Minimum Spacing: 555 ft SDL: 610 ft SDR: 610 ft								
Left Turn Lane \	Warrant: Adva	ncing Volum	ie ≥	431	VPH	No Left Turn	Lane	
Right Turn Tape	er Warrant: Rt	. Turn Volum	ne ≥	37	VPH	No Taper Re	quired	
Right Turn Lane Warrant: Rt. Turn Volume ≥ 76 VPH No Right Turn Lane Required								
Warrant for Left-Turn Storage Lane  2-Lane Highway  Warrants for Right Turn Treatments 2-Lane Highway								
₹ 800 700				S=100	1 2 1 33	1		These
> 600				S=150				Taper Turn Lane
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Advancing Volume, VPH Approach Total, VPH								
<ol> <li>The minimum warranted left turn lane length shall be 100' for speeds ≤ 40 mph and 200' for speeds ≥ 45 mph</li> <li>Left turn lanes with high truck volume shall be increased as calculated and tabulated below:</li> </ol>						npn		
			age Length Inc					
	S = 100'	S = 125'	S = 150'	S = 175'	S = 200°	S = 250'	S = 300'	
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General Project Information Enter a value for all input cells								
Project Name: Hickory Grove - Rte 54 at Site Driveway - Build 2027 PM								
County:	Hanover Cou	nty						
Reviewer:	GS				Date:	5/17/2023		
Adjacent Roads	way Data	JAN 18			w <sub>m</sub> F			
Adjacent Road	Name:	Rte 54				Number of I	anes: 2	
Posted Speed:		55	mph *	C	lassification:	Minor Rural	Arterial	
AADT:	N/A	VPD D:	N/A *Use Design S	k: speed if avails		enter N/A if	factors are un	known
Trip Generation	1			, -111-	Will live			
Generated Trips	s:	836	VPD	% Trucks in I	intrance:	2	%	
Right In:	6	VPH		Advancing V	olume:	384	VPH	
Left In:	20	VPH	5%	Opposing Vo		270	VPH**	
				**Also used	as Approach	ning Volume f	or Rt. Turns	
Entrance Criter	ia		Entrance is a	Standard Cor	nmercial Ent	rance		
Entrance Type:	Entrance Type: Full Access Entrance							
Minimum Spacing: 555 ft SDL: 610 ft SDR: 610 ft								
Left Turn Lane Warrant: Advancing Volume ≥458 VPH								
Right Turn Taper Warrant: Rt. Turn Volume ≥ 43 VPH <b>No Taper Required</b>								
Right Turn Lane Warrant: Rt. Turn Volume ≥ 84 VPH <b>No Right Turn Lane Required</b>								
Wa	arrant for Left- 2-Lane	Turn Storage Highway	e Lane	S 100			r Right Turn Ti Lane Highway	
₹ 800				S=100		+		Theor
> 600				S=150				Taper Turn Lane
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Advancing Volume, VPH Approach Total, VPH								
<ol> <li>The minimum warranted left turn lane length shall be 100' for speeds ≤ 40 mph and 200' for speeds ≥ 45 mph</li> <li>Left turn lanes with high truck volume shall be increased as calculated and tabulated below:</li> </ol>						nph		
E CIT LUITIBLE			rage Length Inc					
	S = 100'	S = 125'	S = 150'	S = 175'	S = 2001	S = 250'	S = 300'	
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Version 2.1 -Based on July 2012 Appendix F - VDOT Road Design Manual 8-1-12

Historical Comn	nission Recomi	mendation	



Post Office Box 470, Hanover, Virginia 23069

#### **MEMORANDUM**

TO:

Gretchen Biernot, Current Planning Manager

FROM:

Claudia Cheely, Senior Planner II/

**SUBJECT:** 

Historical Commission Recommendation

REZ2023-00003, Hickory Hill II, L.L.C.

DATE:

March 9, 2023

At the Historical Commission meeting on March 7, 2023, the Commission reviewed the request to rezone from A-1, Agricultural District, to RS(c), Single-Family Residential District with conditions, for the purpose of developing 155 single family age restricted homes (76 carriage homes and 79 detached homes). The Commission reviewed this request because the property is adjacent and near two surveyed cemeteries and the Civil War feature, Jackson's March from Beaverdam to Mechanicsville, passes by this property along Providence Church Road. Both cemeteries are far enough away that they will not be impacted by the development of the subject property and there are no physical features in this area related to the Civil War feature. The Commission determined this request will not impact the historic resources.

/cdc

**Agency Review Comments** 

### Cheely, Claudia

From:

Watkins, Tyrone O.

Sent:

Thursday, July 6, 2023 9:48 AM

To: Cc: Cheely, Claudia

Biernot, Gretchen W.

Subject:

RE: Hickory Hill II LLC REZ2023-00003 (1).pdf

#### Hi Claudia.

Dan is correct, this comment may be addressed during the construction plan design with site grading. My comment is driven by the existing topography of the land. If RS zoning requires public utility service then Bay will need to design the development accordingly. Coordinating the grading with the lot layout can lead to an acceptable design for the RS zoning. It is also possible that area outside of the natural or developed sanitary sewer drainage shed could be utilized as part of their amenities or open space that do not require sewer service, again this is all part of Bay's overall design in developing a cohesive construction plan. In general, I am confident that they can design an approvable construction plan layout that can be served by gravity public sanitary sewer. If you need additional clarification on this comment to send to the Board let me know. Thanks

#### Tyrone O. Watkins, P.E.

Engineering Manager Hanover County Public Utilities 7516 County Complex Road Hanover, Virginia 23069

Office: (804) 365-6812

Email: towatkins@hanovercounty.gov

From: Cheely, Claudia <cdcheely@hanovercounty.gov>

Sent: Wednesday, July 5, 2023 5:30 PM

**To:** Watkins, Tyrone O. <TOWatkins@hanovercounty.gov> **Cc:** Biernot, Gretchen W. <gwbiernot@hanovercounty.gov>

Subject: Hickory Hill II LLC REZ2023-00003 (1).pdf

#### Tyrone,

Attached are your comments on this request. At Planning Commission, Comment #2 became a talking point, which the applicant's engineer, Dan Caskie, indicated was issue would that would be addressed with the grading of the property. Can you respond to this or try to talk with Dan about it and then provide me some sort of written comment that indicates whether you're confident or not that they can do this? It's fairly important since the RS District requires all lots to be served by public utilities. We will be sending Board reports next week and would like to include your response.

Thank you! Claudia

## **ZONING STAFF MEETING**

## **Department of Public Utilities Comments**

CASE NAME: Hickory Hills II, LLC

PROJECT #: REZ2023-00003

#### PLEASE MARK THE APPROPRIATE BOX BELOW:

No Comment:		
Comments Below	v: 🗹	
DATE: 2/15/2023		REVIEWED BY: T. Watkins

The Hanover County Department of Public Utilities (DPU) has reviewed the above referenced rezoning request and has the following comments:

- 1. All water and sanitary sewer extensions will need to be completed in accordance with DPU requirements.
- 2. Portions of the property may not be able to be served by the public sewer system.
- 3. Water and sanitary sewer extensions and/or easements will need to be extended to serve nearby properties.
- 4. After the zoning process, but prior to initiating site design or construction drawings, the developer should meet with the Fire Marshal's Office to discuss the proposed layout and determine the location of the fire hydrants. DPU should be consulted either jointly or following the meeting with the Fire Marshal's Office to discuss the layout of the waterlines that will supply the hydrants.
- 5. Two independent waterline connections will be required with approval of more than 50 lots within this development.
- 6. Depending on the proposed phasing of the development a Utility Master Plan must be approved for the entire development prior to DPU recommending approval of any construction plans.

Please feel free to contact me if you have any questions or concerns.



# ANOVER COUNTS Office of the Fire Marshal



Office (804) 365-6195 Fax (804) 537-5488

13326 Hanover Courthouse Road P.O. Box 470 Hanover, VA 23069 fmo@hanovercounty.gov Dispatch (804) 365-6140

# **PLANS REVIEW**

JURISDICTION:	Hanover				
FMO FILE #	REZ2023-00003				
то:	Claudia Cheely				
FROM:	Doug Atkins, Plan Examiner Office 804-365-4858 Email: jdatkins@hanovercounty.gov				
SUBJECT:	Hickory Hill II LLC				
DATE:	2-15-2023				
TYPE OF REVIEW	REZ 1 <sup>st</sup> Review				
COMMENTS:	<ol> <li>The applicant should be aware of the requirements of the Hanover Fire Marshal's office.         The requirements of the Hanover Fire Marshal's Office can be found online at <a href="https://www.hanovercounty.gov/1082/Plans-Review">https://www.hanovercounty.gov/1082/Plans-Review</a> </li> <li>Apparatus access roads with hydrants or buildings greater than 30 feet in height shall be 26 feet wide. The measurement is from the edge of the pavement to the edge of the pavement. Hanover Fire Marshal's Office will not accept the measurement from the curb and gutter as part of the apparatus access road.</li> <li>All turnarounds shall meet the requirements of the 2018 Virginia Statewide Fire Prevention Code Appendix D.</li> </ol>				
TrackIt:	2-15-2023				

# **REZONING**

# **Department of Public Works Comments**

**CASE NAME: Hickory Hill II LLC** 

TRACKIT PROJECT #: REZ2023-00003

PLEASE MARK THE APPROPRIATE BOX BELOW:

No Comment

|X|

Comments Below

DATE: 02/14/2023

REVIEWED BY: MJD

No comments on rezoning.

- Additional development subject to runoff reduction and channel protection. 1)
- Any wetlands and RPA areas must meet regulations. WQIA required in support of these 2) studies.

#### 2/22/2023 10:33:08 AM (Planning Code Compliance - Jason Hazelwood)

Elevations need to show brick and stone.

#### 2/16/2023 2:19:01 PM (Health Department - David N Wachsmann)

If property is to use private water and sewer disposal developer must contact and AOSE/PE to evaluate for the onsite sewage disposal and private water supply. If public water and sewer no comment from Onsite EHS Health.

#### 2/16/2023 3:56:19 PM (Planning Development Review - Donald Stewart)

- Pedestrian path is shown on the Northern side of Providence Road, but it also needs to be shown on the Southern side of the road within the 25' buffer.
- While I understand other communities may have had the parking for boats/RVs/camper etc. I don't think it is appropriate for this neighborhood.
- If it stays, I believe it should be relocated to away from Providence and not immediately within the side yard of two residences. Also I think it should be screened with a fence and slats along with a single row of evergreens.
- \*\*Also the plans show a fence going all the way around that storage lot... where's the fence detail? How tall will it be? Will it be locked? Does the Fire Department need a knox box for access in case of an emergency? What about lighting? If it's locked, how will residents have access to their vehicles?



# COMMONWEALTH of VIRGINIA

#### **DEPARTMENT OF TRANSPORTATION**

RICHMOND DISTRICT 2430 Pine Forest Drive COLONIAL HEIGHTS, VA 23834 www.VDOT.Virginia.gov

March 22, 2023

County of Hanover
Department of Planning and Zoning
P.O. Box 470
Hanover, VA 23069
Attn.: Claudia Cheely

Stephen C. Brich, P.E.

COMMISSIONER

Re: REZ2023-00003 Hickory Grove

Ms. Cheely,

The Department of Transportation, Ashland Residency Transportation and Land Use Section has reviewed the above referenced plans as submitted by the Bay Companies, dated January 19, 2023 and the Traffic Impact Analysis as submitted by Gorove Slade, dated January 2023, and offer the following comments:

- 1. All public roads shall be designed and constructed in accordance with the SSAR Regulations.
- The realignment of existing Providence Church Road and associated right of way will be subject to the policies and procedures set forth in VDOT's Guide to Highway System
   Changes.
- 3. What is the purpose of the proposed street segment and cul-de-sac within the existing Providence Church Road right of way? It does not appear to be necessary for property access.
- 4. Acceptable turn-arounds meeting the SSAR regulations and Appendix B(1) of the VDOT Road Design Manual shall be provided on any future road plans.
- 5. The proposed realignment of Providence Church Road must meet standards appropriate for the current classification, speed, and traffic volume.
- All connections to existing public roads must provide intersection sight distance.
- 7. The proposed road section of 30-ft. measured from the face of curb to face of curb is wider than the minimum for streets providing parking on both sides. Please consider road designs that feature passive traffic calming elements (widths no larger than minimums and utilization of parking lanes with bump-outs).

REZ2023-00003 March 22, 2023 Page 2 of 2

If further information is desired, please contact me at 804-585-3585.

A VDOT Land Use Permit will be required prior to any work within the right of way. The owner/developer must contact the Ashland Residency Transportation and Land Use Section at 804-585-3592 for information pertaining to this process.

Sincerely,

Adam J. Moore P.E. Area Land Use Engineer Ashland Residency **Community Meeting Notes** 

# Community Meeting Notes: REZ2023-00003, Hickory Hill II, L.L.C.

A community meeting on REZ2023-00003 was held by the applicant on April 19, 2023, at 6:00 p.m. at Bass Pro Shops. The project is located on the north line of East Patrick Henry Road at its intersection with Providence Church Road. The applicant is proposing to rezone from A-1, Agricultural District, to RS(c), Single-Family Residential District with conditions. The proposed zoning amendment would allow the development of 155 age-restricted single-family homes (76 carriage homes and 79 detached homes) for a gross density of 2.95 dwelling units per acre.

Approximately 25 people attended the meeting. Representatives of the Board of Supervisors who were present included Rob Monolo and Faye Prichard. Edmonia Iverson and Larry Leadbetter were present to represent the Planning Commission. Staff included Todd Kilduff, Jo Ann Hunter, Gretchen Biernot, and Claudia Cheely.

The applicant introduced the proposed development with basic information describing the features and plans. The floor was then opened for citizens to ask questions and express their concerns. The following is a summary of those questions and concerns along with the applicant's responses (in sub-bullets):

- Traffic concerns Questions were raised about the traffic impact analysis that was submitted with this request because it was based on 2019 data, and VDOT has indicated that traffic counts on Route 54 have increased 10-19% since then. It was noted there is a "blind curve" near Burleigh Drive, where those residents already have a hard time getting out. Not having a right turn lane at the first entrance to the project west of this curve might be an issue especially with all the additional trucks using Route 54. What are the turn lane lengths? It was asked if other proposed development in the area was included in the traffic study (East Ashland/Ironhorse property in particular) and if traffic numbers go up in the area, would they provide improvements to address the additional traffic.
  - o The applicant said the traffic consultant would be contacted about the traffic counts and revise the TIA and recommended roadway improvements, if necessary; will consider the "blind curve" near Burleigh Drive and the distance to the first right turn taper at the first entrance to the west of these locations.
  - o A right turn taper is provided at each entrance that typically is about two car lengths long, and left turn lanes will typically include a length for five cars.
  - o The traffic counts projected for Providence Church Road would not warrant a traffic signal at Route 54.
  - o Background growth in the TIA accounts for the known proposed development.
  - Traffic improvements are determined during the rezoning based on the trips of the proposed development at the time it is built.
- Features of the Development There were questions about the type of homes planned, their square footage, yard size, home visibility from Route 54, the detention pond, the size of the pavilion, the buffers, use of well water for sprinklers in the green spaces, high density, use of emergency access to Providence, the siding material to be used in the homes, preservation of trees in buffers, and when the project would be built out. Also, there was a request for more breaks between long rows of homes for additional pedestrian connections.
  - o Half of the homes will be detached single family and the other half townhomes, with all being between 1,600 and 2,400 square feet in size and starting in price at over \$400K.

- Would be mostly one-story homes or set up for first floor living with master bedrooms on the first floor and at the choice of the buyer, may have a bedroom and bath upstairs for visitors.
- o Backyards would be approximately 20' 50' deep depending on type of lot and unit.
- o Both sides of the interior streets will have sidewalks
- Wide HOA owned buffers with vegetation are provided along Route 54 to partially screen views to homes.
- o The detention pond is for stormwater runoff, and it will include a pier for fishing and walking trails around it. The pond will be stocked with fish and will be aerated. They will have two deep wells for water sprinklers, but one will be set up to keep the water in the pond filled with water during periods of no rain. Residential wells are typically shallow wells, and this should be a deep bored well, so will not draw from the same water source as the shallow wells.
- o The size of the pavilion was not provided, but the applicant felt the location for meetings of the HOA would not be highly attended (usually around 20% of residents) in his experience.
- o The emergency access to Providence cannot be widened enough to accommodate a road connection, but it could accommodate a pedestrian connection, which would be up to Providence as to whether that would be allowed.
- The siding on the homes would be high quality/thick vinyl. Normally have used hardie board, but there has been an issue with peeling on recent hardie board.
- The proposed density is 2.95 dwelling units/acre, which is within the range for Suburban General in the Comprehensive Plan, which allows up to 3.0 units/acre. The proposed density is not too high because this property is adjacent to a major thoroughfare road.
- The 100'-125' buffers shown on the plan will be landscaped and maintained by the HOA; where Providence Church Road is being vacated, that area would be divided between Providence and this development, and this development will work with Providence on what to do with this area and on any pedestrian connections.
- The applicant noted the breaks on long rows of housing for additional pedestrian connections would be considered.
- The applicant proposes connections with Providence so residents from both communities can use each other's trails and sidewalks but only with an agreement with Providence HOA.
- ➤ GPS issue for Providence Church Road It was requested that the County do something about GPS directing truck traffic incorrectly to Providence Church Road (instances with 18-wheelers coming all the way to its dead end and having to turn around on residents' properties or even having to back up the length of the road because the truck was too big to turn around)
  - o The County will look into this issue and see if the Sheriff can resolve it.
- Comprehensive plan The comprehensive plan for this site shows Planned Business; will there be a comp plan amendment prior to the rezoning being considered?
  - This property has been through two previous zoning requests that were in accordance with the Comprehensive Plan but were denied because of the office and retail uses. This tells the applicant that the citizens in this area only want residential, so that is what is being proposed. In addition, the proposed land use for the area as shown on the Envision Hanover maps is residential.

This project is tentatively scheduled for the June Planning Commission meeting. Should you have any questions regarding this meeting, please contact the staff.



## Cheely, Claudia

(804) 317-6141

From: Sent: To: Cc: Subject:	Joe Chambers <joechambers43@yahoo.com> Thursday, June 15, 2023 11:06 PM Maloney, David P.; Cheely, Claudia; Iverson, Edmonia P.; Thomas Agnello Goldstein Mitchell; Keith Manion; John Denler; June Sealy; Heather Pike Agnello; Todd Rogers; Kristin Parrish; Biernot, Gretchen W.; Monolo, J. Robert Re: Comments for Public Hearing Case: REZ2023-00003 Hickory Grove</joechambers43@yahoo.com>
CAUTION: This email originate especially from unknown sen	ed from outside your organization. Exercise caution when opening attachments or clicking links, ders.
Yes, I agree.	
On Thursday, June 15, 202	23 at 04:09:57 PM EDT, Thomas Agnello <ptagnello@gmail.com> wrote:</ptagnello@gmail.com>
Good afternoon,	
these concerns/questions r	
Several of our members wi	Il be at the PH this evening, but I cannot attend due to predetermined travel plans.
similar to what we did last s	quest a meeting between representatives of Providence with County Staff and the Developer summer for this case to discuss concerns and questions before this case goes to the Board for work together cooperatively to try to address PHOA member concerns.
Sincerely,	
Paul Agnello	
PHOA President for 2023	<del></del>



APR 2 0 2023

# COMMENTS ON JANUARY 2023 HICKORY GROVE TIA HICKORY HILL II LLC, REZ2023-00003

HANOVER COUNTY PLANNING OFFICE

These brief comments summarize traffic concerns raised at the April 19, 2023 community meeting.

<u>Existing Conditions</u>. The Traffic Impact Analysis (TIA) improperly uses stale and inadequate 2019 traffic counts and data to assess traffic impacts. Attached is an Excel spreadsheet which supports this conclusion.<sup>1</sup> The data is stale because it was collected on May 29, 2019 almost four years ago. The data is inadequate because it fails to consider VDOT annual reports from 2019 to 2021 and major 10.9% to 19.5% traffic growth in two years on Route 54 East and Goddins Hill Road, as well as the limited traffic data collected in May 2019.

2019 is not representative of existing conditions. A new TIA is required and must be prepared.

<u>Trip Generation</u>. The application at page 13 contains a form for compliance with VDOT and county TIA requirements. The form correctly lists the two codes (251 – senior adult single-family housing, 252 – senior adult multi-family housing) applicable to this rezoning and claims that the daily trip generation (vehicles per day) and AM and PM hour trip generation (vehicles per hour) were determined in compliance with the 11<sup>th</sup> Edition of the ITE Trip Generation Manual. However, the respective page numbers listed (403, 414) are largely cover pages for 20 pages of data and studies for weekday, peak hours, and other periods for code 251 (pages 405-413) and code 252 (pages 415-425).

I have reviewed the 11<sup>th</sup> Edition of the ITE Trip Generation Manual twice and cannot determine how the trip generation numbers in the application and the TIA (page 14) were derived. In my opinion, there is a lack of transparency. The application should not be processed until work papers and supporting documents are provided to resolve this issue.

APRIL RAN

Bob Nelson, Ashland District, Marian 20, 2023

<sup>&</sup>lt;sup>1</sup> The TIA reference to the VDOT 2019 jurisdictional report has been corrected in the attached Excel spreadsheet to page 7. Page 9 is nonetheless relevant because it confirms that May 2019 traffic counts were used in determining the "existing traffic volumes" in the January 2023 TIA. See also, TIA, Appendix B, pp. 35-43 – Peggy Malone & Associates May 29, 2019 Four Hour Traffic Counts.

# VDOT REPORTED ANNUAL AVERAGE DAILY TRAFFIC (AADT) AND ANNUAL AVERAGE WEEKDAY TRAFFIC (AAWDT)

	ROUTE 54 EAST - AS	SHLAND	ROUTE 54 EAST - HANOVER	NOVER	GODDINS HILL RD.	RD.
YEAR	AADT	AAWDT	AADT	AAWDT	AADT	AAWDT
Jan. 2023 TIA, p. 7	9,000		4,300		820	
2019	000′9	6,400	4,300	4,300	820	830
2020	6,100	9,500	4,400	4,300	860	840
2021	6,700	7,100	5,000	2,000	086	096
% increase 2021	11.7%	10.9%	16.3%	16.3%	19.5%	15.7%
		CONCINSI	CONCLUSIONS AND SOURCES			

VDOT reported increases ranging from 10.9% to 19.5% in average daily and weekday traffic in two years on roads studied in the Jan. 2023 TIA. Almost all of the increase was in 2021. The increases must be considered major. The traffic counts in the TIA were taken on May 29, 2019; and the data was collected only during four hours (7-9 AM and 4-6 PM). The TIA traffic count data is almost four years old. It is stale and inadequate. Given major increases in annual average daily and weekday traffic in recent VDOT reports and the stale and inadequate 2019 data used in he Jan. 2023 TIA, 2019 is not representative of current conditions. A new TIA is required and must be prepared. All AADT and AAWDT data is taken from the VDOT Jurisdictional Report 42 for Hanover County and the Town of Ashland. These reports are published in the following year, generally the spring. The 2022 report is not yet available.

AAWDT data is not reported in the TIA but is equal to or higher than AADT on Route 54 East of I-95, the primary road of concern.

**Proffers** 

JUN 3 0 2023

HANOVER COUNTY PLANNING OFFICE

### REZ2023-00003, Hickory Hill II, LLC Proffers:

The undersigned, Hickory Hill II, LLC, owner of parcels designated GPINs 7880-92-1335 (in part) and 7880-81-6802 (the "Property"), voluntarily agree for themselves, their agent, personal representatives, successors and assigns (collectively "the Property Owner") that, in the event the Property is rezoned from A-1 Agricultural District to RS Single-Family Residential District, the development and use of the Property must be subject to the following conditions:

- 1. Age Restriction on Residential Uses. The 150 single-family detached homes within the Property are to be occupied only by persons age 55 years and older and the occupancy of these dwellings must comply with provisions of the Virginia Fair Housing Law (found at Section 36-96.1 et seq. of the Code of Virginia (1950) as amended) and the Federal Fair Housing Act (found at Title VIII of the Civil Rights Act of 1968, as amended, 42 U.S.C. 3601-3619) related to "housing for older persons" as defined in the Virginia Fair Housing Law. A restrictive covenant must be recorded, and a homeowners' association established for the enforcement of such covenants, to limit the use and occupancy of the dwellings as specified herein.
- 2. Right-of-Way Dedication. The Property Owner agrees to dedicate fifty feet (50') of right-of-way measured from the centerline of East Patrick Henry Road (U.S. Route 54) to the Property and twenty-five feet (25') of right-of-way measured from the centerline of Providence Church Road where that road remains for future road widening as shown on the Hanover County Major Thoroughfare Plan, and fifty feet (50') of right-of-way for Realigned Providence Church Road. Dedication must take place when requested by the County and must be free of cost and free of encumbrances interfering with the use for road purposes.
- 3. <u>Transportation Improvements.</u> The Property Owner must be responsible for the following road improvements listed below in accordance with the timing specified herein.
  - a. Prior to the issuance of the first Certificate of Occupancy on the Property, the following improvements must be completed:
    - i. At Realigned Providence Church Road Entrance:
      - 1. Construct one eastbound left-turn lane on Route 54 with 200 feet of storage.
      - 2. Construct one westbound right-turn lane with 200 feet of storage.
      - 3. Provide separate southbound left and right-turn lanes on Realigned Providence Church Road.
    - ii. At Southwest Entrance to Hickory Grove:
      - 1. Construct one eastbound left-turn lane on Route 54 with 200 feet of storage.
      - 2. Construct one westbound 100' right-turn taper.
      - 3. Provide separate southbound left and right-turn lanes on the Southwest Entrance of Hickory Grove.
  - b. All such improvements must be designed and constructed in accordance with VDOT standards and specifications.
- 4. <u>Providence Church Road.</u> If a portion of Providence Church Road is approved for vacation or abandonment along the boundary of the Property, the Property Owner must construct a cul-de-sac prior to such vacation or abandonment to allow access from Providence Church Road on to Rt.

54, and dedicate as soon as practicable, with such cul-de-sac generally shown on the attached exhibit entitled "EXHIBIT OF POSSIBLE PROVIDENCE CHURCH ROAD CUL-DE-SAC, LOCATED IN THE BEAVERDAM DISTRICT OF HANOVER COUNTY, VIRGINIA", dated September 9, 2020, and prepared by the Bay Companies (the "Providence Church Road Plan"). It is intended that the cul-de-sac will partially be located on and serve as access to and from the portion of Providence Church Road that adjoins the property with an existing GPIN of 7880-72-6758, all as shown on such Providence Church Road Plan. The cul-de-sac design will be shown on the construction plans for the adjacent section of the subdivision and bonded as a subdivision improvement.

- 5. Enhanced Thoroughfare Buffer. The 50' road buffer along East Patrick Henry Road must be landscaped in accordance with the thoroughfare buffer standards (Section 26-264.3), which will be enhanced by increasing the required plantings by 25% for trees and 30% for shrubs. Trees must consist of a combination of deciduous and evergreen trees and plant materials must be appropriately clustered to create a more natural and organic design.
- 6. <u>Recreational Amenities.</u> The pedestrian paths and trails, pavilions, gazebo, pergola, community gardens, dog park and pickle ball courts, as shown on the Conceptual Plan, must be designed, bonded, and constructed as subdivision improvements with the first section that is developed.
- 7. <u>Trailer/RV Storage Area Landscaping</u>. The Property Owner must provide Commercial Screening to the back of the Trailer/RV Storage area, as seen from Providence Church Road, unless otherwise approved by the Hanover County Zoning Administrator that existing foliage is found to be adequate. If additional plantings are found necessary, they must consist of a combination of deciduous and evergreen trees and plant materials must be appropriately clustered to create a more natural and organic design.

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Higkory Hill II, LLC, a Virginia limited liability company	
By: What Cape	
Title: Menher Manuge	
COMMONWEALTH OF VIRGINIA,	
COUNTY OF HANOVER, to-wit:  I, Panela G astro, hereby certify that  Michael H Campbell [Title] Member Manager	on behalf of
Hickory Hill II, LLC, has acknowledged the foregoing Proffers before me, this 30 d	lay of
Parly Cour	(SEAL)
Notary Public	MININA G CANA
My Commission Expires: March 6, 2026	NOTARY OF NOTARY
	PUBLIC

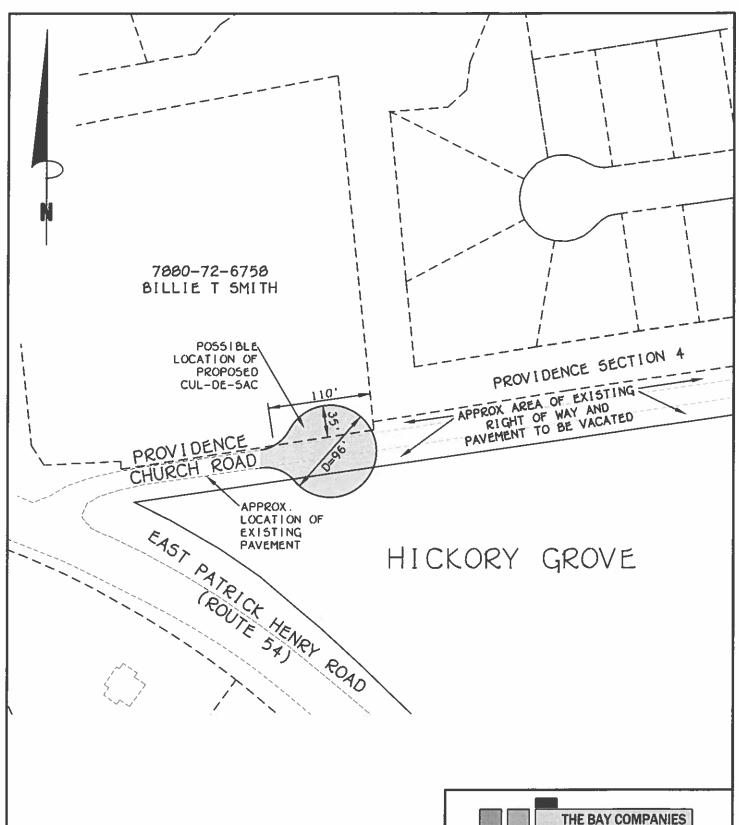


EXHIBIT OF

POSSIBLE PROVIDENCE CHURCH ROAD

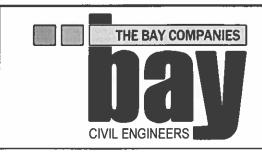
CUL-DE-SAC

LOCATED IN THE BEAVERDAM DISTRICT OF

HANOVER COUNTY, VIRGINIA

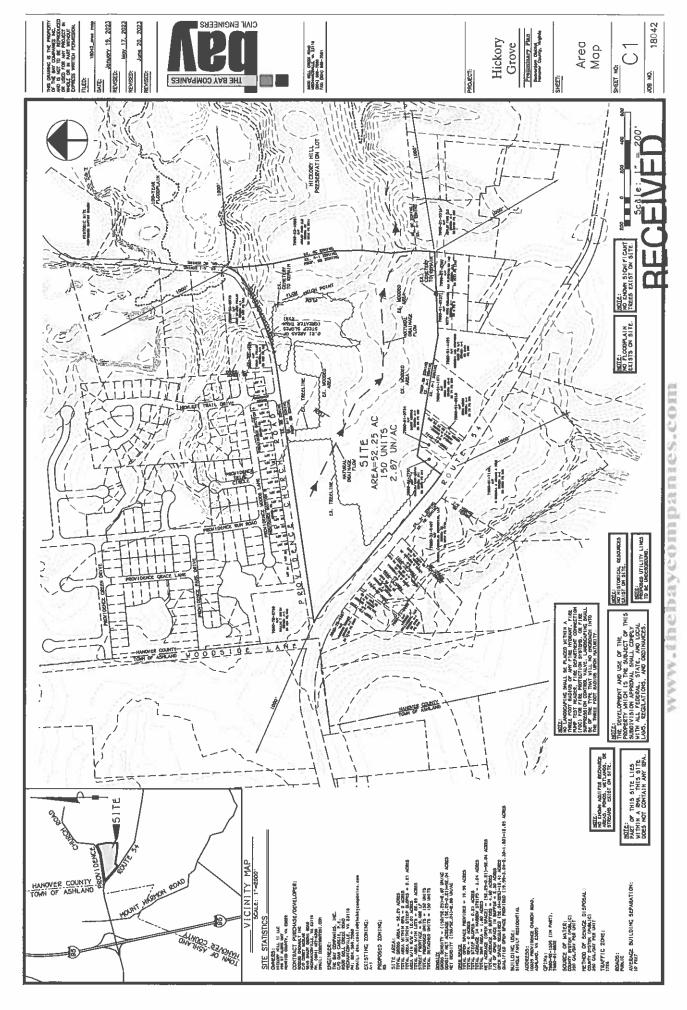
SCALE: 1"= 100'

DATE: SEPTEMBER 9, 2020

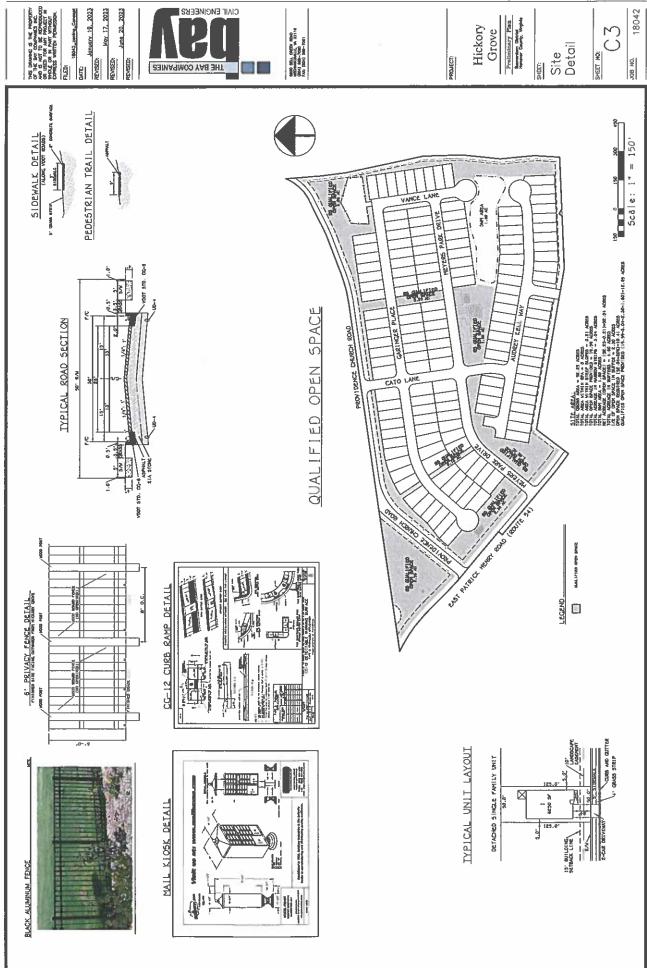


8500 BELL CREEK ROAD MECHANICSVILLE, VA 23116
PH:(804)569-7060 FILED:18042\_cul-de-soc exhibit





www.thebaycompanies.com



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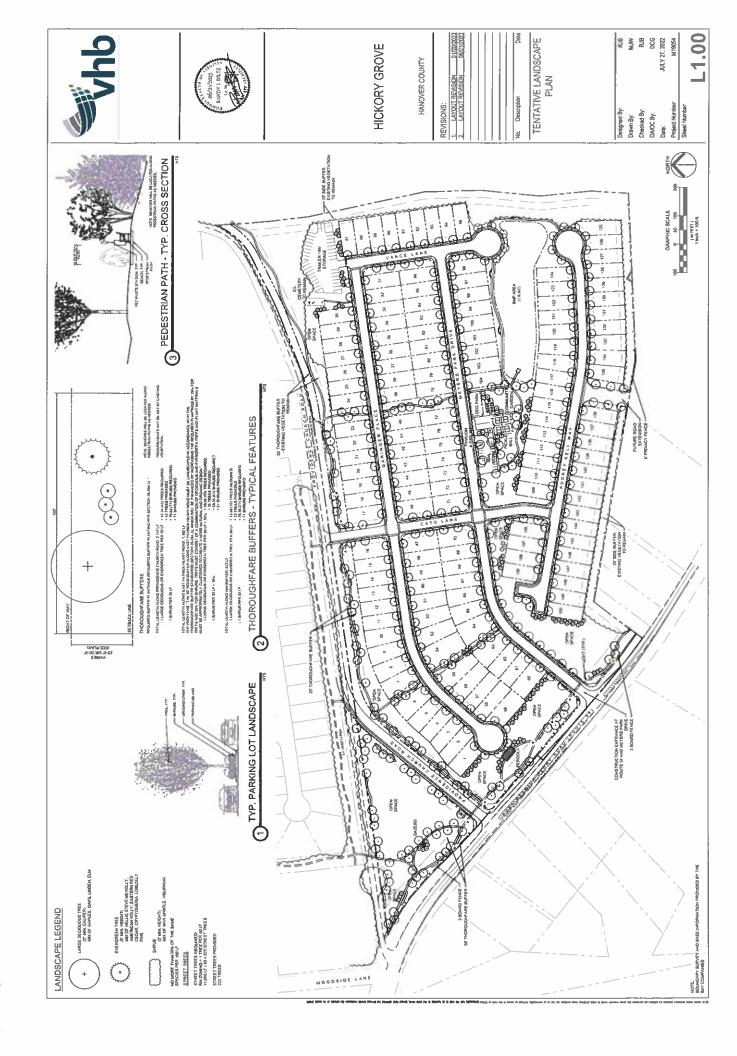
www.thebaycompanies.com

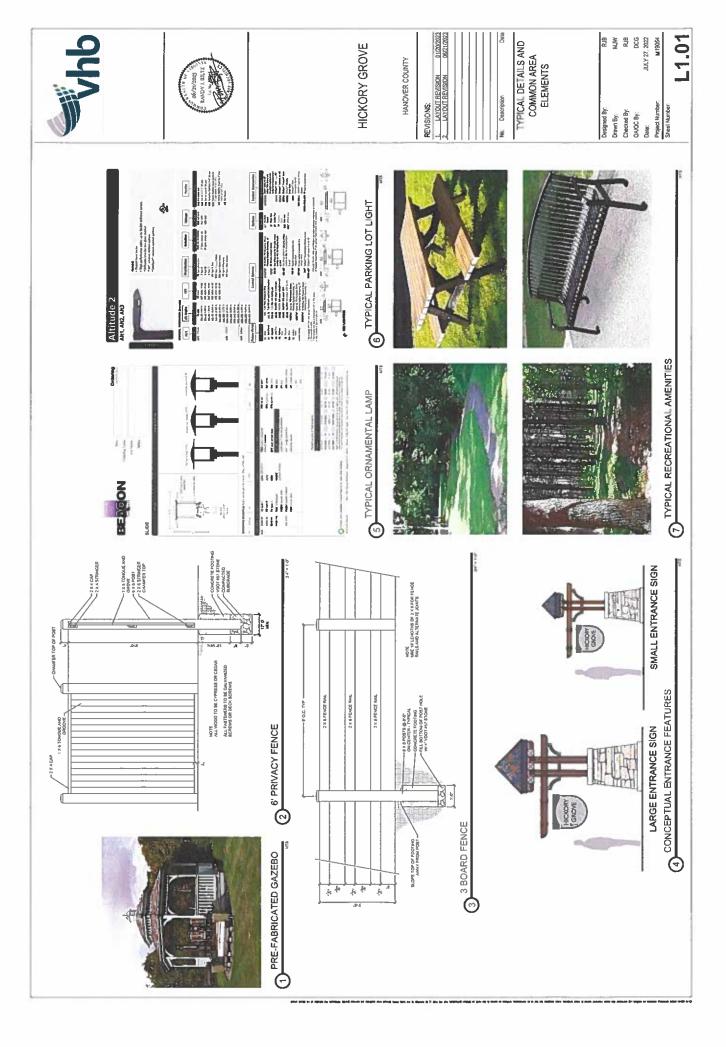
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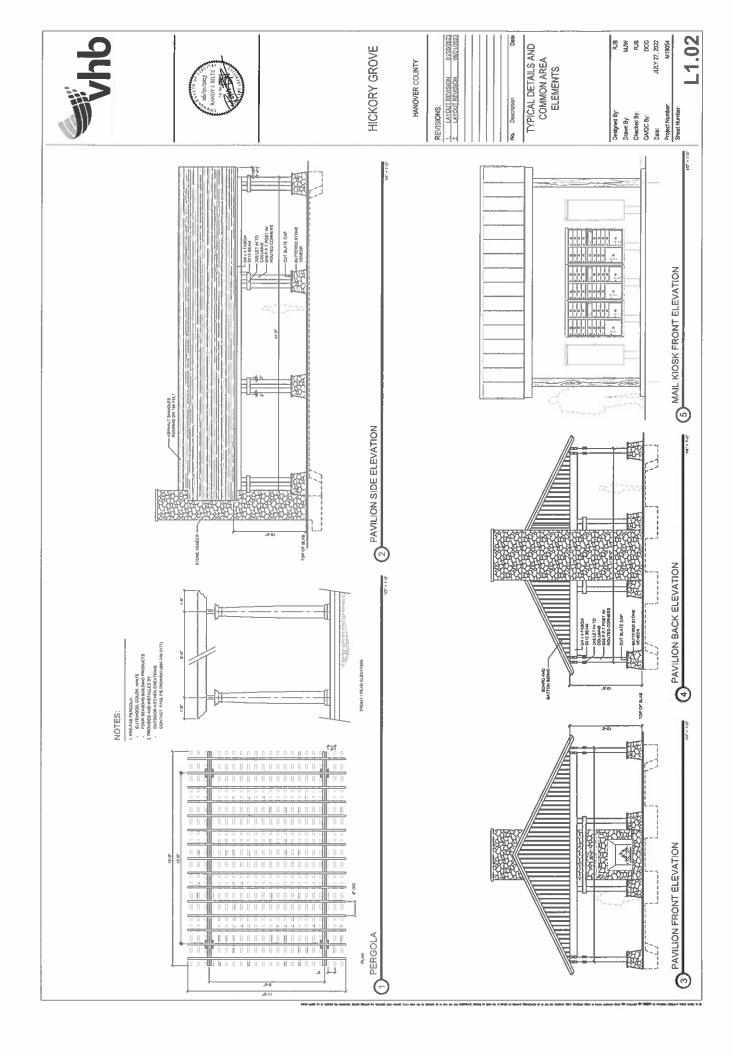


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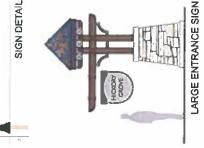












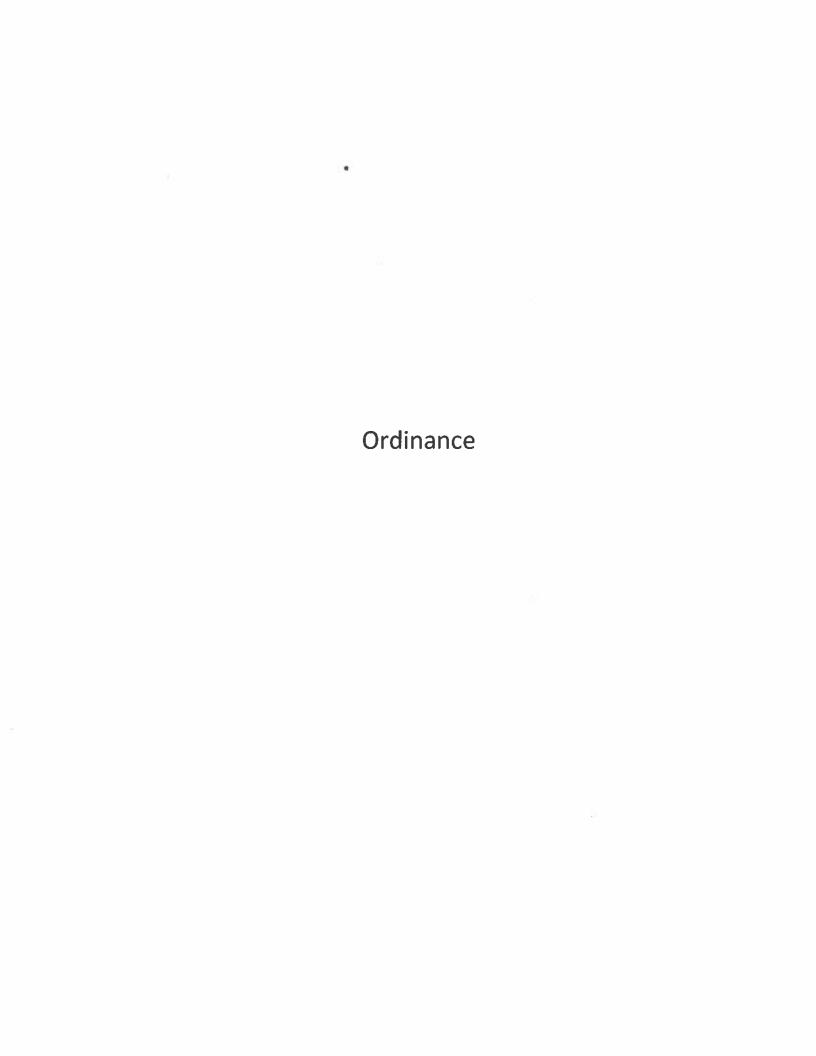












### ORDINANCE REZ2023-00003

### OWNER OF RECORD: HICKORY HILL II, LLC

WHEREAS the Planning Commission of Hanover County has held an advertised public hearing and forwarded this case to the Board of Supervisors with a recommendation of APPROVAL of the adoption of the following amendment to the Zoning Ordinance and Zoning District Map of Hanover County; and

WHEREAS the Board of Supervisors has held public hearings on the July 26, 2023, and advertised in the Mechanicsville Local once a week for two successive weeks as required by Virginia Code Section 15.2-2204; and

WHEREAS the Board of Supervisors has determined that the public necessity, convenience, general welfare, and good zoning practices require this amendment.

NOW, THEREFORE, BE IT ORDAINED by the Board of Supervisors of Hanover County that the Zoning Ordinance and the Zoning District Map of this County are amended (with conditions) by the rezoning of the property described as GPINs 7880-92-1335(part) and 7880-81-6802, consisting of approximately 52.25 acres, and located on the north line of East Patrick Henry Road (State Route 54) at its intersection with Providence Church Road (State Route 662), from A-1, Agricultural District, to RS(c), Single-Family Residential District with conditions, subject to the following conditions, which were proffered by the Applicant on June 30, 2023, and accepted by the Board:

- 1. Age Restriction on Residential Uses. The 150 single-family detached homes within the Property are to be occupied only by persons age 55 years and older and the occupancy of these dwellings must comply with provisions of the Virginia Fair Housing Law (found at Section 36-96.1 et seq. of the Code of Virginia (1950) as amended) and the Federal Fair Housing Act (found at Title VIII of the Civil Rights Act of 1968, as amended, 42 U.S.C. 3601-3619) related to "housing for older persons" as defined in the Virginia Fair Housing Law. A restrictive covenant must be recorded, and a homeowners' association established for the enforcement of such covenants, to limit the use and occupancy of the dwellings as specified herein.
- 2. Right-of-Way Dedication. The Property Owner agrees to dedicate fifty feet (50') of right-of-way measured from the centerline of East Patrick Henry Road (U.S. Route 54) to the Property and twenty-five feet (25') of right-of-way measured from the centerline of Providence Church Road where that road remains for future road widening as shown on the Hanover County Major Thoroughfare Plan, and fifty feet (50') of right-of-way for Realigned Providence Church Road. Dedication must take place when requested by the County and must be free of cost and free of encumbrances interfering with the use for road purposes.
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- 3. Provide separate southbound left and right-turn lanes on Realigned Providence Church Road.
- ii. At Southwest Entrance to Hickory Grove:
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  - 2. Construct one westbound 100' right-turn taper.
  - 3. Provide separate southbound left and right-turn lanes on the Southwest Entrance of Hickory Grove.
- b. All such improvements must be designed and constructed in accordance with VDOT standards and specifications.
- 4. Providence Church Road. If a portion of Providence Church Road is approved for vacation or abandonment along the boundary of the Property, the Property Owner must construct a cul-de-sac prior to such vacation or abandonment to allow access from Providence Church Road on to Rt. 54, and dedicate as soon as practicable, with such cul-de-sac generally shown on the attached exhibit entitled "EXHIBIT OF POSSIBLE PROVIDENCE CHURCH ROAD CUL-DE-SAC, LOCATED IN THE BEAVERDAM DISTRICT OF HANOVER COUNTY, VIRGINIA", dated September 9, 2020, and prepared by the Bay Companies (the "Providence Church Road Plan"). It is intended that the cul-de-sac will partially be located on and serve as access to and from the portion of Providence Church Road that adjoins the property with an existing GPIN of 7880-72-6758, all as shown on such Providence Church Road Plan. The cul-de-sac design will be shown on the construction plans for the adjacent section of the subdivision and bonded as a subdivision improvement.
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BE IT FURTHER ORDAINED that this Ordinance is effective on the date of adoption and the Planning Director is hereby directed to designate the boundaries of the foregoing area as rezoned, subject to conditions, on the Zoning District Map of Hanover County.

On motion of	, seconded by	, the
members of the Board of Supervisors voted to	o approve Ordinance REZ2023-0	00003, as follows:
	Vote:	
Angela Kelly-Wiecek		
Sean M. Davis Susan P. Dibble		
W. Canova Peterson, IV		
Faye O. Prichard		
J. Robert Monolo F. Michael Herzberg		
r. Michael Herzberg		
Public Hearings:		
Planning Commission: June 15, 2023		
Board of Supervisors: July 26, 2023		
Adopted: July 26, 2023		
This is to certify that the above is a true	2001 of DE 72022 00002 adopt	ed by the Usnover Coun
•	ecopy of REZ2023-00003, adopt	ed by the Hallovel Coun
Board of Supervisors on July 26, 2023.		
Dated:		
	John A. Budesky	
	County Administrator/Cle	erk Hanover County
	Board of Supervisors	